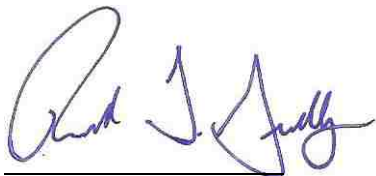


**SPECIFICATIONS
FOR THE
MIDDLE CONCHO BOAT RAMP
IMPROVEMENTS
MIDDLE CONCHO PARK
SAN ANGELO, TEXAS**

PROJECT SPECIFICATIONS

31 0800 COMMISSIONING OF SITEWORK
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DUPLEX GRINDER PUMP



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SECTION 31 0800

COMMISSIONING OF SITEWORK

PART 1 GENERAL

1.01 SECTION INCLUDES

- A. Requirements for Sitework.

PART 2 PRODUCTS

2.01 NOT USED

PART 3 EXECUTION

3.01 EXAMINATION

A. Site Verification of Conditions:

1. 48 hours minimum prior to performing any work on site, contact Dig Tess to arrange for utility location services.
2. Perform minor, investigative excavations to verify location of various existing underground facilities at sufficient locations to assure that no conflict with the proposed work exists and sufficient clearance is available to avoid damage to existing facilities.
3. Perform investigative excavations 5 days minimum in advance of performing any excavation or underground work.
4. Upon discovery of conflicts or problems with existing facilities, notify Engineer within 24 hours.

- B. All work shall be performed in accordance with the Plans and Specifications. In the event there are inconsistencies between the plans and specifications, the more stringent shall apply.

3.02 PREPARATION

A. Protection:

1. Erosion Control: Take precautions necessary to prevent erosion and transportation of soil downstream, to adjacent properties, and into on-site or off-site drainage systems. The contractor and subcontractors shall be responsible for meeting the requirements of the TPDES General Permit No. TXR150000 and shall prepare a *Storm Water Pollution Prevention Plan* where required and adhere to the requirements set forth therein. The Contractor shall provide the Owner and Engineer with copies of all documentation associated with the plan.

2. Dust Control: Take precautions necessary to prevent dust nuisance, both on-site and adjacent to public and private properties. Correct or repair damage caused by dust.
 3. Spillage: Avoid spillage by covering and securing loads when hauling on or adjacent to public streets or highways. Remove spillage and sweep, wash, or otherwise clean project, streets, and highways.
 4. Existing Plants and Features: Do not damage tops, trunks, and roots of existing trees and shrubs on site which are intended to remain. Do not use heavy equipment within branch spread. Interfering branches may be removed only with permission of Engineer. Do not damage other plants and features which are to remain.
- B. Dimensional Control:
1. Survey Control: Refer to the plans for coordinate points or dimensional control data. The contractor shall field verify points and protect the same during construction. The contractor shall be responsible for all construction staking associated with the project.
- C. REPAIR / RESTORATION
1. Adjust existing covers, boxes, and vaults to proposed/finished grade.
 2. Replace broken or damaged covers, boxes, and vaults.
 3. Independently confirm size, location, and number of covers, boxes, and vaults which require adjustment.
- D. TRAFFIC CONTROL
1. Contractor to provide traffic control devices throughout duration of project.
- E. FIELD QUALITY CONTROL
1. Owner will engage a testing and inspection service for quality control testing during construction.
 2. Owner reserves right to require additional testing to re-affirm suitability of completed work including compacted soils which have been exposed to adverse weather conditions.

END OF SECTION

CITY OF SAN ANGELO**ITEM 100****PREPARING RIGHT-OF-WAY****100.1. DESCRIPTION.**

This Item shall govern the clearing and grubbing for the preparation of the Right-of-Way for construction operations by the removal and disposal of all obstructions from the Right-of-Way and from designated easements, where removal of all such obstructions is not otherwise shown on the Plans and specifications.

Such obstructions shall be considered to include remains of houses, foundations, floor slabs, concrete, brick, lumber, plaster, septic tank drain fields, basements, abandoned utility pipes or conduits, equipment, fences, retaining walls, outhouses, and shacks.

This Item shall also include the removal of trees and shrubs and other landscape features not designated for preservation, stumps, brush, roots, vegetation, logs, curb and gutter, driveways, paved parking areas, miscellaneous stone, sidewalks, drainage structures, manholes, inlets, abandoned railroad tracks, scrap iron and debris, whether above or below ground except live utility facilities.

100.2. CONSTRUCTION METHODS.

(1) General. All areas as shown on the Plans, shall be cleared of all structures and obstructions as defined above. Those trees, shrubs and other landscape features specifically designated by the City for preservation shall be carefully protected from abuse, marring, or damage during construction operations. Continual parking and/or servicing of equipment under the branches of trees marked for preservation will not be permitted. When trees and shrubs are designated for preservation and require pruning, they shall be trimmed as directed by the City and all exposed cuts over two (2) inches in diameter shall be treated with a material approved by the City.

Culverts, storm sewers, manholes and inlets shall be removed in proper sequence for maintenance of traffic and drainage.

Underground obstructions, except those items designated for preservation, shall be removed to the following depths:

(a) In areas to receive embankment: one (1) foot below natural ground, except when permitted by the Plans, trees and stumps may be cut off as close to natural ground as practicable on areas which are to be covered by at least three (3) feet of embankment.

(b) In areas to be excavated: one (1) foot below the low elevation of the excavation.

(c) All other areas: one (1) foot below natural ground.

(2) Disposal of Material. Unless otherwise shown herein, all materials and debris removed shall become the property of the Contractor, including all merchantable timber, and shall be removed from the Right-of-Way and disposed of in a manner satisfactory to the City, except that gravel, brick, stone, or broken concrete, when approved by the City, may be used in the roadway embankment. This material shall conform to the requirements of Item 132, "Embankment".

(a) State or National Forest or Park: The provisions shown on the Plans for removal of the timber shall apply. No timber shall be cut or defaced outside of the Right-of-Way lines or material pit limits as indicated on the Plans or by the City.

(b) Burning of Brush: When burning of brush is permitted under applicable laws and by the City, the following shall govern.

(i) Where construction is on new location, the brush shall be piled and burned in the center of the work area.

(ii) When a portion of the project falls within the limits of a state or National Forest or Park, the Contractor shall notify the responsible agencies prior to any burning.

(3) Backfill. Holes remaining after removal of all obstructions, objectionable material, trees, stumps, etc., shall be backfilled with approved material, compacted and restored to approximately its original contours by blading, bulldozing, or by other methods, as approved by the City. In areas to be immediately excavated, the backfilling of holes may not be required when approved by the City.

Before backfilling, the remaining ends of all abandoned storm sewers, culverts, conduits, and water or gas pipes over 3 inches in diameter, shall be plugged with an adequate quantity of concrete to form a tight closure.

100.3 MEASUREMENT.

(1) Methods of Measurement. This Item will be measured by one of the following methods:

(a) Preparing Right-of-Way (Acre). The work performed will be measured by the by the nearest tenth of an acre of Right-of-Way as shown on the Plans.

(b) Preparing Right-of-Way (Station). The work performed will be measured by the "100-Foot Station" regardless of the width of the Right-of-Way as shown on the Plans.

(c) Preparing Right-of-Way (Tree). The work performed will be measured by each tree removed of the diameter specified.

(2) General. Measurement for payment for "Preparing Right-of-Way (Acre)" and for "Preparing Right-of-Way (Station)" will be made only on areas indicated and classified on the Plans as "Preparing Right-of-Way". Work required by the City on additional areas (such as additional Right-of-Way, additional cut and embankment areas, etc.) shall be measured as specified above.

Areas other than those set forth above will not be measured for payment.

100.4. PAYMENT.

The work performed and material furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Preparing Right-of-Way," and/or "Preparing Right-of-Way (Trees)" of the diameter specified. This price shall be full compensation for trimming designated trees and shrubs; for removal and disposal of all obstructions and debris; for backfilling all holes; for furnishing and placing all concrete for plugs; for restoring areas to original condition; and for all labor, equipment, tools and incidentals necessary to complete the work.

All work performed in areas not so designated on the Plans as "Preparing Right-of-Way," except "additional areas" as defined under "Measurement" or specifically covered otherwise, will not be paid for directly but shall be considered as subsidiary work pertaining to the various bid items.

The total payment for this Item will not exceed ten (10) percent of the original contract amount until after the completion of the entire contract work to the satisfaction of the City. That portion of the contract amount for this Item in excess of ten (10) percent of the total contract amount will then be paid on the next estimate after the work is accepted and after the partial release of retainage.

CITY OF SAN ANGELO**ITEM 107****SEEDING FOR EROSION CONTROL****107.1 DESCRIPTION.**

Seeding for Erosion Control shall consist of preparing the areas, providing for sowing of seeds, mulching with straw, hay or cellulose fiber and other management practices for the cut slopes and for the bottom of the channel detention pond area, embankments, dikes, beams and the other areas as shown on the Plans or as directed by the City excluding the rock surface in accordance with these specifications.

107.2 SEEDS.

All seeds shall meet the requirements of the Texas Seed Law including the labeling requirements for showing pure live seed, name and type of seed.

The pure live seed mixture shall consist of the following seeds in the amount noted per acre of planting area.

Green Sprangletop	- 1.2 lbs.
Common Bermuda	- 8.0 lbs.
Klien	- 1.0 lbs.
Perennial Rye	- 2.0 lbs.

Fertilizer, if specified on the Plans, shall be delivered in bags or containers clearly labeled showing the analysis. The fertilizer, if requested by the City, will be subject to testing by the Texas Department of Agriculture in accordance with the Texas Fertilizer Law. A pelleted or granulated fertilizer shall be used which has the analysis shown on the Plans. The rate of application shall be as shown on the Plans.

Straw mulch shall be oat, wheat, or rice straw. Hay mulch shall be prairie grass, Bermuda grass or other acceptable hay. The mulch shall be free of noxious weeds and foreign materials. It shall be kept in a dry condition and shall not be molded or rooted.

107.3 CONSTRUCTION METHODS.

After the areas designated for seeding are cut to the lines, grades, cross sections and to the configurations shown on the Plans, the area shall be cultivated to a depth of at least four inches. The seed bed shall be cultivated sufficiently to reduce the soil to a state of good tilth for proper germination of the grass seeds. The cross-section and configuration previously established shall be maintained throughout the process of cultivation and any necessary reshaping shall be done prior to planting of the seeds.

The seed mixture shall be uniformly distributed over the areas shown on the Plans or where directed by the City by the mechanical equipment. Seed and fertilizer (if called on the Plans) may be distributed at the same time provided that each component is uniformly applied at the specified rate. After planting, the planted area shall be rolled with corrugated roller of the "cultipacker" type. All rolling of the slope areas shall be on the contours. For areas smaller than half an acre, sowing of seeds by hand will be permitted. If sown by hand, the seeds shall be sown in two (2) directions at right angles to each other. Upon completion of planting of the seeds, straw or hay mulch shall be spread uniformly over the seeded area at the rate of approximately one half (1/2) to two (2) tons of hay or two (2) to two and a half tons (2 1/2) of straw per acre. The Contractor in lieu of placing the mulch by hand may use a mulching machine to shoot the mulch over the seeded area. If permitted by the City, cellulose fiber may be used in place of straw or hay mulch. The application rate for cellulose fiber as mulch shall be a minimum of twenty five hundred (2,500) lbs. per acre of area with flat surface and minimum of three thousand (3,000) lbs. per acre of area with sloped surfaces (greater than 10 percent).

Water shall be free of industrial wastes and other objectionable material. Water source to be approved by the City.

The criterion for acceptance of the work under this Item is "Good Stand of Grass." The "Good Stand of Grass" is defined as a minimum of one-half (1/2) inch of well established live grass covering at least 75 percent of the area designated for seeding. If it is necessary, the bald spots in the seeded area shall be selectively reseeded at the direction of the City.

107.4 METHOD OF MEASUREMENT.

The work performed in accordance with the specifications described herein will be measured by the acres, complete in place.

107.5 PAYMENT.

The work performed, materials furnished and measured will be paid for at the unit price bid for "Seeding for Erosion Control" which shall be full compensation for furnishing all materials and for performing all operations necessary to complete the work in an acceptable manner.

If the Contractor were to fail to provide the requirements of this Item as specified herein, the Owner shall reserve the right to engage another Contractor to complete the work and the cost thereof shall be deducted from monies payable to the Contractor for this Item.

CITY OF SAN ANGELO**ITEM 110****EXCAVATION****110.1. DESCRIPTION.**

This Item shall govern for the roadway, channel and/or special excavation of the required material in the areas shown on the Plans and cross sections to the lines, grades, and typical sections as specified. Excavation shall include all materials encountered regardless of their nature or of the manner in which they are removed.

110.2. CONSTRUCTION METHODS.

All excavation shall be performed as specified herein and the completed roadway and/or channels shall conform to the alignment, grades, and typical sections as shown on the Plans or project cross sections or as established by the City.

Unsuitable excavation and excavation in excess of that needed for construction shall be known as "Waste" and shall become the property of the Contractor to be disposed of by him outside the limits of the right-of-way at a location approved by the City. Unsuitable material encountered below subgrade elevation in roadway cuts, when declared "Waste" by the City, shall be replaced with material from the roadway excavation or with other suitable material as approved by the City. This work shall be done in accordance with the provisions of the applicable bid items.

When excavated materials, including topsoil, are utilized in constructing the required roadway sections, payment of replacement will be made under the pertinent placement specification.

During construction the roadbed and ditches shall be maintained in such condition as to insure proper drainage at all times. Ditches and channels shall be so constructed and maintained as to avoid damage to the roadway section. During construction, channels shall be kept drained, insofar as practicable, and the work shall be prosecuted in a neat and workmanlike manner.

All slopes shall be accurately shaped, and care shall be taken that no material is loosened below or outside the required slopes. Exceptions shall be those slopes in rock or other material where, in the judgment of the City, some variation may be permitted. All breakage and slides shall be removed and disposed of in a manner acceptable to the City.

(1) Rock Cuts. The Contractor shall have the following options:

(a) Nonhomogeneous Rocks:

(i) Excavate to finish subgrade elevation, manipulate and compact the subgrade in accordance with Section 132.3.(3).(a) without removal.

(ii) Excavate below grade (undercutting) and replace with embankment material approved by the City. Compaction shall be in accordance with Section 132.3.(3).(a).

(b) Homogeneous Rock.

(i) Excavate to finish subgrade elevation.

(ii) Excavate to finish subgrade elevation, manipulate and compact the subgrade in accordance with Section 132.3(3).(a) without removal.

(iii) Excavate below grade (undercutting) and replace with embankment material, limestone base material, or other material approved by the City. Compaction shall be in accordance with Section 132.3(3).(a).

(2) Earth Cuts. When base and/or pavement structure is placed under this project, all earth cuts shall be scarified to a uniform depth of at least six inches below the required finished subgrade elevation for the entire roadbed width. The material shall be mixed and reshaped by blading and then sprinkled and rolled in accordance with Section 132.3(3).(a) or as shown on the Plans.

(3) Subgrade Tolerances. Tolerances shall be as follows:

(a) Stage Construction. Any deviation in excess of 0.1 foot in cross section and 0.1 foot in 16 feet measured longitudinally shall be corrected by loosening, adding or removing the material, reshaping and recompacting by sprinkling and rolling.

(b) Turn Key Construction. Any deviation in excess of half inch in cross section and half inch in 16 feet measured longitudinally shall be corrected by loosening, adding or removing the material, reshaping and recompacting by sprinkling and rolling.

110.3. MEASUREMENT.

This Item will be measured by the cubic yard in its original position as computed by the method of average end areas.

This is a Plans quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans, except as may be modified by special condition. If no adjustment of quantities is required, additional measurements or calculations will not be required.

Shrinkage or swellage factors will not be considered in determining the calculated quantities.

110.4. PAYMENT.

The work performed and materials furnished in accordance with the Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Excavation (Roadway)," "Excavation (Channel)," "Excavation (Special)" or "Excavation (Roadway and Channel)." This price shall be full compensation for all authorized excavation; for the undercutting subgrade and reworking or replacing the undercut material; for all hauling; for all work required for disposal of material not used elsewhere on the project and for furnishing all labor, materials, tools, equipment and incidentals necessary to complete the work.

CITY OF SAN ANGELO**ITEM 132****EMBANKMENT****132.1 DESCRIPTION.**

This Item shall govern for the placement and compaction of all materials necessary for the construction of roadway embankments, levees, and dykes or any designated section of the roadway where additional material is required.

132.2 MATERIAL.

Materials may be furnished from required excavation in the areas shown in the Plans or from off right-of-way sources obtained by the Contractor and meeting the requirements herein. All embankment shall consist of suitable earth material such as rock, loam, clay, or other such materials as approved by the City that will form a stable embankment.

132.3 CONSTRUCTION METHODS.

(1) **General.** Prior to placing any embankment, all work in accordance with Part II – Construction Details, Item 100, “Prepare Right-of-Way”, shall have been completed on the areas over which the embankment is to be placed. Stump holes or other small excavation in the limits of the embankments shall be backfilled with suitable material and thoroughly tamped by approved methods before commencing embankment construction. The surface of the ground, including disk-loosened ground or any surface roughened by small washes or otherwise, shall be restored to approximately its original slope by blading or other methods. Where shown on the Plans or required by the City, the ground surface thus prepared shall be compacted by sprinkling and rolling.

The City shall be notified sufficiently in advance of opening any material source to allow performance of any required testing.

Unless otherwise shown on the Plans, the surfaces of unpaved areas (except rock) which are to receive embankment shall be loosened by scarifying to a depth of at least six inches. Hillside shall be cut into steps before embankment materials are placed. Placement of embankment materials shall begin at the low side of hillside and slopes. Materials which have been loosened shall be recompacted simultaneously with the new embankment materials placed upon it. The total depth of loosened and new materials shall not exceed the permissible depth of the layer to be compacted.

Trees, stumps, roots, vegetation or other unsuitable materials shall not be placed in embankment.

Unless otherwise shown on the Plans, all embankments shall be constructed in layers approximately parallel to the finished grade of the roadbed.

Embankments shall be constructed to the grade sections shown on the Plans or as established by the City. Each section of the embankment shall correspond to the detailed section or slopes established by the City. After completion of the roadway, it shall be continuously maintained to its finished section and grade until the project is accepted.

(2) Constructing Embankments.

(a) Earth Embankments: Earth embankments shall be defined as those composed principally of material other than rock, and shall be constructed of acceptable material from approved sources.

Unless otherwise specified, earth embankments shall be constructed in successive layers for the full width of the individual roadway cross section and in such lengths as are best suited to the sprinkling and compacting methods utilized.

Layers of embankment may be formed by utilizing equipment and methods which will evenly distribute the material.

A minor quantity of rock or broken concrete encountered in the construction of this project may be incorporated in the lower layers of the embankment if acceptable to the City. Or, it may be placed in the deeper fills, in accordance with the requirements for the construction of rock embankments, provided such placement of rock is not immediately adjacent to structures or in areas where bridge foundations are to be constructed. Also, rock or broken concrete may be placed in the portions of embankments outside the limits of the completed roadbed width where the size of the rock or broken concrete prohibits its incorporation in the normal embankment layers. All exposed reinforced steel shall be cut and removed from the broken concrete.

Each layer of embankment shall be uniform as to material, density and moisture content before beginning compaction. Where layers of unlike materials abut each other, each layer shall be featheredged for at least 100 feet, or the material shall be so mixed as to prevent abrupt changes in the soil. No material placed in the embankment by dumping in a pile or windrow shall be incorporated in a layer in that position, but all such piles or windrows shall be moved by blading or similar methods. Clods or lumps of material shall be broken and the embankment material mixed by blading, harrowing, disking or similar methods until a uniform material of uniform density is achieved in each layer.

It shall be the responsibility of the Contractor to secure a uniform moisture content throughout the layer by such methods as may be necessary.

(b) Embankment Adjacent to Culverts: As a general practice, embankment material placed adjacent to any portion of any structure and in the first two (2) layers above the top of any culvert or similar structure shall be free of any appreciable amount of gravel or stone particles more than four inches in greatest dimension and of such gradation as to permit thorough compaction. When, in the opinion of the City, such material is not readily available, the use of rock or gravel mixed with earth will be permitted, in which case no particle larger than 12 inches in greatest dimension and six (6) inches in least dimension may be used. The percentage of fines shall be sufficient to fill all voids and insure a uniform and thoroughly compacted mass of proper density.

(3) Compaction Method. Compaction of embankments shall be by “Ordinary Compaction” or “Density Control” as shown on the Plans.

(a) Ordinary Compaction. When “Ordinary Compaction” is shown on the Plans, the following provisions shall govern:

Each layer shall not exceed eight (8) inches of loose depth, unless otherwise directed by the City. Each layer shall be compacted in accordance with the provisions governing the Item or Items of “Rolling”. Unless otherwise specified on the Plans, the rolling equipment shall be as approved by the City. Compaction shall continue until there is no evidence of further compaction. Prior to and in conjunction with the rolling operation, each layer shall be brought to the moisture content directed by the City, and shall be kept leveled with suitable equipment to insure uniform compaction over the entire layer. Should the subgrade, for any reason or cause, lose the required stability or finish, it shall be recompacted and finished at the Contractor's expense.

(b) Density Control. When “Density Control” is shown on the Plans, the following provisions shall apply:

Each layer shall be compacted to the required density by any method, type and size of equipment which will give the required compaction. The depth of layers, prior to compaction, shall depend upon the type of sprinkling, mixing and compacting equipment used. However, maximum depth (16 inches loose and 12 inches compacted) shall not be exceeded unless approved by the City. Prior to and in conjunction with the rolling operation, each layer shall be brought to the moisture content necessary to obtain the required density and shall be kept leveled with suitable equipment to insure uniform compaction over the entire layer.

Each layer shall be sprinkled and compacted to the extent necessary to provide the density specified below, unless otherwise shown on the Plans.

DESCRIPTION	DENSITY, PERCENT	MOISTURE
Non-Swelling Soils with plasticity index less than 20	Not less than 95	+/- 2% of optimum
Swelling soils with plasticity index of 20 to 35	Not less than 95	+/- 2% of optimum
Swelling soils with plasticity index over 35	Not less than 95	+/- 2% of optimum

The density determination will be made in accordance with Test Method Tex-114-E / ASTM D 698.

Field density determination by nuclear gage will be made in accordance with Test Method Tex-115-E / ASTM D 2922 for field density and ASTM D 3017 for moisture content.

After each layer of earth embankment is complete, tests as necessary may be made by the City. When the material fails to meet the density requirements or should the material lose the required stability, density, moisture or finish before the next course is placed or the project is accepted,

the layer shall be reworked as necessary to obtain the specified compaction, and the compaction method shall be altered on subsequent work to obtain specified density. Such procedure shall be subject to the approval of the City.

Excessive loss of moisture shall be construed to exist when the subgrade soil moisture content is four percent less than the optimum.

The Contractor may be required to remove a small area of the layer in order to facilitate the taking of density tests. Replacement and compaction of the removed material in the small area shall be at the Contractor's expense.

132.4 TOLERANCES.

The tolerances shall be as follows:

(1) Grade Tolerances.

(a) Stage Construction: Any deviation in excess of 0.1 foot in cross section and 0.1 foot in 16 feet measured longitudinally shall be corrected by loosening, adding or removing the material, reshaping and recompacting by sprinkling and rolling.

(b) Turnkey Construction: Any deviation in excess of 1/2 inch in cross section and 1/2 inch in 16 feet measured longitudinally shall be corrected by loosening, adding or removing the material, reshaping and recompacting by sprinkling and rolling.

(2) Density Tolerances. The City may accept the work provided not more than one out of the most recent three (3) density tests performed is outside the specified density and provided that the failing test is no more than three (3) pounds per cubic foot outside the specified density.

132.5. MEASUREMENT.

This Item will be measured as follows:

(1) General. Shrinkage or swellage factors will not be considered in determining the calculated quantities.

(2) Class 1. Embankment will be measured in its original, natural position, and the volume computed in cubic yards by the method of average end area.

(3) Class 2. Embankment will be measured by the cubic yard in vehicles as delivered on the road.

(4) Class 3. Is a Plans quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans, except as may be modified by General Note. If no adjustment of quantities is required, additional measurements or calculations will not be required.

132.6. PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Embankment", of the compaction method, type and class specified. This price shall be full compensation for furnishing embankment; for hauling; for placing, compacting, finishing and reworking; and for all labor, royalty, tools, equipment and incidentals necessary to complete the work.

When "Ordinary Compaction" is shown on the Plans, all sprinkling and rolling will not be paid for directly, but will be considered subsidiary to this Item, unless shown on the Plans.

When "Density Control" is shown on the Plans, all sprinkling and rolling will be considered subsidiary to this Item.

When subgrade is constructed under this project, correction of soft spots in the subgrade will be at the Contractor's expense.

CITY OF SAN ANGELO**ITEM 200****SUBGRADE PREPARATION****200.1 DESCRIPTION.**

This Item shall govern the scarifying, blading, and rolling of the subgrade to obtain uniform texture and density throughout the required depth as shown on the Plans.

200.2 TESTING.

The subgrade under areas to be paved shall be compacted to a minimum depth of **8 inches** and to a density of **not less than 95 percent for cohesive soils** or **100 percent for noncohesive soils** of the maximum density as determined by Test Method Tex-114-E / ASTM D 698. Noncohesive soils, for the purpose of determining compaction control, are those with a plasticity index (PI) of **less than 6**. The material to be compacted shall be **within +/- 2 percent of optimum moisture content** before rolled to obtain the prescribed compaction (except for expansive soils).

Field density determination will be made in accordance with Test Method Tex-115-E / ASTM D 2922 for field density and ASTM D 3017 for moisture content using a nuclear gage. If nuclear gages are to be used for density determination, the machines shall be calibrated in accordance with ASTM D 2922 using blocks of materials with densities that extend through a range representative of the density of the proposed embankment material.

Compaction is to be tested for density and moisture content acceptance as per Detail S-EE-1.

AASHTO T99 or T-180 (Moisture-Density) is required for soils that have more than 30 percent retained on the 3/4-inch sieve. The moisture-density relationship test procedures ASTM D 698 and D1557 are not applicable for materials with greater than 30 percent retained on the 3/4-inch sieve. A replacement procedure (ASTM D 4718) for the coarse material (greater than 3/4-inch) is used with ASTM methods but only until up to 30 percent is retained. Maximum density testing (ASTM D 4253) may be used but it also limits the material retained on the 1-1/2-inch sieve to 30 percent. The AASHTO T-99 and T-180 are similar to ASTM D 698 and D 1557, except they do not limit the replacement of the coarse material.

Stones or rock fragments larger than 4 inches (100 mm) in their greatest dimension will not be permitted in the top 6 inches (150 mm) of the subgrade. The finished grading operations, conforming to the typical cross section, shall be completed and maintained at least 1,000 feet (300 m) ahead of the paving operations or as directed by the City.

200.3 CONSTRUCTION METHODS.

The roadbed or parking lot subgrade, as case may be, shall be excavated and shaped in conformity with the typical sections shown on the Plans and to the lines and grades established by the City. All unstable or otherwise objectionable material shall be removed or otherwise broken off to a depth of not less than six (6) inches below the surface of the subgrade. Holes or depressions resulting from the removal of such material shall be backfilled with suitable material compacted in layers not to exceed six (6) inches. All soft and unstable material and other portions of the subgrade, which will not compact readily or serve the intended purpose, shall be removed as directed. No direct payment will be made for such removal.

The subgrade shall be scarified to the depth shown on the Plans, then bladed and compacted in the manner outlined in the following paragraph, "Finishing and Compaction". The surface of the subgrade shall be finished to line and grade as established, and be in conformity with the typical sections shown on the Plans. Any deviation in excess of one-half (1/2) inch in cross section and in a length of sixteen feet measured longitudinally shall be corrected by loosening, adding or removing material, reshaping and compacting by sprinkling and rolling. Material excavated in the preparation of the subgrade shall be disposed in a manner acceptable to the City.

200.4 FINISHING AND COMPACTION.

The subgrade course, including an area one foot back of the proposed curb line, or as the case may be, shall be sprinkled as required and rolled as directed until a uniform compaction and the required density is obtained. Compaction of the subgrade may be done using any of the rolling equipment acceptable to the City. Rolling shall continue until the subgrade has been compacted to the required testing minimums per Section 200.2.

Tests will be made at the times and locations selected by the City. Notification will be a minimum of 24 hours.

Rolling shall progress gradually from the sides to the center of the lane under construction by lapping uniformly each preceding tract by at least 12 inches.

After rolling and watering, the subgrade shall be checked by the use of string line or instrument and all portions that do not conform to the lines and grades as shown on the Plans shall be scarified for at least six (6) inches, corrected and recompacted to correct elevations.

Until the base course or pavement is placed, the subgrade shall be maintained free from cuts and depressions, in a smooth and compacted condition true to lines and grade and to the density requirements contained herein. All of the Contractor's hauling and other equipment used in such a way as to cause rutting and raveling of the subgrade shall either be removed from the work or suitable run-ways or other equivalent means shall be provided to prevent rutting.

The Contractor shall be responsible for maintaining and protecting the roadbed or the parking lot subgrade, as the case may be, for the entire length of the project.

During construction, grading of the subgrade shall be conducted so that berms of earth or other material do not substantially impede the flow of storm waters. Ditches and drains along the subgrade shall be maintained so as to drain effectively.

200.5. MEASUREMENT.

The subgrade preparation will be measured by the number of square yards of subgrade prepared and accepted by the Owner.

200.6. PAYMENT.

The amount of subgrade area measured as outlined under "Measurement" will be paid for at a unit price bid for this Item which will be full compensation for removing excess material, shaping, fine grading and compacting the subgrade; for furnishing and hauling all materials, blading and finishing and all labor, tools and incidentals necessary to complete the work.

CITY OF SAN ANGELO**ITEM 210****ROLLING (FLAT WHEEL)****210.1 DESCRIPTION.**

This Item shall govern for the compaction of embankment, flexible base, or surface treatments, by the operation of approved power rollers as herein specified and as directed by the City.

210.2 EQUIPMENT.

(1) **Embankments and Flexible Bases.** Power rollers shall be of the three-wheel, self-propelled type, weighing not less than ten (10) tons and shall provide compression on the rear wheels of not less than 325 pounds per linear inch of tire width. All wheels shall be flat; the rear wheels shall have a diameter of not less than 48 inches, and each shall have a tire width of not less than 20 inches.

210.3 CONSTRUCTION METHODS.

This work shall be done only when directed by the City. Sufficient rollers shall be provided to compact the material in a satisfactory manner. When operations are so isolated from one another that one roller unit cannot perform the required compaction satisfactorily, additional roller units shall be provided.

(1) **Embankments and flexible Bases.** The embankment layer or the base course shall be sprinkled if directed, and rolling with a power roller shall start longitudinally at the sides and proceed towards the center overlapping on successive trips by at least one-half (1/2) of the width of the rear wheel of the power roller. On superelevated curves, rolling shall begin at the low sides and progress toward the high sides. Alternate trips of the roller shall be slightly different in length. Rolling shall continue until discontinued by the City. The rollers, unless otherwise directed, shall be operated at a speed between two (2) and three (3) miles per hour.

(2) **Surface Treatments.** Rolling shall be done as called for in surface treatment items. The sequence of work shall be as specified for embankment layer or base course in accordance with Subarticle 210.3.(1) or as directed by the City. The operating speed shall be as directed by the City.

210.4 MEASUREMENT.

When shown on the Plans to be a pay item, this Item will be measured by the actual hours the power roller works as directed by the City.

210.5 PAYMENT.

The cost of furnishing and operating the equipment as prescribed by this Item will not be paid for directly but will be considered subsidiary to the various bid items of the contract, unless this Item is specified as a pay item in the contract.

When flat wheel rolling is specified as a pay item, the equipment furnished and operated in accordance with this item and measured and provided under "Measurement" will be paid for at the unit price bid for "Rolling (Flat Wheel)". This price shall be compensation for furnishing and operating all equipment; and for all labor, fuel, tools and incidentals necessary to satisfactorily perform the work.

CITY OF SAN ANGELO**ITEM 213****ROLLING (PNEUMATIC TIRE)****213.1 DESCRIPTION.**

This Item shall govern for the compaction of embankment, flexible base, surface treatments, or pavements by the operation of approved pneumatic tire rollers as herein specified and as directed by the City.

213.2 EQUIPMENT.

(1) **General.** When used on seal coats, asphaltic surface treatments and bituminous mixture pavements, the roller shall be self-propelled and equipped with smooth tread tires whether “Rolling (Light Pneumatic Tire)” or “Rolling (Medium Pneumatic Tire)” is specified on the plans. The roller shall be so constructed as to be capable of being operated in both a forward and a reverse direction.

When used on bituminous mixture pavements, the roller shall have suitable provisions for moistening the surface of the tires while operating.

Where turning is impractical or detrimental to the work, and when specifically directed by the City, the roller shall be capable of being operated in a forward or backward motion.

In lieu of the rolling equipment specified, the Contractor may, upon written permission from the City, operate other compacting equipment that will produce equivalent relative compaction in the same period of time as would be expected of the specified equipment, as determined by the City, its use shall be discontinued and the Contractor will be required to furnish the specified equipment.

(2) **The Light Pneumatic Tire Roller.** It shall consist of not less than nine pneumatic tired wheels, running on axles in such manner that the rear group of tires will cover the entire gap between adjacent tires of the forward group, and mounted in a rigid frame and provided with a loading platform or body suitable for ballast loading. The front axle shall be attached to the frame in such manner that the roller may be turned within a minimum circle. The pneumatic tire roller under working conditions shall have an effective rolling width of approximately sixty inches and shall be so designed that by ballast loading, the total load may be varied uniformly from 9,000 to 18,000 pounds. The roller shall be equipped with tires that will afford ground contact pressures of 45 pounds per square inch (p.s.i.) or more. The operating load and tire air pressure shall be within the range of the manufacturer’s charts or tabulations showing the contact areas and contact pressures for the full range of tire inflation pressures and for the full range of loadings for the particular tires furnished. The roller under working conditions shall provide a uniform compression under all wheels. Individual tire inflation pressures shall be within plus or minus five (5) p.s.i. of each other. The pneumatic tire roller shall be drawn by either a suitable crawler type tractor, a pneumatic tired tractor or a truck of adequate tractive effort, or may be of the self-propelled type,

and the roller, when drawn or propelled by either type of equipment, shall be considered a light pneumatic tire roller unit.

(3) The Medium Pneumatic Tire Roller (Type A). It shall consist of not less than seven (7) pneumatic tired wheels, running on axles in such manner that the rear group of tires will cover the entire gap between adjacent tires of the forward group, and mounted in a rigid frame and provided with a loading platform or body suitable for ballast loading. The front axle shall be attached to the frame in such manner that the roller may be turned within a minimum circle. The pneumatic tire roller under working conditions shall have an effective rolling width of approximately 84 inches and shall be so designed that by ballast loading the total load may be varied uniformly from 23,500 to 50,000 pounds. The roller shall be equipped with tires that will afford ground contact pressures to 80 p.s.i. or more. The operating load and tire air pressure shall be within the range of the manufacturer's chart as approved by the City. The roller under working conditions shall provide a uniform compression under all wheels. Individual tire inflation pressures shall be within plus or minus five (5) p.s.i. of each other.

The pneumatic tire roller shall be drawn by either a suitable crawler-type tractor, a pneumatic tired tractor or a truck of adequate tractive effort, or may be of the self-propelled type; and the roller, when drawn or propelled by either type of equipment, shall be considered a medium pneumatic tire roller unit. The power unit shall have adequate tractive effort to properly move the operating roller at variable uniform speeds up to approximately five (5) miles per hour.

(4) The Medium Pneumatic Tire Roller (Type B). It shall conform to the requirements for Medium Pneumatic Tire Roller, Type A as specified in Subarticle 213.2, except that the roller shall be equipped with tires that will afford ground contact pressures to 90 p.s.i. or more.

213.3 CONSTRUCTION METHODS.

This work shall be done only when directed by the City. The embankment layer or the base course shall be sprinkled if directed, and rolling with a pneumatic tire roller shall start longitudinally at the sides and proceed towards the center, overlapping on successive trips by at least one-half (1/2) of the width of the pneumatic tire roller. On superelevated curves, rolling shall begin at the low sides and progress toward the high sides. Alternate trips of the roller shall be slightly different in length. Rolling shall continue until discontinued by the City.

The light pneumatic tire roller shall be operated at speeds directed by the City which shall be between four (4) and twelve (12) miles per hour for asphalt surfacing work and between two (2) and six (6) miles per hour for all other work.

The medium pneumatic tire roller shall be operated at speeds as directed by the City.

Sufficient rollers shall be provided to compact the material in a satisfactory manner. When operations are so isolated from one another that one roller unit cannot perform the required compaction satisfactorily, additional roller units shall be provided.

213.4 MEASUREMENT.

When shown on the plans to be a pay item, this Item will be measured by the actual hours the pneumatic tire roller unit works as directed by the City.

213.5 PAYMENT.

The cost of furnishing and operating the equipment in accordance with this Item will not be paid for directly but will be considered subsidiary to the various bid items of the contract, unless this Item is specified as a pay item in the contract. When pneumatic rolling is specified as a pay item, the equipment furnished and operated in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Rolling (Light Pneumatic Tire)", "Rolling (Medium Pneumatic Tire) (Type A)" or "Rolling (Medium Pneumatic Tire) (Type B)". This price shall be full compensation for furnishing and operating all equipment, and for all labor, fuel, tools, and incidentals necessary to satisfactorily perform the work.

CITY OF SAN ANGELO**ITEM 216****ROLLING (PROOF)****216.1 DESCRIPTION.**

This Item shall govern for furnishing and operating heavy pneumatic tired compaction equipment for locating unstable areas of earthwork or base.

216.2 EQUIPMENT.

The proofrolling equipment shall consist of not less than four (4) pneumatic tired wheels, running on axles carrying not more than two (2) wheels, and mounted in a rigid frame and provided with loading platform or body suitable for ballast loading. All wheels shall be arranged so that they will carry approximately equal loads when operating on uneven surfaces.

The proofroller under working conditions shall have a rolling width of from eight (8) feet to ten (10) feet, and shall be so designed that, by ballast loading, the gross load may be varied uniformly from 25 tons to 50 tons. The tires shall be capable of operating under various loads with variable air pressure up to 150 pounds per square inch. Tires shall be practically full of liquid. (Tires shall be considered as being practically full when liquid will flow from the valve stem of a fully inflated tire with the stem in the uppermost position). The operating load and tire pressure shall be within the range of the manufacturer's chart as directed by the Engineer. The Contractor shall furnish the Engineer charts or tabulations showing the contact areas and contact pressures for the full range of tire inflation pressures and for the full range of loading for the particular tires furnished.

The proofroller shall be towed by a suitable crawler-type tractor or rubber-tired tractor of adequate tractive capacity, or may be of the self-propelled type. A proofroller unit shall consist of either a self-propelled roller or combination of roller and towing tractor.

There shall be a sufficient quantity of ballast available to load the equipment to a maximum gross weight of 50 tons.

Rubber tired tractive equipment shall be used on base courses and asphalt pavements. Other type tractive equipment may be used on embankment subgrade. The heavy pneumatic tire roller unit shall be capable of turning 180 degrees in the crown width or operating in forward and reverse modes.

In lieu of the rolling equipment specified, the Contractor may, upon written permission from the Engineer, operate other compacting equipment that will produce equivalent results in the same period of time as the specified equipment. If the substituted compaction equipment fails to produce the desired results within the same period of time as would be expected of the specified equipment, as determined by the Engineer, its use shall be discontinued.

216.3 CONSTRUCTION METHODS.

This work shall be done only when directed by the Engineer. The subgrade and/or base layer shall be proofrolled to locate unstable areas when directed by the Engineer.

Within the ranges set forth in Article 216.2, the load and tire inflation pressures shall be adjusted as directed by the Engineer. It is proposed to use a contact pressure corresponding as nearly as practical to the maximum supporting value of the earthwork or base. A minimum of two (2) coverages of the proofroller shall be offset by not greater than one (1) tire width. Rollers shall be operated at speeds directed by the Engineer, which shall be between two (2) and six (6) miles per hour.

Where the operation of the proofroller unit shows an area to be unstable or non-uniform, it shall be corrected in accordance with the applicable Item of Work.

216.4 MEASUREMENT.

This Item will be measured by the actual hours the heavy pneumatic tire proofroller unit works as directed by the Engineer.

216.5 PAYMENT.

The equipment furnished and operated in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Rolling (Proof)". This price shall be full compensation for furnishing and operating all equipment; for all labor, tools, fuel and incidentals necessary to satisfactorily perform the work.

Unless otherwise provided on the Plans, payment for reworking unstable or uniform areas, removing and replacing materials, addition of stabilizing materials, and all compaction and incidentals necessary to correct all irregularities will not be made directly but will be considered as subsidiary to the various bid items.

CITY OF SAN ANGELO

ITEM 247

FLEXIBLE BASE

247.1. DESCRIPTION.

This Item shall govern for the delivery, stockpiling and/or the construction of foundation or base courses as herein specified and in conformity with the typical sections and to the lines and grades shown on the Plans or established by the City.

247.2. MATERIALS.

The flexible base material shall be crushed or uncrushed as necessary to meet the requirements herein, and shall consist of durable coarse aggregate particles and binding materials.

(1) Physical Requirements.

(a) General. All types shall meet the physical requirements for the specified grade(s) as set forth in Table 1. Additives, such as, but not limited to, lime, cement or fly ash, shall not be used to alter the soil constants or strengths shown in Table 1, unless otherwise shown on the Plans approved by the City.

The flexible base shall be:

(b) Type A. Type A material shall be crushed stone produced from oversize quarried aggregate, sized by crushing and produced from a naturally occurring single source. Crushed gravel or uncrushed gravel shall not be acceptable for Type A material. No blending of sources and/or additive materials will be allowed in Type A material, unless noted on the Plans or as approved by the City.

(2) Testing: Testing of flexible base materials shall be in accordance with the following TxDOT standard laboratory test procedures:

Moisture Content	Tex-103-E / ASTM D 2216
Liquid Limit	Tex-104-E / ASTM D 4318
Plasticity Index.....	Tex-106-E / ASTM D 4318
Bar Linear Shrinkage	Tex-107-E, (Part II) / NA
Sieve Analysis.....	Tex-110-E / ASTM D 422
Moisture-Density Determination	Tex-113-E / ASTM D 1557
Roadway Density	Tex-115-E / ASTM D 2922 and ASTM D 3017
Wet Ball Mill	Tex-116-E / NA
Triaxial Tests	Tex-117-E, (Part I or II as selected by the City) / NA
Particle Count.....	Tex-460-A, Part I / ASTM D 5821

Samples for testing the base material for triaxial class, soil constants, gradation, and wet ball mill will be taken prior to the compaction operations.

TABLE 1
MATERIAL REQUIREMENTS

Property	Test Method	Grade 2
Master gradation sieve Size (% retained)		
2-1/2 in.	Tex-110-E / ASTM D 422	0
1-3/4 in		0 - 10
7/8 in		---
3/8 in		---
No. 4		45 - 75
No. 40		60 - 85
Liquid Limit, % max. ¹		Tex-104-E / ASTM D 4318
Plasticity Index, max ¹	Tex-106-E / ASTM D 4318	12
Plasticity Index, max ¹		As shown on plans
Wet ball mill, % max. ²	Tex-116-E / NA	45
Wet ball mill, % max. increase passing the No. 40 sieve		20
Classification ³	Tex-117-E / NA	1.1 – 2.3
Minimum compressive strength ³ , psi		
lateral pressure 0 psi		35
lateral pressure 15 psi		175

1. Determine plastic index in accordance with Tex-107-E / NA (linear shrinkage) when liquid limit is unattainable as defined in Tex-104-E / ASTM D 4318.
2. When a soundness value is required by the plans, test material in accordance with Tex-411-A / ASTM C 88.
3. Meet both the classification and the minimum compressive strength, unless otherwise shown on the plans.

(3) Tolerances. Unless otherwise shown on the Plans, the limits establishing reasonably close conformity with the specified gradation and plasticity index are defined by the following:

(a) Gradation. The City may accept the material, providing not more than one out of the most recent five (5) consecutive gradation tests performed are outside the specified gradation and plasticity index are defined by the following:

(b) Plasticity Index. The City may accept the material providing not more than one (1) out of the most recent five (5) consecutive plasticity index samples tested are outside the specified limit by no more than two (2) percentage points.

(4) Material Sources. The flexible base material shall be furnished by the Contractor. When a non-commercial source is utilized, it shall be opened in such manner as to immediately expose the vertical faces of all the various strata of acceptable material. Unless otherwise approved by the City, the material shall be secured and processed by successive vertical cuts extending through all of the exposed strata.

Unless otherwise shown on the Plans, the flexible base material shall be temporarily stockpiled prior to delivery to the roadway. Unless otherwise shown on the Plans, the stockpile shall not be less than ten (10) feet in height and shall be made up of layers not greater than two (2) feet in thickness. After a sufficient stockpile has been constructed, the Contractor may proceed with loading from the stockpile for delivery. In loading from the stockpile for delivery, the material shall be loaded by making successive vertical cuts through the entire depth of the stockpile.

When temporary stockpiles are to be tested for acceptance prior to delivery to its intended use, any stockpile that has been sampled and accepted shall not have material added or removed unless otherwise approved by the City. The Contractor will be charged for additional sampling and testing required as a result of material being removed from a previously approved stockpile without the approval of the City. Such charges will be deducted from the Contractor's estimates.

247.3. CONSTRUCTION METHODS.

(1) Complete in Place:

(a) Preparation of Subgrade or Existing Roadbed. Prior to delivery of the base material, the subgrade or existing roadbed shall be shaped to conform to the typical sections, shown on the Plans or established by the City. This work shall be done in accordance with the provision of the applicable bid items.

When shown on the Plans and directed by the City, the Contractor shall proof roll the roadbed in accordance with Item 216, "Rolling (Proof)". Soft spots shall be corrected as directed by the City.

(b) First Course. It shall be the responsibility of the Contractor to deliver the required amount of base material to each 100-foot station. Base material shall be spread uniformly and shaped the same day as delivered. In the event inclement weather or other unforeseen circumstances render this impractical, the material shall be shaped as soon as practical.

Prior to compacting the flexible base, the flexible base material shall be bladed and shaped to conform to the typical sections as shown on the Plans. All areas of segregated coarse or fine material shall be corrected or removed and replaced with well-graded material, as directed by the City and at the Contractor's expense.

The Contractor shall sprinkle for dust control as directed by the City.

(c) Succeeding or Finish Courses. Construction methods shall be the same as required for the first course. Throughout this entire operation, the shape of each course shall be maintained by blading. Upon completion, the surface shall be smooth and in conformity with the typical section as shown on the Plans and the established lines and grades. Prior to placing the surfacing on the completed base, the base shall be cured to the extent directed by the City.

(d) Compaction Method. The flexible base shall be compacted by "Density Control" as shown on the Plans. Water used for compaction shall conform to the same water source requirements of Item 160.2(2).

The flexible base shall be sprinkled as required and compacted to the extent necessary to provide not less than 95 percent density as determined by TxDOT Test Method Tex-113-E / ASTM D 1557, unless otherwise shown on the Plans. After each section of flexible base is completed, tests as necessary will be made by the City in accordance with TxDOT Test Method Tex-115-E / ASTM D 2922 and ASTM D 3017. When the material fails to meet the density requirements, or it loses the required stability, density or finish before the next course is placed or the project is completed, it shall be reworked and retested in accordance with Section 247.3.(1)(e).

(e) Reworking a Section. Should the base course, due to any reason or cause, lose the required stability, density or finish before the surfacing is complete, it shall be reworked, recompacted and refinished at the sole expense of the Contractor.

(f) Tolerances. Tolerances shall conform to the following:

(i) Density Tolerances. The City may accept the work providing not more than one out of the most recent five consecutive density tests performed is below the specified density, and providing that the failing test is no more than three pounds per cubic foot below the specified density.

(ii) Grade Tolerances. In areas on which surfacing is to be placed, any deviation in excess of 1/4 inch in cross section or 1/4 inch in a length of 16 feet measured longitudinally shall be corrected by loosening, adding or removing material, reshaping and recompacting by sprinkling and rolling.

(g) Thickness Measurement. When the measurement is by the square yard, the flexible base will be measured for depth in units of 4,000 square yards, or fraction thereof. The measurements will be at location(s) determined by the City and performed in accordance with Test Method Tex-140-E. In any unit where flexible base is deficient by more than 1/2 inch in thickness, the deficiency shall be corrected by scarifying, adding material as required, reshaping, recompacting and refishing at the Contractor's expense.

(2) Roadway Delivery. It shall be the responsibility of the Contractor to deliver the required amount of base material to each 100-foot station. All processing or manipulations will be in accordance with the applicable bid items.

(3) Stockpile Delivery. It shall be the responsibility of the Contractor to prepare the stockpile site, to provide and deliver the required amount of base material to the designated stockpile site and to construct the stockpile. Unless otherwise shown on the Plans, the stockpile shall not be

less than ten (10) feet in height and shall be made up of layers not to exceed two (2) feet in thickness.

247.4. MEASUREMENT.

This Item will be measured by either Measurement Class 1, 2, 3, 4, or 5 as shown on the Plans:

(1) Measurement Class 1. Measurement will be by the cubic yard in vehicles of uniform capacity.

(2) Measurement Class 2. Measurement will be by the ton of 2,000 pounds dry weight in vehicles as delivered. A set of standard platform truck scales conforming to the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", shall be furnished by the Contractor and placed at a location approved by the City. When the material is weighed during mixing or batching, re-weighing will not be necessary. The dry weight will be determined by deducting the weight of the moisture in the material at the time of weighing from the gross weight of the material. The moisture in the material will be determined in accordance with Test Method Tex-103-E / ASTM D 2216 at least once each day and more often if conditions warrant.

(3) Measurement Class 3. Measurement will be by the cubic yard in the final stockpile position. The volume of flexible base will be computed in place between the natural ground and the top of the stockpile by the method of average end areas.

(4) Measurement Class 4. Measurement will be by the cubic yard in the completed and accepted final position. The volume of base course will be computed in place between the original subgrade or subbase surfaces, and the lines, grades and slopes of the accepted base course as shown on the Plans by the method of average end areas.

Measurement Class 4 is plan quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans, except as may be modified by General Conditions of Contract Documents. If no adjustment is required, additional measurements or calculations will not be required. No payment will be made for thickness or width exceeding that shown on the typical section or provided on the Plans.

(5) Measurement Class 5. Measurement will be by the square yard of surface area in the completed and accepted position. The surface area of the base course will be based on the width of flexible base as shown on the Plans.

Measurement Class 5 is a plans quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans. If no adjustment is required, additional measurements or calculations will not be required. No payment will be made for thickness or width exceeding that shown on the typical section or provided on the Plans.

247.5. PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Flexible Base (Complete in Place)" of the type, grade and measurement class specified; for "Flexible Base (Roadway Delivery)" of the type, grade and measurement class specified; and for "Flexible Base (Stockpile Delivery)" of the type, grade and measurement class specified. This price shall be full compensation for securing and furnishing all materials, including royalty and freight involved; for furnishing scales and labor involved in weighing the material when required; for loosening, blasting, excavating, screening, crushing and temporary stockpiling when required; for loading all materials; for all hauling and delivering and for all manipulations; sprinkling; for rolling, except for proof rolling; sprinkling for dust control, for labor, tools and incidentals necessary to complete the work except as follows:

When the Plans specify "Flexible Base (Complete in Place)", the unit price bid shall be full compensation for shaping and fine grading the roadbed and for spreading, mixing, blading, compacting, shaping, finishing, and curing the base material.

When the Plans specify "Flexible Base (Roadway Delivery)", the unit price bid will not include processing at the roadway. Measurement will be only by Measurement Class 1 or 2.

When the Plans specify "Flexible Base (Stockpile Delivery)", the unit price bid also will be full compensation for preparing the stockpile area and for spreading and shaping the material in the stockpile. Measurement will be only by Measurement Class 1, 2, or 3.

When proofrolling is shown on the Plans, and when directed by the City, it will be considered subsidiary to the various bid items.

When subgrade is constructed under this project, correction of soft spots will be at the Contractor's expense.

CITY OF SAN ANGELO

ITEM 300

ASPHALTS, OILS, AND EMULSIONS

300.1. DESCRIPTION.

This Item shall for govern providing asphalt cements, cutback and emulsified asphalts, performance-graded asphalt binders, and other miscellaneous asphalt materials as specified on the plans.

300.2. MATERIALS.

Provide asphalt materials that meet the stated requirements when tested in accordance with the referenced Department, AASHTO, and ASTM test methods. Acronyms used in this Item are defined in Table 1

**TABLE 1
ACRONYMS**

Acronym	Definition
Test Procedure Designations	
Tex T or R D	Department AASHTO ASTM
Polymer Modifier Designations	
P SBR or L SBS TR	Polymer-modified Styrene-butadiene rubber (latex) Styrene- butadiene-styrene block co-polymer Tire rubber (from ambient temperature Grinding of truck and passenger tires)
AC	Asphalt cement
AE	Asphalt emulsion
AE-P	Asphalt emulsion prime
A-R	Asphalt-rubber
C	Cationic
EAP&T	Emulsified asphalt prime and tack
H-suffix	Harder residue (lower penetration)
HF	High float
MC	Medium-curing
PCE	Prime, cure and erosion control

TABLE 1 (Continued)
ACRONYMS

Acronym	Definition
MS	Medium-setting
PG	Performance grade
RC	Rapid-curing
RS	Rapid-setting
S-suffix	Stockpile usage
SCM	Special cutback material
SS	Slow-setting

(1) **Asphalt Cement.** Asphalt cement must be homogeneous, water-free, and nonfoaming when heated to 347 °F, and must meet Table 2 requirements.

TABLE 2
ASPHALT CEMENT

Property	Test Procedure	Viscosity Grade									
		AC-0.6		AC-1.5		AC-3		AC-5		AC-10	
		Min	Max	Min	Max	Min	Max	Min	Max	Min	Max
Viscosity 140 °F, poise	T 202	40	80	100	200	250	350	400	600	800	1,200
275 °F, poise		0.4	-	0.7	-	1.1	-	1.4	-	1.9	-
Penetration, 77 °F, 100g, 5 sec.	T 49	350	-	250	-	210	-	135	-	85	-
Flash point, C.O.C., °F	T 48	425	-	425	-	425	-	425	-	450	-
Solubility in trichloroethylene, %	T 44	99.0	-	99.0	-	99.0	-	99.0	-	99.0	-
Spot test	Tex-509-C	Neg.		Neg.		Neg.		Neg.		Neg.	
Tests on residue from Thin-Film Oven Test:	T 179 T 202 T 51										
Viscosity, 140 °F, poise		-	180	-	450	-	900	-	1,500	-	3,000
Ductility ¹ , 77 °F 5 cm/min., cm		100	-	100	-	100	-	100	-	100	-

1. If AC-0.6 or AC 1-5 ductility at 77 °F is less than 100 cm, material is acceptable if ductility at 60 °F is more than 100 cm.

(2) **Polymer-Modified Asphalt Cement.** Polymer-modified asphalt cement must be smooth and homogeneous, and comply with the requirements of Table 3. If requested, supply samples of the base asphalt cement and polymer additives.

TABLE 3
POLYMER-MODIFIED ASPHALT CEMENT

Property	Test Procedure	Polymer-Modified Viscosity Grade							
		AC-5 w/2% SBR		AC-10 w/2% SBR		AC-15P		AC-20-5TR	
		Min	Max	Min	Max	Min	Max	Min	Max
Polymer		SBR		SBR		SBS		TR	
Polymer content, % (solid basis)	Tex-533-C	2.0	-	2.0	-	3.0	-	5.0	-
Dynamic shear, $G^*/\sin.\delta$, 64°C, 10 rad/s, kPa	T 315	-	-	-	-	-	-	1.0	-
Viscosity									
140°F, poise	T 202	700	-	1,300	-	1,500	-	2,000	-
275°F, poise	T 202	-	7.0	-	80	-	8.0	-	10.0
Penetration, 77°F, 100g, 5 sec.	T 49	120	-	80	-	100	150	75	115
Ductility, 5cm/min., 39.2°F, cm	T 51	70	-	60	-	-	-	-	-
Elastic recovery, 50°F, %	Tex-539-C	-	-	-	-	55	-	55	-
Softening point, °F	T 53	-	-	-	-	-	-	120	-
Polymer separation, 48 hr.	Tex-540-C	None		None		None		None	
Flash point, C.O.C., °F	T 48	425	-	425	-	425	-	425	-
Tests on residue from Thin-Film Over Test:									
Retained penetration ratio, 77°F	T 179 T 49	-	-	-	-	0.60	1.00	0.60	1.00
Tests on residue from RTFOT aging and pressure aging:									
Creep stiffness									
S, 18°C, MPa	Tex-541-C and R 28	-	-	-	-	-	-	-	300
m-value, 18°C	T 313	-	-	-	-	-	-	0.300	-

(3) **Cutback Asphalt.** Cutback asphalt must meet the requirements of Tables 4, 5, and 6 for the specified type and grade. If requested, supply samples of the base asphalt cement and polymer additives.

TABLE 4
RAPID-CURING CUTBACK ASPHALT

Property	Test Procedure	Type-Grade					
		RC-250		RC-800		RC-3000	
		Min	Max	Min	Max	Min	Max
Kinematic viscosity, 140°F, cSt	T 201	250	400	800	1,600	3,000	6,000
Water, %	T 55	-	0.2	-	0.2	-	0.2
Flash point, T.O.C., °F	T 79	80	-	80	-	80	-
Distillation test: Distillate, percentage by volume of total distillate	T 78						
to 680°F		40	75	35	70	20	55
to 437°F		65	90	55	85	45	75
to 500°F		85	-	80	-	70	-
to 600°F		70	-	75	-	82	-
Residue from distillation, V %							
Tests on distillation residue:							
Penetration, 100 g, 5 sec. 77°F	T 49	80	120	80	120	80	120
Ductility, 5 cm/min., 77°F, cm	T 51	100	-	100	-	100	-
Solubility in trichloroethylene, %	T 44	99.0	-	99.0	-	99.0	-
Spot test	Tex-509-C	Neg.		Neg.		Neg.	

TABLE 5
MEDIUM-CURING CUTBACK ASPHALT

Property	Test Procedure	Type-Grade							
		MC-30		MC-250		MC-800		MC-3000	
		Min	Max	Min	Max	Min	Max	Min	Max
Kinematic viscosity, 140°F, cSt	T 201	30	60	250	500	800	1,600	3,000	6,000
Water, %	T 55	-	0.2	-	0.2	-	0.2	-	0.2
Flash point, T.O.C., °F	T 79	100	-	150	-	150	-	150	-
Distillation test: Distillate, percentage by volume of total distillate	T 78								
to 680°F		-	25	-	10	-	-	-	-
to 437°F		40	70	15	55	-	35	-	15
to 500°F		75	93	60	87	45	80	15	75
to 600°F		50	-	67	-	75	-	80	-
Residue from distillation, V %									
Tests on distillation residue:									
Penetration, 100g, 5 sec., 77°F	T 49	120	250	120	250	120	250	120	250
Ductility, 5cm/min., 77°F, cm ¹	T 51	100	-	100	-	100	-	100	-
Solubility in trichloroethylene, %	T 44	99.0	-	99.0	-	99.0	-	99.0	-
Spot test	Tex-509-C	Neg.		Neg.		Neg.		Neg.	

TABLE 6
SPECIAL-USE CUTBACK ASPHALT

Property	Test Procedure	Type-Grade					
		MC-2400L		SCM I		SCM II	
		Min	Max	Min	Max	Min	Max
Kinematic viscosity, 140°F, cSt	T 201	2,400	4,800	500	1,000	1,000	2,000
Water, %	T 55	-	0.2	-	0.2	-	0.2
Flash point, T.O.C., °F	T 79	150	-	175	-	175	-
Distillation test:	T 78						
Distillate, percentage by volume of total distillate							
to 680°F		-	-	-	-	-	-
to 437°F		-	35	-	0.5	-	0.5
to 500°F		35	80	20	60	15	50
to 600°F		78	-	76	-	82	-
Residue from distillation, vol %							
Tests on distillation residue:							
Polymer		SBR		-		-	
Polymer content, % (solids basis)	Tex-533-C	2.0	-	-	-	-	-
Penetration, 100 g, 5 sec. 77°F	T 49	150	300	180	-	180	-
Ductility, 5 cm/min., 39.2°F, cm	T 51	50	-	-	-	-	-
Solubility in trichloroethylene, %	T 44	99.0	-	99.0	-	99.0	-

(4) Emulsified Asphalt. Emulsified asphalt must be homogeneous, not separate after thorough mixing, and meet the requirements for the specified type and grade in Table 7, 8, 9, and 10.

TABLE 7
EMULSIFIED ASPHALT

Property	Test Procedure	Type-Grade									
		Rapid-Setting		Medium-Setting				Slow-Setting			
		HFRS-2		MS-2		AES-300		SS-1		SS-1H	
		Min	Max	Min	Max	Min	Max	Min	Max	Min	Max
Viscosity, Saybolt Furol 77°F, sec. 122°F, sec.	T 72	-	-	-	-	75	400	20	100	20	100
Sieve test, %	T 59	-	0.1	-	0.1	-	0.1	-	0.1	-	0.1
Miscibility	T 59	-		-		-		Pass		Pass	
Cement mixing, %	T 59	-	-	-	-	-	-	-	2.0	-	2.0
Coating ability and water resistance: dry aggregate/after spray wet aggregate/after spray	T 59	-		-		Good/Fair Fair/Fair		-		-	
Demulsibility, 35 ml of 0.02 N CaCl ₂ , %	T 59	50	-	-	30	-	-	-	-	-	-
Storage stability, 1 day, %	T 59	-	1	-	1	-	1	-	1	-	1
Freezing test, 3 cycles ¹	T 59	-		Pass		-		Pass		Pass	
Distillation test: Residue by distillation, % by wt. Oil distillate, % by volume of emulsion	T 59	65	-	65	-	65	-	60	-	60	-
		-	0.5	-	0.5	-	5	-	0.5	-	0.5
Tests on residue from distillation: Penetration, 77°F, 100 g, 5 sec. Solubility in trichloroethylene, % Ductility, 77°F, 5 cm/min., cm Float test, 140°F, sec.	T 49	100	140	120	160	300	-	120	160	70	100
	T 44	97.5	-	97.5	-	97.5	-	97.5	-	97.5	-
	T 51	100	-	100	-	-	-	100	-	80	-
	T50	1,200	-	-	-	1,200	-	-	-	-	-

1. Applies only when the City designates for winter use

TABLE 8
EMULSIFIED ASPHALT

Property	Test Procedure	Type-Grade											
		Rapid-Setting				Medium-Setting				Slow-Setting			
		CRS-2		CRS-2H		CMS-2		CMS-2S		CSS-1		CSS-1H	
		Min	Max	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max
Viscosity, Saybolt Furol 77°F, sec. 122°F, sec.	T 72	-	-	-	-	-	-	-	-	20	100	20	100
Sieve Test, %	T 59	-	0.1	-	0.1	-	0.1	-	0.1	-	0.1	-	0.1
Cement mixing, %	T 59	-	-	-	-	-	-	-	-	-	2.0	-	2.0
Coating ability and water resistance: dry aggregate/after spray wet aggregate/after spray	T 59	-	-	-	-	Good/Fair Fair/Fair	Good/Fair Fair/Fair	-	-	-	-	-	-
Demulsibility, 35 ml of 0.8% sodium dioctyl sulfosuccinate, %	T 59	70	-	70	-	-	-	-	-	-	-	-	-
Storage stability, 1 day, %	T 59	-	1	-	1	-	1	-	1	-	1	-	1
Particle charge	T 59	Positive		Positive		Positive		Positive		Positive		Positive	
Distillation test: Residue by distillation, % by wt. Oil distillate, % by volume of emulsion	T 59	65	-	65	-	65	-	65	-	60	-	60	-
		-	0.5	-	0.5	-	7	-	5	-	0.5	-	0.5
Tests on residue from distillation: Penetration, 77°F, 100g, 5 sec.	T 49	120	160	70	110	120	200	300	-	120	160	70	110
Solubility in trichloroethylene, %	T 44	97.5	-	97.5	-	97.5	-	97.5	-	97.5	-	97.5	-
Ductility, 77°F, 5 cm/min., cm	T 51	100	-	80	-	100	-	-	-	100	-	80	-

TABLE 9
POLYMER-MODIFIED EMULSIFIED ASPHALT

Property	Test Procedure	Type-Grade											
		Rapid-Setting				Medium-Setting				Slow-Setting			
		RS-1P		HFRS-2P		AES-150P		AES-300P		AES-300S		SS-1P	
		Min	Max	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max
Viscosity, Saybolt Furol 77°F, sec. 122°F, sec.	T 72	-	-	-	-	75	400	75	400	75	400	30	100
		50	200	150	400	-	-	-	-	-	-	-	-
Miscibility	T 59	-	-	-	-	-	-	-	-	-	-	-	Pass
Coating ability and water resistance: dry aggregate/after spray wet aggregate/after spray	T 59	-	-	-	-	Good/Fair	Good/Fair	Good/Fair	Good/Fair	Good/Fair	Good/Fair	-	-
		-	-	-	-	Fair/Fair	Fair/Fair	Fair/Fair	Fair/Fair	Fair/Fair	Fair/Fair	-	-
Demulsibility, 35 ml of 0.02 N CaCl ₂ , %	T 59	60	-	50	-	-	-	-	-	-	-	-	-
Storage stability, 1 day,	T 59	-	1	-	1	-	1	-	1	-	1	-	1
Breaking index, g	Tex-542-	-	80	-	-	-	-	-	-	-	-	-	-
Distillation test: ¹ Residue by distillation, % by wt, Oil distillate, % by volume of emulsion	T 59	65	-	65	-	65	-	65	-	65	-	60	-
		-	3	-	0.5	-	3	-	5	-	7	-	0.5
Tests on residue from distillation: Polymer content, wt. % (solids basis) Penetration, 77°F, 100 g, 5sec Solubility in trichloroethylene, % Viscosity, 140°F, poise Float test, 140°F, poise Ductility ² , 39.2°F, 5 cm/min., cm Elastic recovery ² , 50°F, %	Tex-533-C	-	-	3.0	-	-	-	-	-	-	-	3.0	-
	T 49	-	-	-	-	-	-	-	-	-	-	-	-
	T 44	225	300	90	140	150	300	300	-	300	-	100	140
	T 202	97	-	97	-	97	-	97	-	97	-	97	-
	T 50	-	-	1,500	-	-	-	-	-	-	-	1,300	-
	T 51	-	-	1,200	-	1,200	-	1,200	-	1,200	-	-	-
	Tex-539-C	-	-	50	-	-	-	-	-	-	-	50	-
Tests on RTFO curing of distillation residue	Tex-541-C	-	-	-	-	-	-	-	-	-	-	-	-
Elastic recovery, 50°F, %	Tex-539-C	-	-	-	-	50	-	50	-	30	-	-	-

1. Exception to T 59: Bring the temperature on the lower thermometer slowly to 350°F ± 10°F. Maintain at this temperature for 20 min. Complete total distillation in 60 ± 5 min. from the first application of heat.
2. HFRS-2P must meet one of either the ductility or elastic recovery requirements.

TABLE 10
POLYMER-MODIFIED CATIONIC EMULSIFIED ASPHALT

Property	Test Procedure	Type-Grade					
		CRS-1P		CRS-2P		CSS-1P	
		Min	Max	Min	Max	Min	Max
Viscosity, Saybolt Furol 77°F, sec.	T 72	-	-	-	-	20	100
122°F, sec.		50	150	150	400	-	-
Sieve test, %	T 59	-	0.1	-	0.1	-	0.1
Demulsibility, 35 ml of 0.8% sodium	T 59	60	-	70	-	-	-
Storage stability, 1 day %	T 59	-	1	-	1	-	1
Breaking index, g	Tex-542-C	-	80	-	-	-	-
Particle charge	T 59	Positive		Positive		Positive	
Distillation test: ¹	T 59						
Residue by distillation, % by wt		65	-	65	-	62	-
Oil distillate, % by vol of emulsion		-	3	-	0.5	-	0.5
Tests on residue from distillation:							
Polymer content, wt. % (solids basis)	Tex-533-C	-	-	3.0	-	3.0	-
Penetration, 77°F, 100 g, 5 sec.	T 49	225	300	90	150	55	90
Viscosity, 140°F, poise	T 202	-	-	1,300	-	-	-
Solubility in trichloroethylene, %	T 44	97.0	-	97.0	-	97.0	-
Softening point, °F	T 53	-	-	-	-	135	-
Ductility, 77°F, 5 cm/min., cm	T 51	-	-	-	-	70	-
Ductility ² , 39.2°F, 5 cm/min., cm	T 51	-	-	50	-	-	-
Elastic recovery ² , 50°F, %	Tex-539-C	45	-	55	-	-	-

1. Exception to T 59: Bring the temperature on the lower thermometer slowly to 350°F ±0°F. Maintain at this temperature for 20 min. Complete total distillation in 60 ± 5 min. from the first application of heat.

2. CRS-2P must meet one of either the ductility or elastic recovery requirements.

(5) Specialty Emulsions. Specialty emulsions may be either asphalt-based or resin-based and must meet the requirements of Table 11.

TABLE 11
SPECIALTY EMULSIONS

Property	Test Procedure	Type-Grade					
		Medium-Setting				Slow-Setting	
		AE-P		EAP&T		PCE ¹	
		Min	Max	Min	Max	Min	Max
Viscosity, Saybolt Furol 77°F, sec.	T 72	-	-	-	-	10	100
122°F, sec.		15	150	-	-	-	-
Sieve test, %	T 59	1	0.1	-	0.1	-	0.1
Miscibility ^{o2}	T 59	0		Pass		Pass	
Demulsibility, 35 ml of 0.10 N CaCl ₂ %	T 59	-	70	-	-	-	-
Storage stability, 1 day, %	T 59	-	1	-	1	-	-
Particle size ⁵ , % by volume, < 2.5	Tex-238-	-	-	90	-	90	-
Asphalt emulsion distillation to 500°F followed by Cutback asphalt distillation of residue to 680°F: Residue after both distillations, % by wt.	T 59 & T 78	40	-	-	-	-	-
Total oil distillate from both distillations, % by vol of emulsion		25	40	-	-	-	-
Residue by distillation, % by wt.	T 59	-	-	60	-	-	-
Residue by evaporation ⁴ , % by wt.	T 59	-	-	-	-	60	-
Tests on residue after all distillation(s):							
Viscosity, 140°F, poise	T 202	-	-	800	-	-	-
Kinematic viscosity ⁵ , 140°F, cSt	T 201	-	-	-	-	100	350
Flash point C.O.C., °F	T 48	-	-	-	-	400	-
Solubility in trichloroethylene, %	T 44	97.5	-	-	-	-	-
Float test, 122°F, sec.	T 50	50	200	-	-	-	-

1. Supply with each shipment of PCE:
 - a) a copy of a lab report from an approved analytical lab, signed by a lab official, indicating the PCE formulation does not meet any characteristics of a Resource Conservation Recovery Act (RCRA) hazardous waste;
 - b) a certification from the producer that the formulation supplied does not differ from the one tested and that no listed RCRA hazardous wastes on PCBs have been mixed with the product; and
 - c) a Material Safety Data Sheet.
2. Exception to T 59; In dilution, use 350 ml of distilled or deionized water and a 1,000-ml beaker.
3. Use Tex-238-F, beginning at “Particle Size Analysis by Laser Diffraction,” with distilled or deionized water as a medium and no dispersant, or use another approved method.
4. Exception to T 59: Leave sample in the oven until foaming ceases, then cool and weigh.
5. PCE must meet either the kinematic viscosity requirement or the particle size requirement.

(6) **Recycling Agent.** Recycling agent and emulsified recycling agent must meet the requirements in Table 12. Additionally, recycling agent and residue from emulsified recycling agent, when added in the specified proportions to the recycled asphalt, must meet the properties specified on the plans.

TABLE 12
RECYCLING AGENT AND EMULSIFIED RECYCLING AGENT

Property	Test Procedure	Recycling Agent		Emulsified Recycling Agent	
		Min	Max	Min	Max
Viscosity, Saybolt Furol, 77°F, sec.	T 72	-	-	15	100
Sieve test, %	T 59	-	-	-	0.1
Miscibility ¹	T 59	-		No coagulation	
Residue by evaporation ² , % by wt.	T 59	-	-	60	-
Tests on recycling agent or residue from evaporation:					
Flash point, C.O.C., °F	T 48	400	-	400	-
Kinematic viscosity, 140°F, cSt	T 201	75	200	75	200
275°F, cSt		-	10.0	-	10.0

1. Exception to T 59: Use 0.02 N CaCl₂ solution in place of water.
2. Exception to T 59: Maintain sample at 300°F until foaming ceases, then cool and weigh.

(7) **Crumb Rubber Modifier.** Crumb rubber modifier (CRM) consists of automobile and truck tires processed by ambient temperature grinding.

CRM must be:

- free from contaminants including fabric, metal, and mineral and other nonrubber substance;
- free-flowing; and
- nonfoaming when added to hot asphalt binder.

When tested in accordance with Tex-200-F, Part I, using at 50-g sample, the rubber gradation must meet the requirements of the grades in Table 13.

TABLE 13
CRM GRADATIONS

Sieve Size (% Passing)	Grade A		Grade B		Grade C		Grade D	Grade E
	Min	Max	Min	Max	Min	Max		
#8	100	-	-	-	-	-	As shown on the plans	As approved
#10	95	100	100	-	-	-		
#16	-	-	70	100	100	-		
#30	-	-	25	60	90	100		
#40	-	-	-	-	45	100		
#50	0	10	-	-	-	-		
#200	-	-	0	5	-	-		

(8) **Crack Sealer.** Polymer modified asphalt-emulsion crack sealer must meet the requirements of Table 14. Rubber-asphalt crack sealer must meet the requirements of Table 15.

TABLE 14
POLYMER-MODIFIED ASPHALT EMULSION CRACK SEALER

Property	Test Procedure	Min	Max
Retational viscosity, 77°F, cP	D 2196, Method A	10,000	25,000
Sieve test, %	T 59	-	0.1
Storage stability, 1 day, %	T 59	-	1
Evaporation Residue by evaporation, % by wt.	Tex-543-C	65	-
Tests on residue from evaporation:			
Penetration, 77°F, 100 g, 5 sec.	T 49	35	75
Softening point, °F	T 53	140	-
Ductility, 39.2°F, 5 cm/min., cm	T 51	100	-

TABLE 15
RUBBER-ASPHALT CRACK SEALER

Property	Test Procedure	Class A		Class B	
		Min	Max	Min	Max
CRM content, Grade A or B, % by wt.	Tex-544-C	22	26	-	-
CRM content, Grade B, % by wt.	Tex-544-C	-	-	13	17
Virgin rubber content ¹ , % by wt.		-	-	2	-
Flash point ² , COC, °F	T 48	400	-	400	-
Penetration ³ , 77°F, 150 g, 5 sec.	T 49	30	50	30	50
Penetration ³ , 32°F, 200 g, 60 sec.	T 49	12	-	12	-
Softening point, °F	T 53	-	-	170	-
Bond ⁴	D5329	-		Pass	

1. Provide certification that the min. % virgin rubber was added.
2. Before passing the test flame over the cup, agitate the sealing compound with a 3/8-to 1/2-in. (9.5-to 12.7-mm) wide, square-end metal spatula in a manner so as to bring the material on the bottom of the cup to the surface, i.e., turn the material over. Start at one side of the thermometer, move around to the other, and then return to the starting point using 8 to 10 rapid circular strokes. Accomplish agitation in 3 to 4 sec. Pass the test flame over the cup immediately after stirring is completed.
3. Exception to T 49: Substitute the cone specified in ASTM D 217 for the penetration needle.
4. No crack in the crack sealing materials or break in the bond between the sealer and the mortar blocks over 1/4 in. deep for any specimen after completion of the test.

(9) Asphalt-Rubber Binders. Asphalt-rubber (A-R) binders are mixtures of asphalt binder and CRM, which have been reacted at elevated temperatures. The A-R binders meet D 6114 and contain a minimum of 15% CRM by weight. Types I or II, containing CRM Grade C, are used for hot mixed aggregate mixtures. Types II and III, containing CRM Grade B, are used for surface treatment binder. Table 16 describes required binder properties.

TABLE 16
A-R BINDERS

Property	Test Procedure	Binder Type					
		Type I		Type II		Type III	
		Min	Max	Min	Max	Min	Max
Apparent viscosity, 347°F, cP	D 2196, Method A	1,500	5,000	1,500	5,000	1,500	5,000
Penetration, 77°F, 100 g, 5 sec.	T 49	25	75	25	75	50	100
Penetration, 39.2°F, 200 g, 60 sec.	T 49	10	-	15	-	25	-
Softening Point, °F	T 53	135	-	130	-	125	-
Resilience, 77°F, %	D 5329	25	-	20	-	10	-
Flash point, C.O.C., °F	T 48	450	-	450	-	450	-
Tests on residue from Thin-Film Oven Test: Retained penetration ratio, 39.2°F, 200 g, 60 sec., % of Original	T 179 T 49	 75	 -	 75	 -	 75	 -

(10) Performance-Graded Binders. PG binders must be smooth and homogeneous, show no separation when tested in accordance with Tex-540-C, and meet Table 17 requirements.

Separation testing is not required if:

- a modifier is introduced separately at the mix plant either by injection in the asphalt line or mixer,
- the binder is blended on site in continuously agitated tanks, or
- binder acceptance is based on field samples taken from an in-line sampling port at the hot mix plant after the addition of modifiers.

TABLE 17

PERFORMANCE-GRADED BINDERS

Property and Test Method	Performance Grade																	
	PG 58			PG 64				PG 70				PG 76				PG 82		
	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Average 7-day max pavement design temperature, °C ¹	< 58			<64				<70				<76				<80		
Min pavement design temperature, °C ¹	> 22	>28	>34	>-16	>22	>-28	>34	>-16	>22	>-28	>34	>-16	>22	>-28	>34	>-16	>22	>-28
ORIGINAL BINDER																		
Flash point, T 48, Min, °C	230																	
Viscosity, T 316 ^{2,3} Max. 3.0 Pa-s. test temperature.	135																	
Dynamic shear, T 315: ⁴ G*/sin(δ), Min. 1.00 kPa Test temperature @ 10rad/sec.°C	58			64				70				76				82		
Elastic recovery, D 6084, 50°F, % Min	-	-	30	-	-	30	50	-	30	50	60	30	50	60	70	50	60	70
ROLLING THIN-FILM OVEN (Tex-541-C)																		
Mass loss, Tex-541-C, Max,	1.0																	
Dynamic shear, T 315: G*/sin(δ), Min, 2.20 kPa Test temperature @ 10rad/sec.,°C	58			64				70				76				82		
PRESSURE AGING VESSEL (PAV) RESIDUE (R 28)																		
PAV aging temperature, °C	100																	
Dynamic shear, T 315: G*/sin(δ), Max, 5000 kPa Test temperature @ 10 rad/sec., °C	25	22	19	28	25	22	19	28	25	22	19	28	25	22	19	28	25	22
Creep stiffness, T 313: ^{5,6} S, max, 300 MPa, m-value, min, 0.300 Test temperature @ 60 sec., °C	-12	-18	-24	-6	-12	-18	-24	-6	-12	-18	-24	-6	-12	-18	-24	-6	-12	-18
Direct tension, T 314: ⁶ Failure strain, min, 1.0% Test temperature @ 1.0 mm/min., °C	-12	-18	-24	-6	-12	-18	-24	-6	-12	-18	-24	-6	-12	-18	-24	-6	-12	-18

1. Pavement temperatures are estimated from air temperatures using an algorithm contained in a Department-supplied computer program may be provided by the Department, or by following the procedures outlined in AASHTO MP 2 and PP 28.
2. This requirement may be waived at the Department's discretion if the supplier warrants that the asphalt binder can be adequately pumped, mixed and compacted at temperatures that meet all applicable safety, environmental, and constructability requirements. At test temperatures where the binder is a Newtonian fluid, any suitable standard means of viscosity measurement may be used, including capillary (T 201 or T 202) or rotational viscometry (T 316).
3. Viscosity at 135°C is an indicator of mixing and compaction temperatures that can be expected in the lab and field. High values may indicate high mixing and compaction temperatures. Additionally, significant variation can occur from batch to batch. Contractors should be aware that variation could significantly impact their mixing and compaction operations. Contractors are therefore responsible for addressing any constructability issues that may arise.
4. For quality control of unmodified asphalt binder production, measurement of the viscosity of the original asphalt binder may be substituted for dynamic shear measurements of $G^*/\sin(\delta)$ at test temperatures where the asphalt is a Newtonian fluid. Any suitable standard means of viscosity measurement may be used, including capillary (T 201 or T 202) or rotational viscometry (T 316).
5. Silicone beam molds, as described in AASHTO TP 1-93, are acceptable for use.
6. If creep stiffness is below 300 MPa, direct tension test is not required. If creep stiffness is between 300 and 600 MPa, the direct tension failure strain requirement can be used instead of the creep stiffness requirement. The m-value requirement must be satisfied in both cases.

300.3. EQUIPMENT.

Provide all equipment necessary to transport, store, sample, heat, apply, and incorporate asphalts, oils and emulsions.

300.4. CONSTRUCTION.

(1) Typical Material Use. Table 18 shows typical materials used for specific applications. These are typical uses only. Circumstances may require use of other material.

TABLE 18
TYPICAL MATERIAL USE

Material Application	Typically Used Materials
Hot-mixed, hot-laid asphalt mixtures	PG binders, A-R binders Types I and II
Surface treatment	AC-5, AC-10, AC-5 w/2% SBR, AC-10 w/2% SBR, AC-15P, AC-20-5TR, HFRS-2, MS-2, CRS-2, CRS-2H, HFRS-2P, CRS-2P, A-R binders Types II and III
Surface treatment (cool weather)	RS-1P, CRS-1P, RC-250, RC-800, RC-3000, MC-250, MC-800, MC-3000, MC-2400L
Precoating	AC-5, AC-10, PG 64-22, SS-1, SS-1H, CSS-1, CSS-1H
Tack coat	PG binders, SS-1H, CSS-1H, EAP&T
Fog seal	SS-1, SS-1H, CSS-1, CSS-1H
Hot-mixed, cold-laid asphalt mixtures	AC-0.6, AC-1.5, AC-3, AES-300, AES-300P, CMS-2, CMS-2S
Patching mix	MC-800, SCM I, SCM II, AES-300S
Recycling	AC-0.6, AC-1.5, AC-3, AES-150P, AES-300P, recycling agent, emulsified recycling agent
Crack sealing	SS-1P, polymer mod AE crack sealant, rubber asphalt crack sealers (Class A, Class B)
Microsurfacing	CSS-1P
Prime	MC-30, AE-P, EAP&T, PCE
Curing membrane	SS-1, SS-1H, CSS-1, CSS-1H, PCE
Erosion control	SS-1, SS-1H, CSS-1, CSS-1H, PCE

TABLE 19
STORAGE AND APPLICATION TEMPERATURES

Type-Grade	Application		Storage Maximum (°F)
	Recommended Range, °F	Maximum Allowable (°F)	
AC-0.6, AC-1.5, AC-3	200-300	350	350
AC-5, AC-10	275-350	350	350
AC-5 w/2% SBR, AC-10 w/2% SBR, AC-15P, AC-20-5TR	300-375	375	360
RC-250	125-180	200	200
RC-800	170-230	260	260
RC-3000	215-275	285	285
MC-30, AE-P	70-150	175	175
MC-250	125-210	240	240
MC-800, SCM I, SMC II	175-260	275	275
MC-3000, MC-2400L	225-275	290	290
HFRS-2, MS-2, CRS-2, CRS-2H, HFRS-2P, CRS-2P, CMS-2, CMS-2S, AES-300, AES-300S, AES-150P, AES-300P	120-160	180	180
SS-1, SS-1H, CSS-1, CSS-1H, PCE, EAP&T, SS-1P, RS-1P, CRS-1P, SCC-1P, recycling agent, emulsified recycling agent, polymer mod AE crack sealant	50-130	140	140
PG binders	275-305	350	350
Rubber asphalt crack sealers (Class A, Class B)	350-375	400	-
A-R binders Types, I, II, and III	325-425	425	425

300.5 MEASUREMENT AND PAYMENT.

The work performed, materials furnished, equipment, labor, tools, and incidentals will not be measured or paid for directly, but is subsidiary or is included in payment for other bid Items.

CITY OF SAN ANGELO**ITEM 301****ASPHALT ANTISTRIPPING AGENTS****301.1 DESCRIPTION.**

This Item shall govern for furnishing and incorporating lime or liquid antistripping agents in the production of asphaltic concrete pavement mixtures and/or asphalt stabilized base mixtures.

301.2 MATERIALS.

(1) **Lime.** The Lime shall meet the requirements of Item 264, "Lime and Lime Slurry".

(2) **Liquid Antistripping Agent.** The antistripping agent shall be a uniform liquid with no evidence of crystallization, settling or separation of components. Prior to delivery, a sample of the antistripping agent proposed for use shall be furnished to the City. Information to be provided with the sample includes the material safety data sheet, the specific gravity of the agent at the manufacturer's recommended addition temperature, the manufacturer's recommended dosage range and handling and storage instructions. The liquid antistripping agent shall be delivered in properly labeled containers, unopened as shipped from the manufacturer, or in sealed tank trucks properly invoiced.

301.3 MIXTURE DESIGN EVALUATION.

Laboratory mixtures of the proposed asphaltic pavement or base will be evaluated during mixture designing.

Hot-placed mixtures, except for Item 342, "Plant Mix Seal" surfacing mixture, will be evaluated for moisture susceptibility as follows, unless otherwise shown on the Plans. Test Method Tex-531-C will be the evaluation procedure, and a minimum tensile strength ratio of 0.70 is required. Laboratory mixture meeting this requirement will be tested in accordance with Test Method Tex-530-C to establish the maximum stripping to be allowed during production verification testing.

Cold-placed mixtures and Item 342, "Plant Mix Seal", surfacing mixture will be evaluated as follows, unless otherwise shown on the Plans. Test Method Tex-530-C will be the evaluation procedure, and a maximum stripping of ten (10) percent is allowed.

If the proposed mixture does not comply with the specified resistance to moisture damage, the Contractor shall make changes in the combination of materials or add an antistripping agent in order to provide a mixture that will comply with the specified resistance to moisture damage.

When lime is used as an antistripping agent, the selected amount shall be in the range of 0.5 to 2.0 percent by weight of the individual aggregate being treated.

When a liquid antistripping agent is used, the selected amount of agent shall be from 0.3 to 1.0 percent by weight of the asphalt in the mixture but shall not exceed the amount recommended by the manufacturer.

When shown on the Plans, a limited number of addition rates will be evaluated for a given antistripping agent.

301.4 CONSTRUCTION METHODS.

(1) General. The Contractor shall provide all the necessary equipment for mixing, handling, metering and dispensing the asphalt antistripping agent.

The produced asphaltic mixture will be evaluated to verify resistance to moisture damage in accordance with Test Method Tex-530-C, unless otherwise shown on the Plans. When Test Method Tex-531-C is the required evaluation procedure during mixture design, the produced mixture shall not strip more than the percentage established during mixture design correlation testing with Test Method Tex-530-C. When Test Method Tex-530-C is the required evaluation procedure during mixture design, a maximum stripping of ten (10) percent is allowed in the produced mixture, unless otherwise shown on the Plans. If testing indicates that the required level of resistance to moisture damage is not being achieved in the plant mixture, production shall cease until trial production indicates that the problem has been corrected.

(2) Lime. Lime shall be added in slurry or dry form. It shall be added between the plant cold feeds and the dryer during mixture production, unless otherwise shown on the Plans. Whether added in slurry or dry form, the method of application shall be such that the lime is thoroughly mixed with the aggregates being treated.

The lime shall be applied to the aggregate at the required rate by means of a metering device. The Contractor shall demonstrate that the metering equipment will properly deliver the required rate of lime. The City must approve the metering equipment and location of lime application.

When lime is added in dry form, Type A hydrated lime shall be used. It shall be mixed with wet aggregate in a suitable pugmill mixer. Additional water shall be introduced into the mixer. If necessary to insure that the aggregate contains at least two (2) percent by weight moisture above the saturated surface dry condition.

(3) Liquid Antistripping Agent. Handling of liquid antistripping agent shall at all times be in accordance with the manufacturer's recommendations. The agent shall not show evidence of any separation or non-uniformity at time of use. For agents which have a high viscosity at normal ambient temperatures, the Contractor shall warm the material by suitable means to the application temperature recommended by the additive manufacturer so that proper consistency for accurate metering is assured. The agent shall be added to the asphalt line at the required rate by means of an in-line-metering device just prior to introduction of the asphalt into the mixing plant. The Contractor shall demonstrate that the meter meets the requirements of TxDOT Item 520, "Weighing and Measuring Equipment". A blending device is required to disperse the additive in the asphaltic material. The City must approve the metering and blending equipment and location.

301.5 MEASUREMENT AND PAYMENT.

The work performed, materials furnished and all labor, tools, equipment and incidentals necessary to complete the work under this Item will not be measured or paid for directly, but will be considered subsidiary to the appropriate construction Item of the contract.

CITY OF SAN ANGELO**ITEM 302****AGGREGATES FOR SURFACE TREATMENTS****302.1 DESCRIPTION.**

This Item shall govern for aggregates and precoated aggregates used in the construction of surface treatments.

302.2 MATERIALS.

(1) Aggregates. Aggregates shall be composed of gravel, crushed gravel, or crushed stone. When specified on the Plans, other aggregate types may be permitted or required. Aggregate from each source shall meet the requirements specified herein. Source is defined as a geographical location of naturally occurring material that can be mined or quarried from the original in-situ deposit.

The aggregate shall not contain more than two (2) percent by weight of soft particles and other deleterious material as determined by Test Method Tex-217-F, Part I.

The aggregate shall not contain more than one (1) percent loss from fine dust, clay-like particles and/ or silt when tested in accordance with Test Method Tex-217-F, Part II.

The flakiness index for the aggregate, as determined by Test Method Tex-224-F, shall not exceed 17 unless otherwise shown on the Plans.

The percent wear, as determined by Test Method Tex-410-A, for each of the materials shall not exceed 35 percent.

Crushed gravel shall have a minimum of 85 percent of the particles retained on the No. 4 sieve with two (2) or more mechanically induced crushed faces, as determined by Test Method Tex-460-A, Part I.

The aggregate will be subjected to five (5) cycles of magnesium sulfate soundness testing in accordance with Test Method Tex-411-A. The loss shall not exceed 25 percent, unless otherwise shown on the Plans.

The polish value for the aggregate used in the surface or finish course shall not be less than the value shown on the Plans, when tested in accordance with Test Method Tex-438-A. Unless otherwise shown on the Plans, the polish-value requirement will apply only to aggregate used on travel lanes. When aggregates requiring polish value are supplied from a source that is rated by the Materials and Tests Division, the Rated Source Polish Value (RSPV) for that source will be used to meet this requirement. When aggregates are supplied from a source that is not rated, the aggregate will be sampled and tested prior to use. The procedures will be in accordance with Test Methods Tex-400-A and Tex-438-A, Part I.

Blending of aggregates to achieve polish value will not be permitted, unless otherwise shown on the Plans. If blending is allowed, Test Method Tex-438-A, Part II, Method B will be used to determine the required blend percentages. However, a minimum of 50 percent by volume of non-polishing aggregate is required.

(2) Precoated Aggregate. Precoated aggregate shall be aggregate of the type and grade specified, coated with 0.5 to 1.5 percent, by weight, of residual bitumen from a precoating material.

When limestone rock asphalt is used, it shall be fluxed with 0.5 to 1.5 percent by weight of fluxing material.

The grade of aggregate specified shall meet all requirements of Articles 302.2 and 302.4 prior to the application of the precoat of fluxing material.

The materials may be mixed on the job or at a central mixing plant and shipped ready for use. Mixes that do not maintain flow qualities such that the precoated aggregate may be satisfactorily spread by approved mechanical spreading devices will not be acceptable.

Materials that are not uniformly and/or properly coated, in the opinion of the City, will not be accepted for use.

(3) Asphaltic Material. The precoating or fluxing material shall meet the requirements of Item 300 "Asphalts, Oils, and Emulsions". Unless otherwise shown on the Plans, any of the types and grades shown in Item 300, "Asphalt, Oils, and Emulsions" may be used.

(4) Water. Water in the amount not to exceed three (3) percent by weight of the mixture may be used in precoating aggregate or fluxing limestone rock asphalt aggregate.

302.3 TYPES.

Various aggregate types are identified as follows:

(1) Uncoated Aggregate Types.

Type A. Type A aggregate shall consist of gravel or crushed stone.

Type B. Type B aggregate shall consist of crushed gravel, or crushed stone.

Type C. Type C aggregate shall be as shown on the Plans.

Type D. Type D aggregate shall consist of crushed gravel, crushed slag or crushed stone.

Type E. Type E aggregate shall be as shown on the Plans.

(2) Precoated Aggregate Types.

Type PA. Type PA shall be precoated aggregate consisting of gravel or crushed stone.

Type PB. Type PB shall be precoated aggregate consisting of crushed gravel, or crushed stone.

Type PC. Type PE shall be precoated aggregate as shown on the Plans.

302.4 GRADES.

When tested by Test Method Tex-200-F, Part I, the gradation requirements shall be as follows:

		Percent By Weight
Grade 1:	Retained on 1" sieve	0
	Retained on 7/8" sieve	0 – 2
	Retained on 3/4" sieve	20 – 35
	Retained on 5/8" sieve	85 – 100
	Retained on 3/8" sieve	95 – 100
	Retained on No. 10 sieve	99 – 100
Grade 2:	Retained on 7/8" sieve	0
	Retained on 3/4" sieve	0 – 2
	Retained on 5/8" sieve	20 – 40
	Retained on 1/2" sieve	80 – 100
	Retained on 3/8" sieve	95 – 100
	Retained on No. 10 sieve	99 – 100
Grade 3:	Retained on 3/4" sieve	0
	Retained on 5/8" sieve	0 – 2
	Retained on 1/2" sieve	20 – 40
	Retained on 3/8" sieve	80 – 100
	Retained on 1/4" sieve	95 – 100
	Retained on No. 10 sieve	99 – 100
Grade 4:	Retained on 5/8" sieve	0
	Retained on 1/2" sieve	0 – 2
	Retained on 3/8" sieve	20 – 35
	Retained on No. 4 sieve	95 – 100
	Retained on No. 10 sieve	99 – 100
Grade 5:	Retained on 1/2" sieve	0
	Retained on 3/8" sieve	0 – 5
	Retained on No. 4 sieve	40 – 85
	Retained on No. 10 sieve	98 – 100
	Retained on No. 20 sieve	99 – 100

When shown on the Plans, the aggregate of the specified grade(s) shall have from 99.5 to 100 percent by weight retained on the No. 200 sieve.

302.5 EQUIPMENT.

(1) Mixing Plants. Mixing plants that will not continuously meet all the requirements of this specification shall be condemned.

Mixing plants may be either the weigh-batch type, the continuous mixing type or the drum mix type. All plants shall be equipped with satisfactory conveyors, power units, aggregate handling equipment, and bins.

If the City approves the use of emulsion as a precoat material, he may also waive the requirement for a dryer, as specified below, if it is demonstrated that a satisfactory coating can be obtained without drying or heating the aggregate.

When using a low-grade fuel oil or waste oil the plant shall meet the requirements of Item 340.4. (2).

(a) Weigh Batch Type.

Cold Aggregate Bin and Proportioning Device. The cold aggregate bins or aggregate stockpiles shall be of sufficient number and size to supply the amount of aggregate required to keep the plant in continuous operation. The proportioning device shall be such as will provide a uniform and continuous flow of aggregate in the desired proportion to the plant.

Dryer. The dryer shall continually agitate the aggregate during heating. The temperature shall be controlled so that the aggregate will not be damaged in the drying and heating operations.

The burner, or combination of burners, and type of fuel used shall be such that in the process of heating the aggregate to the desired or specified temperatures, no residue from the fuel shall adhere to the heated aggregate. A recording thermometer shall be provided which will record the temperature of the aggregate when it leaves the dryer. The dryer shall be of sufficient size to keep the plant in continuous operation. The dryer will not be required for precoating natural limestone rock asphalt.

Screening and Proportioning. The screening capacity and size of the bins shall be sufficient to screen and store the amount of aggregate required to properly operate the plant and keep the plant in continuous operation at full capacity. Proper provisions shall be made to enable inspection forces to have easy and safe access to the proper location on the mixing plant where accurate representative samples of aggregate may be taken from the bins for testing.

Weighing and Measuring Equipment. The weighing and measuring equipment shall be of sufficient capacity and of adequate design for proper batching. The following equipment, conforming to the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", shall be furnished:

1. Aggregate weigh box and batching scales.

2. Bucket and scales for precoat material or fluxing material.

A pressure type flow meter may be used to measure the precoat material or fluxing material for each batch.

If a pressure type flow meter is used to measure the asphaltic material, the requirements of TxDOT Item 520, "Weighing and Measuring Equipment" shall apply.

Provisions of a permanent nature shall be made for checking the accuracy of the asphaltic material-measuring device. The line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for the precoating material.

Mixer. The mixer shall be of the pugmill type, and shall have a capacity of not less than 3000 pounds in a single batch. Any mixer that has a tendency to segregate the aggregate or fails to secure a thorough and uniform mixing with the precoat material or fluxing material shall not be used. All mixers shall be provided with an automatic time lock that will lock the discharge doors of the mixer for the required mixing period. The dump door or doors and the shaft seals of the mixer shall be tight enough to prevent the spilling of aggregate or mixture from the pugmill.

(b) Modified Weigh-Batch Type.

General. This plant is similar to the weigh-batch type plant. The hot bin screens shall be removed and the aggregate control is placed at the cold feeds. The cold feed bins will be the same as those required for the drum-mix type plant.

Cold-Aggregate Bin Unit and Feed System. The number of bins in the cold-aggregate bin unit shall be equal to or greater than the number of stockpiles of individual materials to be used. The bins shall be of sufficient size to store the amount of aggregate required to keep the plant in continuous operation.

When blending materials, the bin unit shall be of proper design to prevent overflow of material from one bin to another. There shall be vertical partitions meeting the requirements of Item 340-4(2). The feed system shall provide a uniform and continuous flow of aggregate in the desired proportion to the dryer. The Contractor shall furnish a chart indicating the calibration of each cold bin in accordance with Construction Bulletin C-14, or other methods of cold bin calibration acceptable to the City.

Scalping Screen. A scalping screen shall be required after the cold feeds and ahead of the hot aggregate surge bins.

Dryer. The dryer shall continually agitate the aggregate during heating. The temperature shall be controlled so that the aggregate will not be damaged in the drying and heating operations. The dryer shall be of sufficient size to keep the plant in continuous operation.

Screening and proportioning. The hot aggregate shall not be separated into sizes after being dried. There shall be one or more surge bins provided between the dryer and the weigh

hopper. Surge bins shall be of sufficient size to hold enough combined aggregate for one complete batch of mixture.

Aggregate Weigh Box and Batching Scale. The aggregate weigh box and batching scales shall be of sufficient capacity to hold and weigh a complete batch of aggregate. The weigh box and scales shall conform to the requirements of TxDOT Item 520, "Weighing and Measuring Equipment".

Asphaltic Material Measuring System. If an asphaltic material bucket and scales are used, they shall be of sufficient capacity to hold and weigh the necessary asphaltic material for one batch. The bucket and scales shall conform to the requirements of TxDOT Item 520, "Weighing and Measuring Equipment".

If a pressure type flow meter is used to measure the asphaltic material, the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", shall apply. This system shall include as automatic temperature compensation device to insure a constant percent by weight of asphaltic material in the mixture.

Provisions of a permanent nature shall be made for checking the accuracy of the asphaltic material-measuring device. The asphalt line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for the asphaltic material.

Mixer. The mixer shall be of the pugmill type and shall have a capacity of not less than 3,000 pounds (of natural-aggregate mixture) in a single batch, unless otherwise shown on the Plans. Any mixer that has a tendency to segregate the aggregate or fails to secure a thorough and uniform mixing with the precoat material or fluxing material shall not be used. All mixers shall be provided with an automatic timer that will lock the discharge doors of the mixer for the required mixing period. The dump door or doors and the shaft seals of the mixer shall be tight enough to prevent spilling of aggregate or mixture from the pugmill.

(c) Continuous Mixing Type.

Cold Aggregate Bin and Proportioning Device. Same as for weigh-batch type of plant.

Dryer. Same as for weigh-batch type of plant.

Screening and Proportioning. Same as for weigh-batch type of plant. These requirements shall also apply to materials that are stockpiled and that are proposed for direct use by a continuous mixing plant without the use of plant bins.

Aggregate Proportioning Device. The aggregate proportioning device shall be so designed that when properly operated a uniform and continuous flow of aggregate into the mixer will be maintained.

Spray Bar for Precoat Material and Fluxing Material. The spray bar for the precoat material or fluxing material shall be so designed that the material will spray uniformly and continuously into the mixer.

Meter for Precoat Material or Fluxing Material. An accurate recording meter for precoat material or fluxing material shall be placed in the line leading to the spray bar so that the accumulative amount of precoat material or fluxing material being used can be accurately determined. Provisions of a permanent nature shall be made for checking the accuracy of the meter output.

Mixer. The mixer shall be of the continuous type and shall have a capacity of not less than 40 tons of mixture per hour. Any mixer that has a tendency to segregate the aggregate or fails to secure a thorough and uniform mixing of the aggregate with the precoat material or fluxing material shall not be used.

(d) Drum Mix Plant

General. The plant shall be adequately designed and constructed for the process of mixing aggregates and precoat material. The plant shall be equipped with satisfactory conveyors, power units, aggregate-handling equipment and feed controls.

Cold Aggregate Bin and Feed System. The number of bins in the cold aggregate bin unit shall be equal to or greater than the number of stockpiles of individual materials to be used. The bin unit shall be of sufficient size to store the amount of aggregate required to keep the plant in continuous operation.

When blending materials, the bin unit shall be of proper design to prevent overflow of material from one bin to another. There shall be vertical partitions meeting the requirements of Item 340.4(2). The feed system shall provide a uniform and continuous flow of aggregate in the desired proportion to the dryer. The Contractor shall furnish a chart indicating the calibration of each cold bin in accordance with Construction Bulletin C-14 or other methods acceptable to the City.

The system shall provide positive weight measurement of the combined cold aggregate feed by use of belt scales. Provisions of a permanent nature shall be made for checking the accuracy of the measuring device as required by TxDOT Item 520, "Weighing and Measuring Equipment". When a belt scale is used, mixture production shall be maintained so that the scale is used, mixture production shall be maintained so that the scale normally operates between 50 and 100 percent of its rated capacity. Belt scale operation below 50 percent of the rated capacity may be allowed by the City if accuracy checks show the scale to meet the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", at the selected rate and it can be satisfactorily demonstrated to the City that mixture uniformity and quality have not been adversely affected.

Scalping Screen. A scalping screen shall be required, after the cold feeds and ahead of the combined aggregate belt scale.

Precoat Material Measuring System. An asphaltic material measuring device meeting the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", shall be placed in the line leading to the mixer so that the cumulative amount of precoat material used can be accurately determined. Provisions of a permanent nature shall be made for checking the accuracy of the measuring device output. The line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for

the precoat material. The measuring system shall include an automatic temperature compensation device to maintain a constant percent by weight of precoating material in the mixture.

Synchronization Equipment for Feed-Control Systems. The precoat material feed-control shall be coupled with the total aggregate weight-measuring device to automatically vary the precoat material feed rate to maintain the required proportion.

Mix System. The mix system shall control the temperature so that aggregate and asphalt will not be damaged in the drying, heating and mixing operations. A continuously recording thermometer shall be provided which will indicate the temperature of the mixture as it leaves the mixer.

Surge-Storage System. A surge-storage system shall be required to minimize the production interruptions during the normal day's operations. A device such as a gob hopper or other similar devices approved by the City to prevent segregation in the surge-storage bin shall be required.

(2) Heating Equipment for Precoat Material and Fluxing Material. Heating equipment for precoat material and fluxing material shall be adequate to heat the required amount of material to the desired temperature. The material may be heated by steam coils, which shall be absolutely tight. Direct fire heating will be permitted, provided the heating system used is manufactured by a reputable concern and there is positive circulation of the liquid throughout the heater. Agitation with steam or air will not be permitted. The heating apparatus shall be equipped with a recording thermometer with a 24-hour chart that will record the temperature of the precoat material or fluxing material where it is at the point of highest temperature.

302.6 STORAGE, PROPORTIONS, AND MIXING.

(1) Aggregate Storage. If the aggregates are stored or stockpiled, they shall be handled in such a manner as to prevent segregation, the mixing of the various materials or sizes, and the contamination with foreign materials. The grading of aggregates proposed for use and as supplied to the mixing plant shall be uniform. When required by the City, additional material shall not be added to stockpiles that have been samples for approval.

When asphalt cement is the precoating material, stockpile height shall be limited to approximately three feet immediately after production to limit the build up of heat. These stockpiles may be consolidated after cooling adequately, in the opinion of the City.

The use of limestone rock asphalt aggregate containing moisture in excess of the saturated surface-dry condition will not be permitted. Excess moisture will be evidenced by the visual surface moisture on the aggregate or any unusual quantities of fines clinging to the aggregate.

(2) Storage and Heating of Precoating Material or Fluxing Material. The precoating or fluxing material storage shall be ample to meet the requirements of the plant. The precoating material shall not be heated in storage above the maximum temperature set forth in Item 300, "Asphalts, Oils, and Emulsions". All equipment used in the storage and handling of precoat

material or fluxing material shall be kept in a clean condition at all times and shall be operated in such manner that there will be no contamination with foreign matter.

(3) Feeding and Drying of Aggregate. The feeding of various sizes of aggregate, other than natural limestone rock asphalt, to the dryer or drum mixer shall be done through the cold aggregate bin and proportioning device in such a manner that a uniform and constant flow of material in the required proportions will be maintained. The aggregate shall be heated to the temperature necessary to produce a mixture meeting the requirements of Item 302.2(3).

(4) Proportioning. The proportioning of the various materials entering into the mixture shall be as directed by the City and in accordance with these specifications. Aggregate shall be proportioned by weight using the weigh box and batching scales herein specified when the weigh-batch type of plant is used and by volume using the aggregate proportioning device when the modified weigh-batch type, the continuous mixer type or drum mix plant is used. The precoat material or fluxing material shall be proportioned by weight or by volume based on weight using the specified equipment.

(5) Mixing.

(a) Weigh-Batch Type and Modified Weigh-Batch Type Mixer. In the charging of the weigh box and in the charging of the mixer from the weigh box, such methods or devices shall be used as are necessary to secure a uniform mixture. In introducing the batch into the mixer, the aggregate shall be introduced first; shall be mixed thoroughly, as directed, to uniformly distribute the various sizes throughout the batch before the precoat material or fluxing material is added; the precoat material or fluxing material shall then be added and the mixing continued until such time that the aggregate is properly coated. This mixing period may be varied, if, in the opinion of the City, the mixture is not uniform.

(b) Continuous or Drum Mix Type Mixer. The amount of aggregate and precoat material or fluxing material entering the mixer and the rate of travel through the mixer shall be so coordinated that a uniform mixture of the specified grading and percent by weight of precoat material or fluxing material will be produced.

302.7 MEASUREMENT AND PAYMENT.

Aggregates provided in accordance with this specification will be measured and paid for in accordance with the governing specifications for the items of construction in which these materials are used.

CITY OF SAN ANGELO**ITEM 310****ASPHALT PRIME COAT****310.1 DESCRIPTION.**

This Item shall consist of an application of asphaltic material on the completed base course in accordance with these specifications.

310.2 MATERIALS.

The asphaltic material used for the prime coat shall be of the type and grade shown on the plans and when tested by approved laboratory methods shall meet the requirements of Item 300, "Asphalts, Oils, and Emulsions".

310.3 CONSTRUCTION METHODS.

Prime coat shall not be applied when the air temperature is below 60°F and falling, but it may be applied when the air temperature is above 50°F and rising, the air temperature being taken in the shade away from artificial heat. Asphaltic material shall not be placed when general weather conditions, in the opinion of the City, are not suitable.

When, in the opinion of the City, the base is satisfactory to receive the prime coat, the surface shall be prepared by sweeping or other approved methods. If found necessary by the City, the surface shall be lightly sprinkled just prior to application of the asphaltic material. The asphaltic material shall be applied on the clean base by an approved type of self-propelled pressure distributor so operated as to distribute the material at the rate directed by the City, evenly and smoothly, under a pressure necessary for proper distribution. The Contractor shall provide all necessary facilities for determining the temperature of the asphaltic material in all of the heating equipment and in the distributor, for determining the rate at which it is applied, and for securing uniformity at the junction of two (2) distributor loads.

All storage tanks, piping, retorts, booster tanks, and distributors used in storing or handling asphaltic material shall be kept clean and in good operating condition at all times and shall be operated in such manner that there will be no contamination of the asphaltic material with foreign material. It shall be the responsibility of the Contractor to provide and maintain in good working order a recording thermometer at the storage heating unit at all times.

The distributor tank, when used for pay purposes, shall have been calibrated within ten years from the date it is first used on this project, unless modifications and or changes occur in the tank system that may alter the volume. The tank calibration procedure shall be in accordance with Test Method Tex-922-K, Part I, and shall be signed and sealed by a registered professional engineer. Unless otherwise shown on the plans, the Contractor shall provide the tank calibration and shall furnish the City an accurate and satisfactory calibration record prior to beginning the work.

The City may at any time verify calibration accuracy in accordance with Test Method Tex-922-K, Part II, and may perform the required recalibration if the calibration is found to be in error.

The City will select the temperature of application and the Contractor shall apply the asphalt at a temperature within 15°F of the temperature selected.

WARNING TO CONTRACTORS. Attention is called to the fact that asphaltic materials are flammable. The utmost care should be taken to prevent open flames from coming in contact with the asphaltic material or the gases of it. The Contractor shall be responsible for any fires or accidents, which may result from heating the asphaltic materials.

The Contractor shall be responsible for the maintenance of the surface until the work is accepted by the City.

No traffic, hauling or placing of any subsequent courses shall be permitted over the freshly applied prime coat until authorized by the City.

The asphalt or emulsion may be diluted as directed by the City, but in no case shall the base asphalt material may be diluted no more than 30 percent. Pay for prime will be by the gallon of asphalt actually used before dilution.

310.4 MEASUREMENT.

This Item will be measured at the point of delivery on the road by the gallon of asphaltic material at the applied temperature. The quantity to be measured for payment shall be the number of gallons used, as directed, of the accepted prime coat.

310.5 PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under Item 310.4, "Measurement" will be paid for at the unit price bid for "Asphaltic Material", of the type and grade specified. This price shall be full compensation for cleaning the area to be primed; for furnishing all required materials; for all heating, hauling, mixing and distributing the asphaltic material as specified; for all freight involved; and for all manipulation, labor, tools, equipment and incidentals necessary to complete the work.

CITY OF SAN ANGELO
ITEM 316
SURFACE TREATMENT

316.1 DESCRIPTION.

This Item shall govern for the construction of a surface treatment composed of a single, double or triple application of asphaltic material, each covered with aggregate, constructed on existing pavements or on the prepared base course or surface in accordance with these specifications. Quantities for the different types of surfaces and materials will be shown on the Basis of Estimate in the Plans.

316.2 MATERIALS.

All materials shall be of the type and grade shown on the Plans and shall conform to the pertinent material requirements of the following Items:

Item 300, "Asphalts, Oils, and Emulsions"

Item 302, "Aggregate for Surface Treatments"

TxDOT Item 303, "Aggregate for Surface Treatments (Lightweight)"

316.3 EQUIPMENT.

(1) Distributor. The distributor shall be a self-propelled pressure type, equipped with an asphaltic material heater and a distributing pump capable of pumping the material at the specified rate through the distributor spray bar. The distributor spray bar shall be capable of fully circulating the asphaltic material. In order to prevent streaking or irregular distribution of asphaltic material, the distributor spray bar shall contain nipples and valves so constructed that the nipples will not become partially plugged with congealing asphaltic material. Distributor equipment shall include a tachometer, pressure gauges, volume measuring devices, and a thermometer for reading the temperature of tank contents.

The distributor tank when used for pay purposes, shall have been calibrated within three (3) years from the date it is first used on this project. The tank calibration procedure shall be in accordance with Test Method Tex-922-K, Part 1, and shall be signed and sealed by a registered professional engineer. Unless otherwise shown on the Plans, the Contractor shall provide the tank calibration record prior to beginning the work. The City may at any time verify calibration accuracy in accordance with Test Method Tex-922-K, Part II, and may perform the required re-calibration if the calibration is found to be in error.

When a uniform application of asphaltic material is not being achieved, the City may require that the spray bars on the distributor be controlled by an operator riding in such a position at the rear of the distributor that operation of all sprays is in full view.

(2) **Aggregate Spreader.** A self-propelled continuous-feed aggregate spreader shall be used which will uniformly spread aggregate at the rate specified by the City.

(3) **Rollers.** Rolling equipment shall meet the governing specifications for Item 210, “Rolling (Flat Wheel)” and Item 213, “Rolling (Pneumatic Tire)”.

(4) **Broom.** The broom shall be a rotary, self-propelled power broom for cleaning existing surfaces.

(5) **Asphalt Storing and Handling Equipment.** All equipment used in storing or handling asphaltic material shall be kept clean and in good operating condition at all times and shall be operated in such a manner that there will be no contamination of the asphaltic material. The Contractor shall provide and maintain a recording thermometer to continuously indicate the temperature of the asphaltic material at the storage heating unit when storing of asphalt is permitted.

(6) Vehicles used for hauling aggregate shall be of uniform capacity unless otherwise authorized by the City.

316.4 CONSTRUCTION METHODS.

(1) **General.** Temporary stockpiling of aggregates on the right-of-way will be permitted, provided that the stockpiles are so placed as to allow for the safety of the traveling public and not obstruct traffic or sight distance, and do not interfere with access from abutting property, nor with roadway drainage.

The aggregate placement sites will be subject to the City. Location of stockpiles shall be either a minimum of 30 feet from the edge of the travel lanes or shall be signed and barricaded as shown on the Plans.

Surface treatments shall not be applied when the air temperature is below 60° F. and is falling, but may be applied when the air temperature is above 50° F. and is rising, the air temperature being taken in the shade and away from artificial heat. Surface treatments shall not be applied when the temperature of the surface on which the surface treatment is to be applied is below

60° F. When latex modified asphalt cement is specified, surface treatments shall not be applied when the air temperature is below 80° F. and is falling, but may be applied when air temperature is above 70 F. and is rising and shall not be applied when the temperature of the surface on which the surface treatment is to be applied is below 70° F. When cutback asphalt or asphaltic materials designed for cool weather placement are used, application may occur whenever the air and surface temperatures are acceptable to the City. Asphaltic material shall not be placed when general weather conditions, in the opinion of the City, are not suitable.

The area to be treated shall be cleaned of dirt, dust or other deleterious matter by sweeping or other approved methods. If it is found necessary by the City, the surface shall be lightly sprinkled with water just prior to the application of asphaltic material. The rates shown on the Plans for asphalt and aggregate are for estimating purposes only. The rates may be varied as directed by the City.

The City will select the temperature of application within the limits recommended in Item 300, "Asphalts, Oils and Emulsions". The Contractor shall apply the asphalt at a temperature within 15° F. of the temperature selected.

The width of each application of asphaltic material shall be such to allow uniform application and immediate covering with aggregate. The Contractor shall be responsible for uniform application of asphaltic material at the junction of distributor loads. Paper or other suitable material shall be used to prevent overlapping of transverse joints. Longitudinal joints shall match lane lines unless otherwise authorized by the City. Application of asphaltic material will be measured as necessary to determine the rate of application.

The Contractor shall clear the finished surface of any surplus aggregate by sweeping or other approved methods after all rolling is completed.

Prior to final acceptance of the project, aggregate stockpiles deemed undesirable by the City shall be removed by the Contractor. The temporary stockpile areas shall be left in a neat condition satisfactory to the City.

When plans include "Aggregate (Stockpiled)", aggregate of the type and grade specified shall be stockpiled within the limits of the project at sites designated by the City.

(2) One-Course Surface Treatments or First Course of a Multiple Surface Treatment. Asphaltic material shall be applied by an approved distributor so operated as to distribute the material under a pressure necessary for uniform distribution.

The Contractor shall protect the existing raised pavement markers by any means acceptable to the City for one-course surface treatments, unless otherwise shown on the Plans.

Aggregates shall be immediately and uniformly applied and spread by the specified aggregate spreader, unless otherwise authorized by the City.

After applying the aggregate, the entire surface shall then be broomed, bladed or raked as required by the City and shall be thoroughly rolled with the type or types of rollers specified herein or as shown on the Plans.

The Contractor shall be responsible for the maintenance of the surface treatment until the City accepts the work. All holes or failures in the surface shall be repaired by use of additional asphalt and aggregate. All fat or bleeding surfaces shall be covered with approved cover material in such a manner that the asphaltic material will not adhere to or be picked up by the wheels of vehicles.

(3) Two Course or Three Course Surface Treatments. It is the intent of this specification that the application of asphalt and aggregate for multiple courses be applied within the same day or immediately thereafter and prior to opening the roadway to traffic.

The asphaltic material for each course of the surface treatment shall be applied and covered with aggregate in the same manner specified for the first application. Each surface shall then be broomed,

bladed or raked as required by the City and thoroughly rolled as specified for the first course. Asphaltic material and aggregate for each course shall be applied at the rates directed by the City.

The Contractor shall be responsible for the maintenance of each course until covered by the succeeding course or until the work is accepted by the City. All holes or failures in the surface shall be repaired by use of additional asphalt and aggregate. All fat or bleeding surface shall be covered with approved cover material in such a manner that the asphaltic material will not adhere to or be picked up by the wheels of vehicles.

316.5 MEASUREMENT.

(1) Asphalt Material. Asphaltic material will be measured as follows and as specified on the Plans.

(a) Volume. Asphaltic material will be measured at point of application on the road in gallons at the applied temperature. The quantity to be measured for payment shall be the number of gallons used, as directed, in the accepted surface treatment.

(2) Aggregates. Aggregate will be measured by the cubic yard in vehicles as applied on the road.

316.6 PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit prices bid for "Asphalt" and "Aggregate", of the type and grade specified. These prices shall each be full compensation for cleaning and sprinkling the existing surface; for furnishing, preparing, hauling, and placing all materials; for protecting existing pavement markers; for rolling, removing excess aggregate, and cleaning up stockpiles; for all freight and heating involved; and for all manipulations, labor, tools, equipment, and incidentals necessary to complete the work.

CITY OF SAN ANGELO**ITEM 340****HOT MIX ASPHALTIC CONCRETE PAVEMENT****340.1. DESCRIPTION.**

This Item shall govern for the construction of a base course, a level-up course, a surface course or any combination of these courses as shown on the Plans, each course being composed of a compacted mixture of aggregate and asphalt cement mixed hot in a mixing plant, in accordance with the details shown on the Plans and the requirements herein.

340.2. MATERIALS.

The Contractor shall furnish uncontaminated materials of uniform quality that meet the requirements of the plans and specifications. Contractor shall notify the Engineer of all material sources. Contractor shall notify the Engineer before changing any material source or formulation. When the Contractor makes a source or formulation change, the Engineer will verify that the requirements of this Item are met and may require a new laboratory mixture design, trial batch, or both. The Engineer may sample and test project materials at any time during the project to verify compliance.

(1) Aggregate. The aggregate shall be composed of a coarse aggregate, a fine aggregate, and if required or allowed, mineral filler and may include reclaimed asphalt pavement (RAP). The use of RAP may be required on the Plans. RAP use will be allowed in all mixtures except as specifically excluded herein or on the Plans. Samples of each aggregate shall be submitted for approval in accordance with TxDOT Item 6, "Control of Materials".

Aggregate from each stockpile shall meet the quality requirements of Table 1 and other requirements as specified herein. The aggregate contained in RAP will not be required to meet Table 1 requirements except as shown on the Plans.

(a) Coarse Aggregate. Coarse aggregate stockpiles must have no more than 20% material passing the No. 8 sieve. Contractor is to provide aggregates from sources listed in the *Bituminous Rated Source Quality Catalog* (BRSQC). Contractor is to provide aggregate from nonlisted sources only when tested by the Engineer and approved before use. Allow 30 calendar days for the Engineer to sample, test, and report results for nonlisted sources. Contractor is to provide coarse aggregate with at least the minimum surface aggregate classification (SAC) shown on the plans. SAC requirements apply only to aggregates used on the surface of travel lanes, unless otherwise shown on the plans.

Class B aggregate meeting all other requirements in Table 1 may be blended with a Class A aggregate in order to meet requirements for Class A materials. When blending Class A and B aggregates to meet a Class A requirement, ensure that at least 50% by weight of the material retained on the No. 4 sieve comes from the Class A aggregate source. Blend by volume if the bulk specific gravities of the Class A and B aggregates differ by more than 0.300. When

blending, do not use Class C or D aggregates. For blending purposes, coarse aggregate from RAP will be considered as Class B aggregate.

(b) Reclaimed Asphalt Pavement (RAP). RAP is defined as a salvaged, milled, pulverized, broken or crushed asphaltic pavement. The RAP to be used in the mix shall be crushed or broken to the extent that 100 percent will pass the (two) 2-inch sieve.

Dirt or other objectionable materials shall not contaminate the stockpiled RAP. Unless otherwise shown on the Plans, stockpiled, crushed RAP must have either a decantation of no more than 5 percent or a plasticity index of no more than eight (8), when tested in accordance with Test Method Tex-406-A, Part I, or Test Method Tex-106-E / ASTM D 4318, respectively. This requirement applies to stockpiled RAP from which the asphalt has not been removed by extraction.

City-owned RAP sources that are designated on the Plans will be available for use by the Contractor. Only RAP from City-owned sources will be allowed in mixes using more than 20 percent RAP, unless otherwise shown on the Plans. When RAP sources are designated, either in stockpile or existing pavements, the approximate gradation, asphalt content, and asphalt cement properties of this material will be shown on the Plans for material existing in pavements, or in a special provision "Local Material Sources for Reclaimed Asphaltic Pavement" for material in existing stockpiles. Any Contractor-owned RAP that is to be used on this project shall remain the property of the Contractor while stockpiled and shall not be intermingled with City-owned RAP stockpiles. Any unused Contractor-owned RAP material shall be removed from the project site upon completion of the project.

Only RAP from designated sources may be used in surface courses.

Excess RAP removed from designated sources will remain the property of the City and will be delivered to stockpile locations shown on the Plans.

(c) Fine Aggregate. Fine aggregates consist of manufactured sands, screenings, and field sands. The fine aggregate is defined as that part of the aggregate passing the No. 10 sieve and shall be of uniform quality throughout. When specified on the Plans, certain fine aggregate material may be allowed, required or prohibited. However, a maximum of 15 percent of the total virgin aggregate may be field sand or other uncrushed fine aggregate.

Fine aggregate stockpiles must meet the gradation requirements in Table 2. Supply fine aggregates that are free from organic impurities. The Engineer may test the fine aggregate in accordance with Tex-408-A to verify the material is free from organic impurities. At most 15% of the total aggregate may be field sand or other uncrushed fine aggregate. With the exception of field sand, use fine aggregate from coarse aggregate sources that meet the requirements shown in Table 1, unless otherwise approved. If 10% or more of the stockpile is retained on the No. 4 sieve, test the stockpile and verify that it meets the requirements in Table 1 for coarse aggregate angularity (Tex-460-A) and flat and elongated particles (Tex-280-F).

TABLE 1
AGGREGATE QUALITY REQUIREMENTS*

Property	Test Method	Requirement
COARSE AGGREGATE		
SAC	AQMP	As shown on plans
Deleterious Material, % max	Tex-217-F Part I	1.5
Decantation, %, max	Tex-217-F Part II	1.5
Micro-Deval abrasion, %, max	Tex-461-A	Note 1
Los Angeles abrasion, %, max	Tex-410-A	40
Magnesium sulfate soundness, 5 cycles, %, max	Tex-411-A	30 ²
Coarse aggregate angularity, 2 crushed faces, %, min	Tex-460-A, Part I	85 ³
Flat and elongated particles@ 5:1, %, max	Tex-280-F	10
FINE AGGREGATE		
Linear Shrinkage, %, max	Tex-107-E	3
COMBINED AGGREGATE⁴		
Sand Equivalent, %, min	Tex-203-F	45

1. Not used for acceptance purposes. Used by the Engineer as an indicator of the need for further investigation.
2. Unless otherwise shown on the plans.
3. Unless otherwise shown on the plans. Only applies to crushed gravel.
4. Aggregates, without mineral filler, RAP, or additives, combined as used in the job-mix formula (JMF).

TABLE 2
GRADATION REQUIREMENTS FOR FINE AGGREGATE

Sieve Size	% Passing by Weight or Volume
3 / 8"	100
#8	70 - 100
#200	0 - 3

Screenings shall be supplied from sources whose coarse aggregate meets the Los Angeles abrasion and magnesium sulfate soundness loss requirements shown in Table 1, unless otherwise shown on the Plans.

(d) Mineral Filler. Mineral filler shall consist of thoroughly dried stone dust, Portland cement, lime, fly ash, or other mineral dust approved by the City. The mineral filler shall be free from foreign matter.

When a specific type of mineral filler is specified on the Plans, fines collected by the baghouse or other air cleaning or dust collecting equipment shall not be used to meet this requirement. When mineral filler is not specifically required, the addition of baghouse or other collected fines will be permitted if the mixture quality is not adversely affected in the opinion of the City. In no

case shall the amount of material passing the No. 200 sieve exceed the tolerances of the job-mix formula or the master gradation limits.

When mineral filler is specified or allowed by the City, or baghouse fines are permitted to be added to the mixture, it shall be proportioned into the mix by a vane meter or an equivalent measuring device acceptable to the City. A hopper or other acceptable storage system shall be required to maintain a constant supply of mineral filler to the measuring device.

The measuring device for adding mineral filler shall be tied into the automatic plant controls so that the supply of mineral filler will be automatically adjusted to plant production and provide a consistent percentage to the mixture. When shown on the Plans, the measuring device for adding baghouse fines shall have controls in the plant control room, which will allow manual adjustment of feed rates to match plant production rate adjustments.

When mineral filler is used, it may not exceed 3% linear shrinkage when tested in accordance with Tex-107-E. When tested by Test Method Tex-200-F (Part I or Part III, as applicable), the mineral filler shall meet the gradation requirements of Table 3, unless otherwise shown on the Plans.

**TABLE 3
GRADATION REQUIREMENTS FOR MINERAL FILLER**

Sieve Size	% Passing by Weight or Volume
#8	100
#200	55 - 100

(2) Asphaltic Material.

(a) Paving Mixture. Asphalt cement for the paving mixture shall be of the grade shown on the Plans or designated by the City and shall meet the requirements of the Item 300, “Asphalts, Oils and Emulsions”. The Contractor shall notify the City of the source of the asphaltic material prior to design of the asphaltic mixture. This source shall not be changed during the course of the project without authorization of the City. Should the source of asphaltic material be changed, the moisture resistance of the new material combination will be evaluated to verify that the requirements of Section 340.3(1) are met.

(b) RAP Paving Mixture. When more than 20 percent RAP is used in the produce mixture, the asphalt in the RAP shall be restored to the properties indicated below. Restoration will be made by adding asphalt recycling agent and/or virgin asphalt cement meeting the requirements of Item 300, “Asphalts, Oils, and Emulsions”.

The mixture design will include recovery of asphalt from the RAP in accordance with Test Method Tex-211-F. The recovered asphalt shall be blended in the laboratory with the amount of asphalt cement and/or asphalt recycling agent selected for the project. The following tests shall be performed on the laboratory blend:

- i. Viscosity, 140° F, poises - Test Method Tex-528-C
- ii. Thin Film Oven Aging Test - Test Method Tex-510-C
- iii. Viscosity, 140° F, poises, on residue from the Thin Film Oven Aging Test - Test Method Tex-528-C
- iv. Penetration at 77° F, 100 g, 5 sec, on residue from the Thin Film Oven Aging Test - Test Method Tex-502-C

The viscosity in poises equivalent to the residue penetration at 77° F shall be calculated as set forth in Test Method Tex-535-C. The viscosity index of the residue shall then be calculated as follows:

$$\text{Residue Viscosity Index} = \frac{\text{Residue Viscosity, poises, equivalent to Penetration at 77°F}}{\text{Residue Viscosity, 140°F, poises}}$$

The aging index of the laboratory blended asphalt shall be determined as follows:

$$\text{Aging Index} = \frac{\text{Residue Viscosity, 140°F, poises}}{\text{Original Viscosity, 140°F, poises}}$$

The laboratory blended asphalt shall meet the following requirements:

- Residue Viscosity Index, maximum 1500
- Aging Index, maximum..... 3.0

Samples of asphalt recovered from plant produced mixture shall show the asphalt to meet the following requirements when tested in accordance with Test Methods Tex-211-F and Tex-502C:

	<u>Minimum</u>	<u>Maximum</u>
Penetration, 77°F, 100 g, 5 sec	30	55

(c) Tack Coat. Asphaltic materials, shown on the Plans or approved by the City, shall meet the requirements of Item, 300, “Asphalts, Oils and Emulsions”.

(3) Additives. Additives to facilitate mixing and/or improve the quality of the asphaltic mixture or tack coat shall be used when noted on the Plans or may be used with the authorization of the City.

Unless otherwise shown on the Plans, the Contractor may choose to use either lime or a liquid antistripping agent to reduce the moisture susceptibility of the aggregate. The evaluation and addition of antistripping agents will be in accordance with Item 301, “Asphalt Antistripping Agents”.

340.3. PAVING MIXTURES.

The paving mixtures shall consist of a uniform mixture of aggregate, hot asphalt cement, and additives if allowed or required.

An asphalt mixture design is a laboratory process, which includes the determination of the quality of the asphalt and the individual aggregates, the development of the job-mix formula, and the testing of the combined mixture.

The job-mix formula lists the quantity of each component to be used in the mix and the combined gradation of the aggregates used.

(1) Mixture Design. The Contractor shall furnish the City with representative samples of the materials to be used in production. Using these materials, the mix shall be designed in accordance with Construction Bulletin C-14 and Test Method Tex-204-F to conform with the requirements herein. Unless otherwise shown on the Plans, the City will furnish the mix design for the mixtures when using 20 percent or less RAP. The City may accept a design from the Contractor, which was derived using these design procedures.

The second and subsequent mixture designs, or partial designs, for each type of paving mixture which are necessitated by changes in the material or at the request of the Contractor will be charged to the Contractor when a rate is shown on the Plans.

The Contractor shall furnish the mixture design for all mixtures containing more than 20 percent RAP. This mixture design shall include, in addition to the results of the tests required for virgin mixes, the results of tests run on the proposed asphalt blend. The Contractor shall furnish the City with representative samples of all materials to be used in the proposed mixture. The City will verify the proposed mixture design. Should the City's tests find that the proposed mixture design does not meet the requirements of this specification, the Contractor shall furnish another mixture design.

The bulk specific gravity will be determined for each aggregate to be used in the design mixture. If the determined values vary by 0.30 or more, the Volumetric Method, Test Method Tex-204-F, Part II, will be used. The Bulk specific gravity of aggregates in RAP will be determined on extracted aggregates. Provide the Engineer with split samples of the mixtures and blank samples used to determine the ignition oven correction factors. The Engineer will determine the aggregate and asphalt correction factors from the ignition oven using Tex-236-F. The Engineer will use a Texas gyratory compactor calibrated in accordance with Tex-914-F in molding production samples. The Engineer will perform Tex-530-C and retain the tested sample for comparison purposes during production. The Engineer may waive the requirement for the boil test.

When properly proportioned, for the type specified, the blend of aggregates shall produce an aggregate gradation, which will conform to the limits of the master grading shown in Table 4. Unless otherwise shown on the Plans, the gradation of the aggregate will be determined in accordance with Test Method Tex-200-F, Part I (Dry Sieve Analysis), to develop the job-mix formula.

The master grading limits for the appropriate type and the proposed job-mix formula will be plotted on a gradation chart with sieve sizes raised to the 0.45 power. This plot must show that the promised job-mix formula is within the limits of the master grading. Gaps in gradation shown by this plot should be avoided.

TABLE 4
MASTER GRADING BANDS (% PASSING BY WEIGHT OR VOLUME)
AND VOLUMETRIC PROPERTIES

Sieve Size	Type				
	A Coarse Base	B Fine Base	C Coarse Surface	D Fine Surface	F Fine Mixture
1-1/2"	98 - 100	--	--	--	--
1"	78 - 94	98 - 100	--	--	--
3/4"	64 - 85	84 - 98	95 - 100	--	--
1/2"	50-70	--	--	98 - 100	--
3/8"	--	60 - 80	70 - 85	85 - 100	98 - 100
#4	30 - 50	40 - 60	43 - 63	50 - 70	80 -86
#8	22 - 36	29 - 43	32 - 44	35 - 46	38 - 48
#30	8 - 23	13 - 28	14 - 28	15 - 29	12 - 27
#50	3 - 19	6 - 20	7 - 21	7 - 20	6 - 19
#200	2 - 7	2 - 7	2 - 7	2 - 7	2 - 7
Design VMA¹, % Minimum					
--	12	13	14	15	16
Plant-Produced VMA, % Minimum					
--	11	12	13	14	15

1. Voids in Mineral Aggregates.

The voids in the mineral aggregate (VMA) will be determined as a mixture design requirement only, in accordance with Test Method Tex-207-F, and shall not be less than the value indicated in Table 4.

Unless otherwise shown on the Plans, the mixture of aggregate, asphalt and additives proposed for use will be evaluated in the design stage for moisture susceptibility, in accordance with Item 301, "Asphalt Antistripping Agents". The City may waive this test if a similar design, using the same ingredients, has proven satisfactory.

To substantiate the design, trial mixtures shall be produced and tested using all of the proposed project materials and equipment prior to any placement. The City may waive trial mixtures if similar designs have proven satisfactory.

(2) Density. The mixture should be designed to produce an acceptable mixture at optimum density of 96 percent, when tested in accordance with Test Method Tex-207-F and Test Method Tex-227-F. The operating range for control of laboratory density during production shall be optimum density plus or minus 1.5 percent.

Laboratory density is a mixture design and process control parameter. If the laboratory density of the mixture produced has a value outside the range specified above, the Contractor shall investigate the cause and take corrective action. If three consecutive test results fall outside the specified range, production shall cease unless test results or other information indicate, to the satisfaction of the City, that the next mixture to be produced will be within the specified range.

(3) Stability. The materials used in the mixture design shall produce a mixture with a stability value of at least 35, unless otherwise shown on the Plans, when tested in accordance with Test Method Tex-208-F.

If, during production, the stability value falls below the specified minimum, the City and the Contractor shall closely evaluate other test result values for specification compliance such as gradation, asphalt content, moisture content, crushed faces, etc., to determine the cause and take corrective action. If three consecutive test results fall below the minimum value specified, production shall cease unless test results or other information indicate, to the satisfaction of the City, that the next material to be produced will meet the minimum value specified.

(4) Job-Mix Formula Field Adjustments. The Contractor shall produce a mixture of a uniform composition closely conforming to the approved job-mix formula.

If, during initial days of production, it is determined that adjustments to the mixture design job-mix formula are necessary to achieve the specified requirements, or to more nearly match the aggregate production, the City may allow adjustment of the mixture design job-mix formula within the tolerances of Table 5 without a laboratory redesign of the mixture.

The Engineer will adjust the asphalt content to maintain desirable laboratory density near the optimum value while achieving other mix requirements.

(5) Types. The aggregate gradation of the job-mix formula shall conform to the master grading limits shown in Table 4 for the type mix specified on the Plans.

(6) Operational Tolerances. The gradation of the aggregate and the asphalt cement content of the produced mixture shall not vary from the job-mix formula by more than the tolerances shown in Table 5.

During production, the Contractor shall not exceed the operational tolerances in Table 5. Stop production if testing indicates tolerances are exceeded on:

- 3 consecutive tests on any individual sieve,
- 4 consecutive tests on any of the sieves, or
- 2 consecutive tests on asphalt content.

Begin production only when test results or other information indicate, to the satisfaction of the Engineer, that the next mixture produced will be within Table 5 tolerances.

TABLE 5
OPERATIONAL TOLERANCES

Description	Test Method	Allowable Difference from JMF Target
Individual % retained for #8 sieve and larger	Tex-200-F or Tex-236-F	$\pm 5.0^1$
Individual % retained for sieves smaller than #8 and larger than #200		$\pm 3.0^1$
% passing the #200 sieve		$\pm 2.0^1$
Asphalt content, %	Tex-236-F	$\pm 0.3^1$
Laboratory-molded density, %	Tex-207-F	± 1.0
VMA, %, min		Note 2

1. When within these tolerances, mixture production gradations may fall outside the master grading limits; however, the percent passing the #200 sieve will be considered out of tolerance when outside the master grading limits.
2. Test and verify that Table 4 requirements are met.

When disagreements concerning determination of specification compliance occur between allowed sampling and testing procedures, extracted aggregate testing shall take precedence over cold feed belt testing.

When cold feed belt samples are used for job control, the City will select the sieve analysis method that corresponds with the one used to determine the mixture design gradation. The tolerances will be adjusted as outlined in Test Method Tex-229-F.

340.4. EQUIPMENT .

(1) General. All equipment for the handling of all materials, mixing, placing and compacting of the mixture shall be maintained in good repair and operating condition and subject to the approval of the City. Any equipment found to be defective and potentially having a negative effect on the quality of the paving mixture or ride quality will not be allowed.

(2) Mixing Plants. Mixing plants may be the weigh-batch type, the modified weigh-batch type, the drum-mix type, or the specialized recycling type. All plants shall be equipped with satisfactory conveyors, power units, mixing equipment, aggregate handling equipment, bins and dust collectors.

Automatic proportioning devices are required for all plants and shall be in accordance with TxDOT Item 520, "Weighing and Measuring Equipment".

It shall be the Contractor's responsibility to provide safe and accurate means to enable inspection forces to take all required samples, to provide permanent means for checking the output of any specified metering device, and to perform calibration and weight checks as required by the City. When cold feed belt sampling is to be used for gradation testing, occasional stoppage of the belt may be necessary unless the City approves other means of sampling.

When using fuel oil heavier than Grade No. 2, or waste oil, the Contractor shall insure that the fuel delivered to the burner is at a viscosity of 100 SSU or less, when tested in accordance with Test Method Tex-534-C, to insure complete burning of the fuel. Higher viscosities will be allowed if recommended by the burner manufacturer. If necessary, the Contractor shall preheat the oil to maintain the required viscosity.

The Contractor shall provide means for obtaining a sample of the fuel, just prior to entry into the burner, in order to perform the viscosity test. The Contractor shall perform this test or provide a laboratory test report that will establish the temperature of the fuel necessary to meet the viscosity requirements. There shall be an in-line thermometer to check the temperature of the fuel delivered to the burner.

Regardless of the burner fuel used, the burner or combination of burners and types of fuel used shall provide a complete burn of the fuel and not leave any fuel residue that will adhere to the heated aggregate or become mixed with the asphalt.

(a) Weigh-Batch Type.

Cold Aggregate Bin Unit and Proportioning Device. The cold aggregate bin unit shall have at least four bins of sufficient size to store the amount of aggregate required to keep the plant in continuous operation and of proper design to prevent overflow of material from one bin to another. There shall be vertical partitions between each bin and on each end of the bins of sufficient height so that any overflow will be on to the front and back, and not allow overflow to the sides or between bins. Overflow that might occur shall not fall onto any feeder belt. The proportioning device shall not fall onto any feeder belt. The proportioning device shall provide a uniform and continuous flow of aggregate in the desired proportion to the dryer. Each aggregate shall be proportioned from a separate bin.

If RAP is used, a separate cold bin shall be required. The RAP feed system shall be equipped with a scalping screen to remove particles over two (2) inches in size. The cold bin system shall supply the proper amount of RAP to the weigh box. RAP will not be allowed in the hot bins.

When mineral filler is used, as specified in Section 340.2 (1)(d), an additional bin shall be provided.

Dryer. The dryer shall continually agitate the aggregate during heating. The temperature shall be controlled so that the aggregate will not be damaged in the drying and heating operations. The dryer shall be of sufficient size to keep the plant in continuous operation.

Screening and Proportioning. The screening capacity and size of the hot aggregate bins shall be sufficient to screen and store the amount of aggregate required to properly operate

the plant and keep the plant in continuous operation at full capacity. The hot bins shall be constructed so that oversize and overloaded material will be discarded through overflow chutes.

Provisions shall be made to enable inspection forces to have easy and safe access to the proper location on the mixing plant where representative samples may be taken from the hot bins for testing. The aggregate shall be separated into at least four (4) bins when producing Type "A", Type "B" or Type "C" mixtures, at least three (3) bins when producing Type "D" mixture and at least two bins when producing Type "F" mixture. These bins shall contain the following sizes of aggregates in percentages by weight or by volume, as applicable.

Type "A" (Coarse Graded Base Course):

Bin No. 1 - shall contain aggregates of which 85 to 100 percent will pass the No. 10 sieve.

Bin No. 2 - shall contain aggregates of which at least 85 percent will be of such size as to pass the 1/2 inch sieve and be retained on the No. 10 sieve.

Bin No. 3 - shall contain aggregates for which at least 85 percent will be of such size as to pass the 7/8 inch sieve and be retained on the 3/8 inch sieve.

Bin No. 4 - shall contain aggregates of which at least 85 percent will be of such size as to pass the 1-1/2 inch sieve and be retained on 7/8 inch sieve.

Type "B" (Fine-Graded Base):

Bin No. 1 - shall contain aggregates of which 85 to 100 percent will pass the No. 10 sieve.

Bin No. 2 - shall contain aggregates of which at least 70 percent will be of such size as to pass the No. 4 sieve and be retained on the No. 10 sieve.

Bin No. 3 - shall contain aggregates for which at least 75 percent will be of such size as to pass the 3/8 inch sieve and be retained on the No. 4 sieve.

Bin No. 4 - shall contain aggregates for which at least 75 percent will be of such size as to pass the 1 inch sieve and be retained on the 3/8 inch sieve.

Type "C" (Coarse-Graded Surface Course):

Bin No. 1 - shall contain aggregates of which 85 to 100 percent will pass the No. 10 sieve.

Bin No. 2 - shall contain aggregates of which at least 70 percent will be of such size as to pass the No. 4 sieve and be retained on the No. 10 sieve.

Bin No. 3 - shall contain aggregates for which at least 75 percent will be of such size as to pass the 3/8 inch sieve and be retained on the No. 4 sieve.

Bin No. 4 - shall contain aggregate of which at least 75 percent will be of such size as to pass the 7/8 inch sieve and will be retained on the 3/8 inch sieve.

Type “D” (Fine-Graded Surface Course):

Bin No. 1 - shall contain aggregates of which 85 to 100 percent will pass the No. 10 sieve.

Bin No. 2 - shall contain aggregates of which at least 70 percent will be of such size as to pass the No. 4 sieve and be retained on the No. 10 sieve.

Bin No. 3 - shall contain aggregates of which at least 75 percent will be of such size as to pass the 1/2 inch sieve and be retained on the No. 4 sieve.

Type “F” (Fine-Graded Mixture):

Bin No. 1 - shall contain aggregates of which 85 to 100 percent will pass the No. 10 sieve.

Bin No. 2 - shall contain aggregates of which at least 75 percent will be of such size as to pass the 3/8 inch sieve and be retained on the No. 10 sieve.

Aggregate Weigh Box and Batching Scale. The aggregate weigh box and batching scales shall be of sufficient capacity to hold and weigh a complete batch of aggregate. The weigh box and scales shall conform to the requirements of the TxDOT Item 520 “Weighing and Measuring Equipment”.

Asphaltic Material Measuring System. If an asphaltic material bucket and scales are used, they shall be of sufficient capacity to hold and weigh a complete batch of aggregate. The weigh box and scales shall conform to the requirements of TxDOT Item 520, “Weighing and Measuring Equipment.”

If pressure type flow meter is used to measure the asphaltic material, the requirements of TxDOT Item 520, “Weighing and Measuring Equipment”, shall apply. This system shall include an automatic temperature compensation device to insure a constant percent by weight of asphaltic material in the mixture.

Provisions of a permanent nature shall be made for checking the accuracy of the asphaltic material-measuring device. The asphalt line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for the asphaltic material.

Mixer. The mixer shall be of the pugmill type and shall have a capacity of not less than 3,000 pounds (of natural-aggregate mixer) in a single batch, unless otherwise shown on the Plans. Any mixer that has a tendency to segregate the aggregate or fails to secure a thorough and uniform mixture with the asphaltic material shall not be used. All mixers shall be provided with an automatic timer that will lock the discharge doors of the mixer for the required mixing period. The dump door or doors and the shaft seals of the mixer shall be tight enough to prevent spilling of aggregate or mixture from the pugmill.

Surge-Storage System and Scales. A surge-storage system may be used to minimize the production interruptions during the normal day's operations. A device such as a gab hopper or other device approved by the City to prevent segregation in the surge-storage bin shall be used. The mixture shall be weighed upon discharge from the surge-storage system.

When a surge-storage system is used, scales shall be standard platform truck scales or other equipment such as weigh hopper (suspended) scales and shall conform to TxDOT Item 520, "Weighing and Measuring Equipment". If truck scales are used, they shall be placed at a location approved by the City. If other weighing equipment is used, the City may require weight checks by truck scales for the basis of approval of the equipment.

Recording Device and Record Printer. The mixture shall be weighed for payment. If a surge-storage system is used, an automatic recording device and a digital record printer shall be provided to indicate the date, project identification number, vehicle identification, total weight of the load, tare weight of the vehicle, the weight of asphaltic mixture in each load and the number of loads for the day, unless otherwise indicated on the Plans. When surge-storage is not used, batch weights will be used as the basis for payment and automatic recording devices and automatic digital record printers in accordance with TxDOT Item 520, "Weighing and Measuring Equipment", shall be required.

(b) Modified Weigh-Batch Type.

General. This plant is similar to the weigh-batch type plant. The hot bin screens shall be removed and the aggregate control is placed at the cold feeds. The cold feed bins shall be the same as those required for the drum-mix type plant.

Cold-Aggregate Bin Unit and Feed System. The number of bins in the cold-aggregate bin unit shall be equal to or greater than the number of stockpiles of individual materials to be used.

The bins shall be of sufficient size to store the amount of aggregate required to keep the plant in continuous operation and of proper design to prevent overflow of material from one bin to another. There shall be vertical partitions between each bin and on each end of the bins of sufficient height so that any overflow will be to the front and back and not allow overflow to the sides or between bins. Overflow that might occur shall not fall onto any feeder belt. When required by the City, an approved stationary-scalping screen shall be placed on top of the field sand bin to eliminate roots and other objectionable material. The feed system shall provide a uniform and continuous flow of aggregate in the desired proportion to the dryer. The Contractor shall furnish a chart indicating the calibration of each cold bin in accordance with Construction Bulletin C-14 or other methods of cold bin calibration acceptable to the City.

When mineral filler is used, as specified in Section 340.2 (1)(d), an additional bin shall be provided.

If RAP is used, a separate cold bin shall be required. The RAP feed system shall be equipped with a scalping screen to remove particles over two (2) inches in size. The cold bin system shall supply a uniform and proper amount of RAP to the mixture. The RAP may be added at the weigh box, the system shall include means acceptable to the City to verify that the correct amount of RAP is continuously being fed.

Scalping Screen. A scalping screen shall be required after the cold feeds and ahead of the hot aggregate surge bins.

Dryer. The dryer shall continually agitate the aggregate during heating. The temperature shall be controlled so that the aggregate will not be damaged in the drying and heating operations. The dryer shall be of sufficient size to keep the plant in continuous operation.

Screening and Proportioning. The hot aggregate shall not be separated into sizes after being dried. There shall be one (1) or more surge bins provided between the dryer and the weigh hopper. Surge bins shall be of sufficient size to hold enough combined aggregate for one complete batch of mixture.

Aggregate Weigh Box and Batching Scale. The aggregate weigh box and batching scales shall be of sufficient capacity to hold and weigh a complete batch of aggregate. The weigh box and scales shall conform to the requirements of TxDOT Item 520, "Weighing and Measuring Equipment".

Asphaltic Material Measuring System. If an asphaltic material bucket and scales are used, they shall be of sufficient capacity to hold and weigh the necessary asphaltic material for one batch. The bucket and scales shall conform to the requirements of TxDOT Item 520, "Weighing and Measuring Equipment".

If a pressure type flow meter is used to measure the asphaltic material, the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", shall apply. This system shall include an automatic temperature compensation device to insure a constant percent by weight of asphaltic material in the mixture.

Provisions of a permanent nature shall be made for checking the accuracy of the asphaltic material measuring device. The asphalt line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for the asphaltic material.

Mixer. The mixer shall be of the pugmill type and shall have a capacity of not less than 3,000 pounds (of natural-aggregate mixture) in a single batch, unless otherwise shown on the Plans. Any mixer that has a tendency to segregate the aggregate or fails to secure thorough and uniform mixture with the asphaltic material shall not be used. All mixers shall be provided with an automatic timer that will lock the discharge doors of the mixer for the required mixing period. The dump door or doors and the shaft seals of the mixer shall be tight enough to prevent spilling of aggregate or mixture from the pugmill.

Surge-Storage System and Scales. A surge-storage system may be used to minimize the production interruptions during the normal day's operations. A device such as a gob hopper or other device approved by the City to prevent segregation in the surge-storage bin shall be used. The mixture shall be weighed upon discharge from the surge-storage system.

When a surge-storage system is used, scales shall be standard platform truck scales or other equipment such as weigh hopper (suspended) scales and shall conform to TxDOT Item 520, "Weighing and Measuring Equipment". If truck scales are used, they shall be placed at a location approved by the City. If other weighing equipment is used, the City may require weight checks by truck scales for the basis of approval of the equipment.

Recording Device and Record Printer. The mixture shall be weighed for payment. If a surge-storage system is used, an automatic recording device and a digital record printer shall be provided to indicate the date, project identification number, vehicle identification, total weight of the load, tare weight of the vehicle, the weight of asphaltic mixture in each load and the number of loads for the day, unless otherwise indicated on the Plans. When surge-storage is not used, batch weighs will be used as the basis for payment and automatic recording devices and automatic digital record printers in accordance with TxDOT Item 520, "Weighing and Measuring Equipment", shall be required.

(c) Drum Mix Type.

General. The plant shall be adequately designed and constructed for the process of mixing aggregates and asphalt. The plant shall be equipped with satisfactory conveyors, power units, aggregate-handling equipment and feed controls.

Cold-Aggregate Bin and Feed System. The number of bins in the cold-aggregate bin unit shall be equal to or greater than the number of stockpiles of individual materials to be used.

The bins shall be of sufficient size to store the amount of aggregate required to keep the plant in continuous operation and of proper design to prevent overflow of material from one bin to another. There shall be vertical partitions between each bin and on each end of the bins of sufficient height so that any overflow will be to the front and back and not allow overflow to the sides or between bins. Overflow that might occur shall not fall onto any feeder belt. When required by the City, an approved stationary-scalping screen shall be placed on top of the field sand bin to eliminate roots and other objectionable material. The feed system shall provide a uniform and continuous flow of aggregate in the desired proportion to the mixer. The Contractor shall furnish a chart indicating the calibration of each cold bin in accordance with Construction Bulletin C-14 or other methods of cold bin calibration acceptable to the City.

The system shall provide positive weight measurement of the combined cold aggregate feed by use of belt scales or other approved devices. Provisions of a permanent nature shall be made for checking the accuracy of the measuring device as required by TxDOT Item 520, "Weighing and Measuring Equipment". When a belt scale is used, mixture production shall be maintained so that the scale normally operates between 50 percent and 100 percent of its rated capacity. Belt scale operation below 50 percent of the rated capacity may be allowed by the City if accuracy checks show the scale to meet the requirements of TxDOT Item 520, "Weighing and Measuring Equipment", at the selected rate. It shall be satisfactorily demonstrated to the City that mixture uniformity and quality have not been adversely affected.

If RAP is used, a separate cold bin shall be required. The RAP feed system shall be equipped with a scalping screen to remove particles over two (2) inches in size prior to the weighing device. There shall be adequate cold bin controls to provide a uniform amount of RAP to the mixture.

When RAP is used, positive weight measurement of RAP shall be provided by the use of belt scales or other approved devices.

Scalping Screen. A scalping screen shall be required after the cold feeds and ahead of the combined aggregate belt scales.

Asphaltic Material Measuring System. An asphaltic material measuring device meeting the requirements of TxDOT Item 520, “Weighing and Measuring Equipment”, shall be placed in the asphalt line leading to the mixer so that the cumulative amount of asphalt used can be accurately determined. Provisions of a permanent nature shall be made for checking the accuracy of the measuring device output. The asphalt line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for the asphaltic material. The measuring system shall include an automatic temperature compensation device to maintain a constant percent by weight of asphaltic material in the mixture.

Synchronization Equipment for Feed-Control Systems. The asphaltic material feed-control shall be coupled with the total aggregate weight-measuring device to automatically vary the asphalt-feed rate in order to maintain the required proportion.

Mixing System. The mixing system shall control the temperature so that the aggregate and asphalt will not be damaged in the drying, heating and mixing operations. A continuously recording thermometer shall be provided which will indicate the temperature of the mixture as it leaves the mixer.

Surge-Storage System and Scales. A surge-storage system shall be used to minimize the production interruptions during the normal day's operations. A device such as a gob hopper or other device approved by the City to prevent segregation in the surge-storage bin shall be used. The mixture shall be weighed upon discharge from the surge-storage system.

Scales shall be standard platform truck scales or other equipment such as weigh hopper (suspended) scales and shall conform to TxDOT Item 520, “Weighing and Measuring Equipment”. If truck scales are used, they shall be placed at a location approved by the City. If other weighing equipment is used, the City may require weight checks by truck scales for the basis of approval of the equipment.

Recording Device and Record Printer. Automatic recording devices and automatic digital record printers shall be provided to indicate the date, project identification number, vehicle identification, total weight of the load, tare weight of the vehicle, the weight of asphaltic mixture in each load and the number of loads for the day in accordance with TxDOT Item 520, “Weighing and Measuring Equipment”, unless otherwise shown on the Plans.

(d) Specialized Recycling Type.

General. Alternate methods of heating may be used which will not abnormally age the asphalt cement. This type of plant shall be capable of continually producing a minimum of 150 tons per hour of completed asphalt mixture that will meet all the requirements of this specification.

Cold-Aggregate Bin Unit and Feed System. The cold aggregate feed system and controls shall meet all the requirements as listed under the drum-mix type plant.

Scalping Screen. A scalping screen shall be required after the cold feeds and ahead of the combined aggregate belt scales.

Dryer. The dryer shall continually agitate the RAP and aggregate during heating. The temperature shall be controlled so that the aggregate and asphalt will not be damaged in the drying and heating operations. The dryer shall be of sufficient size to keep the plant in continuous operation.

Asphaltic Material Measuring System. An asphaltic material measuring device meeting the requirements of Item 520, "Weighing and Measuring Equipment", shall be placed in the asphalt line leading to the mixer so that the cumulative amount of asphalt used can be accurately determined. Provisions of a permanent nature shall be made for checking the accuracy of the measuring device output. The asphalt line to the measuring device shall be protected with a jacket of hot oil or other approved means to maintain the temperature of the line near the temperature specified for the asphaltic material. The measuring system shall include an automatic temperature compensation device to maintain a constant percent by weight of asphaltic material in the mixture.

Synchronization Equipment for Feed-Control Systems. The asphaltic material feed-control shall be coupled with the total aggregate weight-measuring device to automatically vary the asphalt-feed rate in order to maintain the required proportion.

Mixer. The mixer shall be of the continuous mechanical mixing type. Any mixer that has a tendency to segregate the mixture or fails to secure a thorough and uniform mixture shall not be used. A continuously recording thermometer shall be provided which will indicate the temperature of the mixture as it leaves the mixer.

Surge-Storage System and Scales. A surge-storage system shall be used to minimize the production interruptions during the normal day's operations. A device such as a gob hopper or other device approved by the City to prevent segregation in the surge-storage bin shall be used. The mixture shall be weighed upon discharge from the surge-storage system.

Scales shall be standard platform truck scales or other equipment such as weigh hopper (suspended) scales and shall conform to TxDOT Item 520, "Weighing and Measuring Equipment". If truck scales are used, they shall be placed at a location approved by the City. If other weighing equipment is used, the City may require weight checks by truck scales for the basis of approval of the equipment.

Recording Device and Record Printer. Automatic recording devices and automatic digital record printers shall be provided to indicate the date, project identification number, vehicle identification, total weight of the load, tare weight of the vehicle, the weight of asphaltic mixture in each load and the number of loads for the day in accordance with TxDOT Item 520, "Weighing and Measuring Equipment", unless otherwise shown on the Plans.

(3) Asphaltic Material Heating Equipment. Asphaltic material heating equipment shall be adequate to heat the required amount of asphaltic material to the desired temperature. The heating apparatus shall be equipped with a continuously recording thermometer with a 24-hour chart that will record the temperature of the asphaltic material at the location of highest temperature.

(4) Spreading and Finishing Machine. The spreading and finishing machine shall be approved by the City and shall be meet the requirements indicated below.

(a) Screed Unit. The spreading and finishing machine shall be equipped with a heated compacting screed. It shall produce a finished surface meeting the requirements of the typical cross sections and the surface tests.

Extensions added to the screed shall be provided with the same compacting action and heating capability as the main screed unit, except for use on variable depth tapered areas and/or as approved by the City.

The spreading and finishing machine shall be equipped with an approved automatic dual longitudinal screed control system and automatic transverse screed control system. The longitudinal controls shall be capable of operating from any longitudinal grade reference including a stringline, ski, mobile stringline, or matching shoe.

The Contractor shall furnish all equipment required for grade reference. Personnel trained in the use of this type of equipment shall maintain it in good operating condition.

The grade reference used by the Contractor may be of any type approved by the City. Control points, if required by the Plans, shall be established for the finished profile in accordance with TxDOT Item 5, "Control of the Work". These points shall be set at intervals not be exceed 50 feet. The Contractor shall set the grade reference from the control points. The grade reference shall have sufficient support so that the maximum deflection shall not exceed 1/16 inch between supports.

(b) Tractor Unit. The tractor unit shall be equipped with a hydraulic hitch sufficient in design and capacity to maintain contact between the rear wheels of the hauling equipment and the pusher rollers of the finishing machine while the mixture is being unloaded.

No portion of the weight of hauling equipment, other than the connection, shall be supported by the asphalt paver. No vibrations or other motions of the loading equipment, which could have a detrimental effect on the riding quality of the completed pavement, shall be transmitted to the paver.

The use of any vehicle which requires dumping directly into the finishing machine and which the finishing machine cannot push or propel to obtain the desired lines and grades without resorting to hand finishing will not be allowed.

(5) Material Transfer Equipment. Equipment to transfer mixture from the hauling units or the roadbed to the spreading and finishing machine will be allowed unless otherwise shown on the Plans. A specific type of material transfer equipment shall be required when shown on the Plans.

(a) Windrow Pick-up Equipment. Windrow pick-up equipment shall be constructed in such a manner that substantially all the mixture deposited on the roadbed is picked up and loaded into the spreading and finishing machine. The mixture shall not be contaminated with foreign material. The loading equipment shall be designed so that it does not interfere with the spreading and finishing machine in obtaining the required line, grade and surface without resorting to hand finishing.

(b) Material Feeding System. Material feeding systems shall be designed to provide a continuous flow of uniform mixture to the spreading and finishing machine. When use of a material feeding system is required on the Plans, it shall meet the storage capacity, remixing capability, or other requirements shown on the Plans.

(6) Motor Grader. The motor grader, when used, shall be a self-propelled power motor grader and shall be equipped with smooth tread pneumatic tired wheels unless otherwise directed. It shall have a blade length of not less than 12 feet and a wheelbase of not less than 16 feet.

(7) Rollers. Rollers provided shall meet the requirements for their type as follows:

(a) Pneumatic Tire Rollers. The roller shall be acceptable medium pneumatic tire roller conforming to the requirements of Item 213, "Rolling (Pneumatic Tire)", Type B, unless otherwise specified on the Plans. Pneumatic-tire rollers used for compaction shall provide a minimum 80-psi ground contact pressure. When used for kneading and sealing the surface only, they shall provide a minimum of 55-psi ground contact pressure.

(b) Two Axle Tandem Roller. The roller shall be an acceptable self-propelled tandem roller weighing not less than eight (8) tons.

(c) Three Wheel Roller. This roller shall be an acceptable self-propelled three-wheel roller weighing not less than ten (10) tons.

(d) Three Axle Tandem Roller. This roller shall be an acceptable self-propelled three-wheel roller weighing not less than ten (10) tons.

(e) Trench Roller. This roller shall be an acceptable self-propelled trench roller equipped with a sprinkler for keeping the wheels wet and an adjustable road wheel so that the roller may be kept level during rolling. The drive wheel shall be not less than 20 inches wide. The roller under working conditions shall produce not less than 325 pounds per linear inch of roller width and be so geared that a speed of approximately 1.8 miles per hour is obtained in low gear.

(f) Vibratory Steel-Wheel Roller. This roller shall have a minimum weight of six (6) tons. The compactor shall be equipped with amplitude and frequency controls and shall be specifically designed to compact the material on which it is used.

(8) Straightedges and Templates. When directed by the City, the Contractor shall provide acceptable ten (10) foot straightedges for surface testing. Satisfactory templates shall be provided as required by the City.

(9) Alternate Equipment. When permitted by the City, equipment other than that specified which will consistently produce satisfactory results may be used.

340.5. STOCKPILING, STORAGE AND MIXING:

(1) Stockpiling of Aggregates.

(a) Weigh-Batch Plant. Prior to Stockpiling of aggregates, the area shall be cleaned of trash, weeds, grass and be relatively smooth and well drained. The stockpiling shall be done in a manner that will minimize aggregate degradation, segregation, mixing of one stockpile with another, and will not allow contamination with foreign material.

The plant shall have at least a two-day supply of aggregates on hand before production can begin and at least a two-day supply shall be maintained through the course of the project, unless otherwise directed by the City.

No stockpile shall contain aggregate from more than one source.

Coarse aggregates for mixture Types “A”, “B”, and “C” shall be separated into at least two stockpiles of different gradation, such as a large-coarse-aggregate and a small-coarse-aggregate stockpile, except when the use of large percentages of RAP preclude the need for two virgin coarse aggregate stockpiles.

When shown on the Plans, coarse aggregates for Type “D” mixtures shall also be separated into at least two stockpiles.

No coarse-aggregate stockpile shall contain more than 15 percent by weight of material that will pass a No. 10 sieve.

Fine-aggregate stockpiles may contain coarse aggregate in amounts up to 20 percent by weight. This requirement does not apply to stone screening stockpiles, which must meet the gradation requirements shown in Section 340.2 (1)(c), unless otherwise shown on the Plans.

Prior to starting RAP stockpiling operations, the Contractor shall develop and submit in writing to the City an acceptable stockpile production procedure and management plan, which will ensure that a homogeneous stockpile of RAP is available. Stockpiles of contractor-owned RAP material shall be completely established at the plant site prior to submission of mixture design samples and shall be of sufficient quantity to meet the material requirements of the project for which they are prepared. When shown on the Plans, plant site stockpiles composed of RAP from designated sources shall be of the minimum size shown on the Plans prior to submission of mixture design samples.

When required by the City, additional material shall not be added to stockpiles that have previously been sampled for approval.

Equipment of an acceptable size and type shall be furnished to work the stockpiles and prevent segregation and degradation of the aggregates.

(b) Modified Weigh-Batch Plant. The stockpiling requirements for aggregate shall be the same as required for a drum-mix type plant.

(c) Drum-Mix Plant. When a drum-mix plant is used, the following stockpiling requirements for coarse aggregates shall apply in addition to the aggregate stockpiling requirements listed under Section 340.5. (d)(a).

Once a job mix design has been established in accordance with Section 340.3, the virgin coarse aggregates delivered to the stockpiles shall not vary on any grading size fraction by more than plus or minus eight (8) percentage points from the percentage found in the samples submitted by the Contractor and upon which the job-mix formula was based. Should the gradation of virgin coarse aggregates in the stockpiles vary by more than the allowed tolerance, the City may stop production. If production is stopped, new aggregates shall be furnished that meet the gradations of the aggregates submitted for the design job-mix formula or a new mix design shall be formulated in accordance with Section 340.3.

When the volume of production from a commercial plant makes sampling of all coarse aggregate delivered to the stockpiles impractical, cold feeds will be sampled to determine stockpile uniformity. Should this sampling prove the stockpiles non-uniform beyond the acceptable tolerance, separate stockpiles which meet these specifications may be required.

(d) Specialized Recycling Plant. The stockpiling requirements for aggregate shall be the same as required for drum-mix type plant.

(2) Storage and Heating of Asphaltic Materials. The asphaltic material storage capacity shall be ample to meet the requirements of the plant. Asphalt shall not be heated to temperature in excess of that specified in the Item 300, "Asphalts, Oils, and Emulsions". All equipment used in the storage and handling of asphaltic material shall be kept in a clean condition at all times and shall be operated in such manner that there will be no contamination with foreign matter.

(3) Feeding and Drying of Aggregate. The feeding of various sizes of aggregate and RAP, if applicable, to the dryer shall be done through the cold aggregate bins and the proportioning device in such a manner that a uniform and constant flow of materials in the required proportions will be maintained. The aggregate shall be dried and heated to the temperature necessary to produce a mixture having the specified temperature.

(4) Mixing and Storage.

(a) Weigh-batch Mixer. In introducing the batch into the mixer, all aggregate shall be introduced first and shall be mixed thoroughly for a period of five (5) seconds to uniformly distribute the various sizes throughout the batch before the asphaltic material is added. The asphaltic material shall then be added and the mixing continued for a wet mixing period of not less than 15 seconds. The mixing period shall be increased if, in the opinion of the City, the mixture is not uniform or the aggregates are not properly coated.

Temporary storing or holding of the asphaltic mixture by the surge-storage system will be permitted during the normal day's operation. Overnight storage will not be permitted unless authorized in the Plans or by the City. The mixture coming out of the surge-storage bin shall be of equal quality to that coming out of the mixer.

(b) Modified Weigh-Batch Plant. The mixing and storage requirements shall be the same as is required for a standard weigh-batch plant.

(c) Drum Mix Plant. The amount of aggregate and asphaltic material entering the mixer and the rate of travel through the mixing unit shall be so coordinated that a uniform mixture of the specified grading and asphalt content will be produced.

Temporary storing or holding of the asphaltic mixture by the surge-storage system will be required during the normal day's operation. Overnight storage will not be permitted unless authorized in the Plans or by the City. The mixture coming out of the surge-storage bin shall be of equal quality to that coming out of the mixer.

(d) Specialized Recycling Plant. The mixing and storage requirements shall be the same as that stated for the drum-mix plant.

(e) Discharge Temperature. The City will select the target discharge temperature of the mixture between 250° F and 350° F. The mixture, when discharged from the mixer, shall not vary from this selected temperature more than 25° F, but in no case shall the temperature exceed 360° F.

(f) Moisture Content. The mixture produced from each type of mixer shall have a moisture content not greater than 1 percent by weight when discharged from the mixer, unless otherwise shown on the Plans and/or specified by the City. The moisture content shall be determined in accordance with Test Method Tex-212-F.

(g) RAP. If RAP is used, it shall be mixed and blended so that there is no evidence of unseparated particles in the mixture as it leaves the mixer.

340.6. CONSTRUCTION METHODS.

(1) General. It shall be the responsibility of the Contractor to produce, transport, place and compact the specified paving mixture in accordance the requirements herein.

The asphaltic mixture, when placed with a spreading and finishing machine or the tack coat shall not be placed when the air temperature is below 50° F and is falling, but it may be placed when the air temperature is above 40° F and is rising.

The asphaltic mixture, when placed with a motor grader, shall not be placed when the air temperature is below 60° F and is falling, but it may be placed when the air temperature is above 50° F and is rising.

The air temperature shall be taken in the shade away from artificial heat.

Mat thicknesses of 1 1/2 inches and less shall not be placed when the temperature of the surface on which the mat is to be placed is below 50° F.

Additional surface temperature requirements may be shown on the Plans.

It is further provided that the tack coat or asphaltic mixture shall be placed only when the humidity, general weather conditions and temperature and moisture condition of the base, in the opinion of the City, are suitable.

If, after being discharged from the mixer and prior to placing, the temperature of the asphaltic mixture is 50°F or more below the selected discharge temperature established by the City, all or any part of the load may be rejected and payment will not be made for the rejected material.

(2) Tack Coat. The surface upon which the tack coat is to be placed shall be cleaned thoroughly to the satisfaction of the City. The surface shall be given a uniform application of tack coat using asphaltic materials of this specification. This tack coat shall be applied with an approved sprayer at a rate not to exceed 0.05-gallon residual asphalt per square yard of surface, as directed by the City. Where the mixture will adhere to the surface on which it is to be placed without the use of a tack coat, the tack coat may be eliminated by the City. All contact surfaces of curbs and structures and all joints shall be painted with a thin uniform application of tack coat. During the application of tack coat, care shall be taken to prevent splattering of adjacent pavement, curb and gutter and structures. The tack coat shall be rolled with a pneumatic tire roller when directed by the City.

(3) Transporting Asphaltic Concrete. The asphaltic mixture shall be hauled to the work site in tight vehicles previously cleaned of all foreign material. The dispatching of the vehicles shall be arranged so that all material delivered is placed and all rolling completed during daylight hours unless otherwise shown on the Plans. In cool weather or for long hauls, covering and insulating of the truck bodies may be required. If necessary, to prevent mixture from adhering to the body the inside of the truck may be given a light coating of release agent satisfactory to the City.

(4) Placing.

(a) The asphaltic mixture shall be dumped and spread on the approved prepared surface with the specified spreading and finishing machine. When properly compacted, the finished pavement shall be smooth, of uniform texture and density and shall meet the requirements of the typical cross sections and the surface tests. In addition, the placing of the asphaltic mixture shall be done without tearing, shoving, gouging or segregating the mixture and without producing streaks in the mat.

Unloading into the finishing machine shall be controlled so that bouncing or jarring the spreading and finishing machine shall not occur and the required lines and grades shall be obtained without resorting to hand finishing, except as shown under Section 340.6.(4)(d).

Unless otherwise shown on the Plans, dumping of the asphaltic mixture in a windrow and then placing the mixture in the finishing machine with windrow pick-up equipment will be permitted. The windrow pick-up equipment shall be operated in such a manner that substantially all the mixture deposited on the roadbed is picked up and loaded into the finishing machine without contamination by foreign material. The windrow pick-up equipment will be so operated that the finishing machine will obtain the required line, grade and surface without resorting to hand finishing. Any operation of the windrow pick-up equipment resulting in the accumulation and subsequent shedding of accumulated material into the asphaltic mixture will not be permitted.

(b) When approved by the City, level-up courses may be spread with a motor grader.

(c) The spreading and finishing machine shall be operated at a uniform forward speed consistent with the plant production rate, hauling capability, and roller train capacity to result in a continuous operation. The speed shall be slow enough that stopping between trucks is not ordinarily required. If, in the opinion of the City, sporadic delivery of material is adversely affecting the mat, the City may require paving operations to cease until acceptable methods are provided to minimize starting and stopping of the paver.

The hopper flow gates of the spreading and finishing machine shall be adjusted to provide an adequate and consistent flow of material. These shall result in enough material being delivered to the augers so that they are operating approximately 85 percent of the time or more. The augers shall provide means to supply adequate flow of material to the center of the paver. Augers shall supply an adequate flow of material for the full width of the mat, as approved by the City. Augers should be kept approximately one-half to three-quarters full of mixture at all times during the paving operation.

(d) When the asphaltic mixture is placed in a narrow strip along the edge of an existing pavement, or used to level up small areas of an existing pavement, or placed in small irregular areas where the use of a finishing machine is not practical, the finishing machine may be eliminated when authorized by the City.

(e) Adjacent to flush curbs, gutters and structures, the surface shall be finished uniformly high so that when compacted it will be slightly above the edge of the curb or structure.

(f) Construction joints of successive courses of asphaltic material shall be offset at least six (6) inches. Construction joints on surface courses shall coincide with lane lines, or as directed by the City.

(g) If a pattern of surface irregularities or segregation is detected, the Contractor shall make an investigation into the causes and immediately take the necessary corrective action. With the approval of the City, placement may continue for no more than one full production day from the time the Contractor is first notified and while corrective actions are being taken. If the problem still exists after that time, paving shall cease until the Contractor further investigates the causes and the City approves further corrective action to be taken.

(5) Compacting.

(a) The pavement shall be compacted thoroughly and uniformly with the necessary rollers to obtain the compaction and cross section of the finished paving mixture meeting the requirements of the Plans and specifications.

(b) When rolling with the three wheel, tandem, or vibratory rollers, rolling shall start by first rolling the joint with the adjacent pavement and then continue by rolling longitudinally at the sides and proceed toward the center of the pavement, overlapping on successive trips by at least one foot, unless otherwise directed by the City. Alternate trips of the roller shall be slightly different in length. On super-elevated curves, rolling shall begin at the low side and progress toward the high side unless otherwise directed by the City.

When rolling with vibratory steel wheel rollers, equipment operation shall be in accordance with Item 217, "Rolling (Vibratory)", and the manufacturer's recommendations, unless otherwise directed by the City. Vibratory rollers shall not be left vibrating while not rolling or when changing directions. Unless otherwise shown on the Plans or approved by the City, vibratory rollers shall not be allowed in the vibrating mode on mats with a plan depth of less than 1-1/2 inches.

The motion of the rollers shall be slow enough to avoid other than usual initial displacement of the mixture. If any displacement occurs, it shall be corrected to the satisfaction of the City. The roller shall not be allowed to stand on pavement, which has not been fully compacted. To prevent adhesion of the surface mixture to the steel-wheel rollers, the wheels shall be kept thoroughly moistened with water, but an excess of water will not be permitted. Necessary precautions shall be taken to prevent the dropping of diesel, gasoline, oil, grease or other foreign matter on the pavement, either when the rollers are in operation or when standing.

(c) The edges of the pavement along curbs, headers and similar structures, and all places not accessible to the roller, or in such positions as will not allow thorough compaction with the rollers, shall be thoroughly compacted with lightly oiled tamps.

(d) Rolling with a trench roller will be required on widened areas, in trenches and other limited areas where satisfactory compaction cannot be obtained with the approved rollers.

(6) In-Place Compaction Control. In-place compaction control is required for all mixtures. Unless otherwise shown on the Plans, air void control shall be required.

(a) Air Void Control. The Contractor shall be responsible for determining the number and type of rollers to be used to obtain compaction to within the air void range required herein. The rollers shall be operated in accordance with the requirements of this specification and as approved by the City.

Unless otherwise shown on the Plans, rolling with pneumatic-tire roller to seal the surface shall be provided. Rolling with a tandem or other steel-wheel roller shall be provided if required to iron out any roller marks.

Asphaltic concrete shall be placed and compacted to contain from five (5) to nine (9) percent air voids. The percent air voids will be calculated using the maximum theoretical specific gravity of the mixture determined according to Texas State Department of Transportation Test Method Tex-227-F. Roadway specimens, which shall be either cores or sections of asphaltic pavement, will be tested according to Test Method Tex-207-F. The nuclear-density gauge or methods which correlate satisfactorily with results obtained from project roadway specimens may be used when approved by the City. Unless otherwise shown on the Plans, The Contractor shall be responsible for obtaining the required roadway specimens at his expense and in a manner and at locations selected by the City.

If the percent air voids in the compacted placement is greater than nine (9) percent but is ten (10) percent or less, production may proceed with subsequent changes in the construction operations and/or mixture. If the air void content is not reduced to between five (5) and nine (9) percent within one production day from the time the Contractor is notified, production shall cease. At that point, a test section as described below shall be required.

If the percent air voids is more than ten (10) percent, production shall cease immediately and a test section shall be required as described below.

In either case, the Contractor shall only be allowed to place a test section of one lane width, not to exceed 0.2 mile in length, to demonstrate that compaction to between five (5) and nine (9) percent air voids can be obtained. This procedure will continue until a test section with five (5) to nine (9) percent air voids can be produced. Only (2) two test sections per day will be allowed. When a test section producing satisfactory air void content is placed, full production may then resume.

Increasing the asphalt content of the mixture in order to reduce pavement air voids will not be allowed.

If the percent air voids is determined to be less than five (5) percent, immediate adjustments shall be made to the plant production by the Contractor, as approved by the City, within the tolerances as outlined in Section 340.3.(4), so that an adequate air void level results.

The Contractor is encouraged to perform supplemental compaction testing for his own information.

(b) Ordinary Compaction Control. When the requirement of air void control has been removed by plan note, one three-wheel roller, one pneumatic-tire roller, and one tandem roller shall be furnished for each compaction operation except as provided below or approved by the City. The City may waive the use of a tandem roller when the surface is already adequately smooth and further steel-wheel rolling is shown to be ineffective. With approval of the City, the Contractor may substitute a vibratory roller for the three-wheel roller and/or the tandem roller. Use of at least one pneumatic-tire roller is required. Additional or heavier rollers shall be furnished if required by the City.

Rolling patterns shall be established by the Contractor as outlined in Test Method Tex-207-F, Part IV, to achieve the maximum compaction, unless otherwise directed by the City. The selected rolling pattern shall be followed unless changes in the mixture or placement conditions occur which affect compaction. When changes in the mixture or placement conditions occur, a new rolling pattern shall be established.

(c) Compaction Cessation Temperature. Regardless of the method required for in-place compaction control, all rolling for compaction shall be completed before the mixture temperature drops below 175° F.

(7) Opening to Traffic. The pavement shall be opened to traffic when directed by the City. The Contractor's attention is directed to the fact that all construction traffic allowed on pavement open to the public will be subject to the State laws governing traffic on highways.

If the surface ravel, flushes, ruts or deteriorates in any manner prior to final acceptance of the work, it will be the Contractor's responsibility to correct this condition at his expense to the satisfaction of the City and in conformance with the requirements of this specification.

340.7. MEASUREMENT.

The quantity of Asphaltic concrete will be measured by the composite weight or composite volumetric method.

(1) **Composite Weight Method.** Asphaltic concrete will be measured by the ton of 2000 pounds of the composite “Asphaltic Concrete” of the type actually used in the completed and accepted work in accordance with the Plans and specifications for the project. The composite asphaltic concrete mixture is hereby defined as the asphalt, aggregate, RAP and additives as noted in the Plans and/or approved by the City.

If mixing is done by a drum mix plant or specialized recycling plant, measurement will be made on scales as specified herein.

If mixing is done by weigh-batch plant or modified weigh-batch plant, measurement will be determined on the batch scales unless surge-storage is used. Records of the number of batches, batch design and the weight of the composite “Asphaltic Concrete” shall be kept. Where surge-storage is used, measurement of material taken from the surge-storage bin will be made on truck scales or suspended hopper scales.

(2) **Composite Volumetric Method.** The asphaltic concrete will be measured by the cubic yard of compacted “Asphaltic Concrete” of the type actually used in the completed and accepted work in accordance with the Plans and specifications for the project. The composite asphaltic concrete mixture is hereby defined as the asphalt, aggregate, RAP and additives as noted in the Plans and/or approved by the City. The volume of the composite asphaltic concrete mixture shall be calculated by the following formula:

$$V = \frac{W}{62.4 (27) Ga}$$

V = Cubic yards of compacted “Asphaltic Concrete”

W = Total weight of asphaltic concrete in pounds

Ga = Average actual specific gravity of three molded specimens as prepared by Test Method Tex-206-F and determined in accordance with Test Method Tex-207-F.

If mixing is done by a drum-mix plant or specialized recycling plant, the weight “W” will be determined by scales as specified herein.

If mixing is done by a weigh-batch plant or modified weigh-batch plant and surge-storage is not used, weight will be determined by batch scales and records of the number of batches, batch designs and weight of asphalt and aggregate shall be kept. Where surge-storage is used, measurement of the material taken from the surge-storage bin will be made on truck scales or suspended hopper scales.

340.8 PAYMENT.

(1) The work performed and materials furnished in accordance with this Item and measured as

provided under “Measurement” will be paid for at the unit price bid for the “Asphaltic Concrete” of the type specified.

Measurement Method	Bid Item	Unit of Measure
Composite Weight	Asphaltic Concrete	Ton
Composite Volumetric	Asphaltic Concrete	Cubic Yard

The payment based on the unit bid price shall be full compensation for quarrying, furnishing all materials, additives, freight involved, for all heating, mixing, hauling, cleaning the existing base course or pavement, tack coat, placing, rolling and finishing asphaltic concrete mixture, transporting RAP from designated sources, transporting any excess RAP to locations shown on the Plans, and for all manipulations, labor, tools, equipment and incidentals necessary to complete the work.

(2) All templates, straightedges, core drilling equipment, scales and other weighing and measuring devices necessary for the proper construction, measuring and checking of the work shall be furnished, operated and maintained by the Contractor at his expense.

(3) City-owned RAP from sources designated on the Plans will be available at no cost to the Contractor.

CITY OF SAN ANGELO**ITEM 360****CONCRETE PAVEMENT****360.1. DESCRIPTION.**

This item shall govern for the construction of Portland Cement Concrete pavement with or without monolithic curbs on a prepared subgrade or sub-base course, in accordance with the typical sections shown on the Plans, the lines and grades established by the City and the requirements herein.

360.2. PAVING CONSTRUCTION PLAN.

The Contractor shall submit a paving construction plan for approval by the City prior to beginning pavement construction operations. The plan shall contain the mix design, methods of construction, and description of equipment to be used in mixing, placing, finishing, curing, and miscellaneous materials.

360.3. MATERIALS.

Unless otherwise shown on the Plans or required herein, all materials shall conform to the requirements of the pertinent Items of City of San Angelo Standard Specifications for Construction as follows:

- Item 300, "Asphalts, Oils, and Emulsions"
- Item 420, "Concrete Structures"
- Item 421, "Portland Cement Concrete"
- Item 433, "Joint Sealants and Fillers"
- Item 437, "Concrete Admixtures"
- Item 440, "Reinforcing Steel"
- TxDOT Item 526, "Membrane Curing"

except for the following:

(1) Portland Cement Concrete. Classification and mix design shall conform to Class "P" Portland Cement Concrete as defined in Item 421, "Portland Cement Concrete", unless otherwise shown on the Plans.

(2) Joint Sealants and Fillers. These materials shall be of the size, shape and type shown on the Plans.

Unless otherwise shown on the Plans, the joint sealant materials to be used shall be self-leveling silicone pavement sealant as manufactured by Dow Corning, Crafc0, Inc., or approved equal.

(3) Dowels for Expansion and Contraction Joints. Dowels shall be smooth, straight steel dowels of the size and type shown on the Plans and shall conform to the requirements of ASTM A615, Grade 60. The free end of dowels shall be smooth and free of burrs.

Coat dowels with a thin film of grease or other approved de-bonding material. Provide dowel caps on the lubricated end of each dowel bar used in an expansion joint. Provide dowel caps filled with a soft compressible material with enough range of movement to allow complete closure of the expansion joint.

(4) Positioning and Support Devices for Reinforcement and Joint Assemblies. These devices shall be of sufficient structural quality to prevent movement of the dowels or steel reinforcement during concrete placement and finishing. The devices shall be a type approved by the City.

Positioning and supporting devices (chairs) for steel reinforcement bars shall be either plastic or metal and of sufficient number to maintain the position of the bars within the allowable tolerances.

(5) Reinforcing Steel. ASTM A616 Grade 60 will be permitted for straight bars only. Reinforcing steel that requires bending shall be ASTM 615 Grade 40 with the spacing reduced to two thirds (2/3) of that shown for Grade 60 reinforcing steel. When shown on the Plans, corrosion protection shall be applied to dowels and tie bars.

(a) Tie Bars. Tie bars at weakened plane longitudinal joints shall be straight reinforcing bars. Tie bars at longitudinal construction joints shall be either multiple piece tie bars or straight reinforcing bars, when equipment or conditions permit.

(b) Multiple Piece Tie Bars. Multiple piece tie bars (threaded coupling or other adequate devices) shall develop a tensile strength over their entire length equal to 1-1/4 times the yield strength of the tie bars shown. Each end of multiple piece tie bars shall consist of deformed reinforcement of at least the size shown on the Plans, conforming to City of San Angelo Item 440, "Reinforcing Steel".

360.4. EQUIPMENT.

(1) General. All equipment shall be maintained in good condition and approved by the City before the Contractor will be permitted to begin construction of the pavement. Weighing, measuring equipment and mixer at Portland Cement Concrete Plant, hauling equipment, agitator trucks, grade control equipment, shall conform to the requirements set forth in applicable City of San Angelo Specifications relative to production and installation of Portland Cement Concrete Pavement.

(2) Forms.

(a) Side Forms. Side forms shall be of metal except as otherwise provided herein and shall be of approved cross section. The length of form sections shall not be less than ten (10) feet, and each section shall provide for staking in position with not less than three (3) pins. Forms shall be of ample strength and shall be provided with adequate devices to secure them in place so the forms

will withstand, without visible springing or settlement, the impact and vibration of the spreading and finishing machinery. In no case shall the base of the form be less than eight (8) inches wide for a form depth of eight (8) inches or more in height. The forms shall be free from warps, bends or kinks, and shall be sufficiently true to provide a reasonably straight edge on the concrete.

Flexible or curved forms of wood or metal of proper radius shall be used for curves of 100-foot radius or less.

(b) Curb Forms. Outside curb forms shall be of wood or metal of a section satisfactory to the City, straight, free of warp, and shall be of a depth at least equal to the depth of the curb. They shall be securely mounted on the paving forms and maintained in true position during the placing of the concrete. Inside curb forms, if required, shall be of approved material and of such design as to provide the curb required and shall be rigidly attached to the outside curbs forms.

(3) Equipment for Spreading, Consolidating, Finishing Surface Texturing Concrete shall conform to the requirements set forth in Item 360, "Concrete Pavement".

360.5. QUALITY OF CONCRETE.

The quality of concrete shall be in accordance with Item 421, "Portland Cement Concrete".

360.6. SUBGRADE.

(1) **Preparation of Subgrade or Subbase.** The concrete pavement shall be constructed on prepared subgrade. When Slip Form equipment is used, a firm subgrade or subbase (stabilized or unstabilized) shall be maintained outside the limits of the pavement for the support of the Slip Form equipment. Refer to Item 200, "Subgrade Preparation" for additional information.

360.7. PLACEMENT OF REINFORCING STEEL AND JOINT ASSEMBLIES.

All reinforcing steel, including steel wire fabric reinforcement, tie bars, dowel bars, and load transmission devices shall be accurately placed and secured in position in accordance with Item 440, "Reinforcing Steel", and additional requirements set forth in Item 360, "Concrete Pavement".

360.8. CONCRETE MIXING AND PLACING.

(1) **Mixing.** Concrete mixing shall be in conformance with Items 421, "Portland Cement Concrete", and TxDOT Item 522, "Portland Cement Concrete Plants".

(2) **Workability of Concrete.** The concrete shall be workable, cohesive, possess satisfactory finishing qualities, and have a consistency conforming to the specified slump requirements. If detrimental bleeding occurs and this condition cannot be corrected by reasonable re-proportioning of the ingredients, the bleeding shall be immediately corrected by one or more of the following listed measures:

Redesign of the batch.

Addition of mineral filler to fine aggregate.

Increase of cement content.
Use of appropriate approved admixture.

When, in the opinion of the City, excessive bleeding occurs and corrective actions do not satisfactorily reduce bleeding, concrete placement operations shall cease until the concrete mixture has been redesigned.

When the method of transporting concrete produces excessive segregation and/or bleed water on the surface of the concrete, the method used shall be discontinued and a satisfactory method shall be provided. Such segregated concrete will be subject to rejection as directed by the City.

(3) Placing. Unless otherwise shown on the Plans, the concrete shall be placed using either forms or a slipform paver. Any concrete not placed as herein prescribed within the time limits specified will be rejected.

The Contractor shall provide a system satisfactory to the City for determining that concrete delivered to the site meets the specified requirements for mixing and time of placing as outlined under Item 360, "Concrete Pavement".

The concrete shall be placed as near as possible to its final location and in such manner as to minimize segregation and re-handling. Where hand spreading is necessary, concrete shall be distributed to the required depth by use of shovels. The use of rakes will not be permitted. Concrete shall be placed, consolidated and finished to conform to the required section and grade.

(a) Double Strike-Off Method. Unless otherwise shown on the Plans, when concrete placement is accomplished in two (2) lifts (double strike-off method) to allow placing the reinforcement after the first lift, the first lift shall be uniformly spread and/or struck off so that the final position of the longitudinal steel will be within one half (1/2) inch of the position shown on the Plans. The second lift shall be placed as soon as reinforcing steel is in place and prior to initial set of the first lift. The second lift shall not be placed later than 20 minutes after strike-off of the first lift.

(b) Placing Curbs. Where curbs are placed monolithically concrete for monolithic curbs shall be the same as for the pavement and must be placed while the pavement concrete is still plastic.

Where curbs are placed separately, they shall be placed in conformance with TxDOT Item 410, "Concrete Curb, and Gutter, Valley Gutter, Alley Apron, Driveways and Sidewalks".

(4) Consolidation. All concrete placed for pavement shall be consolidated by approved mechanical vibrators operated ahead of the finishing machine. Unless otherwise shown on the Plans, pan type vibrators shall be used for double lift placement of concrete and the immersion type vibrators shall be used for full-depth placement, unless otherwise approved by the City. Vibratory equipment shall extend across the pavement, but shall not come in contact with the side forms. Mechanically operated vibrators shall be mounted and operated in such manner as not to interfere with the transverse or longitudinal joints. Hand operated vibrators shall be used to consolidate concrete in areas not accessible to the machine mounted vibrators.

360.9. JOINTS.

(1) General. All transverse and longitudinal joints, when required in the pavement, shall be of the type or alternate type shown on the Plans and shall be constructed at the required location and alignment, in relationship to the tie bars and joint assemblies, and in accordance with details shown on the Plans. Stakes, braces, brackets or other devices shall be used as necessary to keep the entire joint assembly in true vertical and horizontal position.

Careful workmanship shall be exercised in the construction of all joints to insure that the concrete sections are completely separated by an open joint or by the joint materials and to insure that the joints will be true to the required section. Joints shall be cleaned and sealed in accordance with Item 438, "Cleaning and/or Sealing Joints and Cracks (Portland Cement Concrete)". The sequence of construction of joints if deemed necessary shall be approved by the City.

Excessive spalling of the joint groove shall be repaired to the satisfaction of the City prior to the installation of the sealant.

When sawed joints are used, they shall be sawed to the depth as shown on the Plans as soon as sawing can be accomplished without damage to the pavement. Once sawing has commenced it shall be continued until completed and all such sawing must be completed within 12 hours of placement. Sawing must be accomplished even in rain and cold weather. Should the sawing for any day's placement fail to be completed within 12 hours, the following concrete placement shall be limited to the amount that was sawed on time. This limitation shall continue until the sawing crew demonstrates it can handle a larger volume of sawing. If marring of the surface occurs, the City may extend the 12-hour limit.

The Contractor shall keep a standby power driven concrete saw on the project at all times when concrete operations are under way.

When membrane curing is used, the part of the seal, which has been disturbed by sawing operations, shall be re-sprayed by the Contractor with additional curing compound.

(2) Expansion Joints. Transverse expansion joints shall be constructed in accordance with the details shown on the Plans. After the finishing machine and before the carpet drag and tining machines have passed over the joint the Contractor shall inspect the joint filler for correctness of position. The Contractor shall make any required adjustment in position of the filler and shall install the joint seal space form in accordance with the Plans. The concrete faces of the joint seal space shall be left true to line and section throughout the entire length of the joint.

(3) Weakened Plane Joints. Weakened plane joints shall consist of transverse contraction joints and longitudinal joints. Unless otherwise shown on the Plans, the transverse joints shall be formed or sawed perpendicular to the centerline and surface of the pavement.

The joints shall be constructed in the sequence of operations, as shown on the Plans.

Chalk line, string line, sawing template or other approved methods shall be used to provide a true joint alignment.

(4) Transverse Construction Joints.

When the placing of concrete is stopped, a bulkhead of sufficient cross sectional area to prevent deflection, accurately notched to receive the load transmission devices and shaped accurately to the cross section of the pavement shall be provided.

Intentional stoppage of the placing of concrete shall be either at an expansion joint or at a weakened plane joint, when load transmission devices are shown on the Plans. When the design for load transmission does not include dowels, intentional stoppage shall be in the middle of a slab.

When an unintended stoppage of the placing of concrete occurs, the Contractor shall immediately place the available concrete to a line and install the above described bulkhead at right angles to the centerline of the pavement, perpendicular to the surface and at the required elevation. Concrete shall be placed and finished to this bulkhead. Any concrete remaining on the subgrade ahead shall be removed and disposed of as directed by the City. When placement of concrete is resumed before the concrete has set to the extent that the concrete will stand on removal of the bulkhead, the new concrete shall be consolidated with the first. The edge created by construction joints of this type shall have a joint seal space and shall be sealed as required for contraction joints.

At transverse construction joints in continuously reinforced concrete pavement, the reinforcement or load transmission device immediately beyond the joint will be protected against vibration or impact by the Contractor until paving resumes.

(5) Longitudinal Construction Joints. Longitudinal construction joints shall be of the type and at the locations shown on the Plans.

(6) Joint Filler Boards. Joint filler boards shall be of the size, shape and type as shown on the Plans. Boards shall be anchored by appropriate methods against their displacement while placing concrete.

(7) Curb Joints. Joints in the curb shall be provided and shall be of the same type and location as the adjacent pavement. The expansion joint material shall be of the same thickness, type and quality as specified for the pavement. All expansion joints shall be carried through the curb.

When transverse sawed joints are provided for the pavement, the curb placement shall be delayed until all transverse joints in the pavement have been sawed. Dowel bars shall be placed as shown on the Plans while the pavement concrete is still plastic, unless otherwise approved by the City. The weakened plane joint in the monolithic curbs may be formed or sawed.

360.10 SPREADING AND FINISHING.

(1) Machine-Finishing. All concrete pavement shall be finished with approved self-propelled machines.

The consistency of the concrete as placed should allow the completion of all finishing operations without the addition of water to the surface. When field conditions require additional moisture for the final concrete surface finishing operation, the water shall be applied to the surface by a fine, light fog mist and the amount of water added shall be held to a minimum.

When required by the City, the Contractor shall perform sufficient checks with a long handled ten (10) foot straightedge on the plastic concrete to insure that the final surface will be within the tolerances specified below. The check shall be made with the straightedge parallel to the centerline. Each pass thereof shall lap half of the preceding pass. All high spots shall be removed and all depressions over 1/16-inch in depth shall be filled with fresh concrete and floated. The checking and floating shall be continued until the surface is true to grade and free of depressions, high spots, voids and rough spots.

Final finish shall consist of a combination of a carpet drag and metal tine finish, unless otherwise shown on the Plans. Final finish shall be completed before the concrete has attained its initial set.

The final finish shall be accomplished by first drawing the specified carpet drag longitudinally along the pavement. The actual contact surface shall be regulated so that a coarse texture satisfactory to the City is obtained.

Immediately following the carpet drag, the pavement surface shall be given a transverse metal-tine finish. The metal-tine device shall be operated to obtain randomly spaced grooves approximately 3/16-inch deep, with minimum depth of 1/8-inch and approximately .083 inch wide. Successive passes of the tines shall not overlap a previous pass. Manual methods for achieving similar results may be used on ramps and other irregular sections of pavement.

After completion of texturing, the edge of the slab and joints shall be carefully finished as directed by the City.

(2) Hand Finishing. Hand finishing if permitted shall conform to the requirements specified herein.

When hand finishing is permitted, the concrete shall be struck off with an approved strike-off screed to such elevation that, when consolidated and finished, the surface of the pavement shall conform to the required section and grade.

The pavement shall be straightedged prior to final finishing. Other operations and surface tests shall be as required for machine finishing.

360.11. CURING.

(1) General. All concrete pavement shall be cured for a period of not less than 72 hours from the beginning of curing operations. All exposed surfaces, including vertical surfaces of the placed concrete, shall be cured immediately after finishing operations have been completed, in accordance with the requirements specified herein.

Failure to maintain adequate curing shall be cause for immediate suspension of concreting operations.

The applied curing material may be removed as necessary to saw joints or to comply with the requirements for any surface test. The hardened concrete surface shall be maintained wet with a water spray, if required, and the curing material replaced immediately after completion of sawing, testing and any required surface correction.

(2) Polyethylene Film Curing. After the final finish and the concrete surface has attained initial set, the concrete surface shall be wetted with water, applied in the form of a fine spray and covered with the polyethylene film so placed and weighted as to remain in direct contact with the surface. The polyethylene film blanket shall be maintained in place continuously for not less than the specified curing period.

All joints shall be sealed in a manner acceptable to the City to provide a moisture-proof lap.

The polyethylene film blankets shall be adequately weighted to prevent displacement or billowing due to wind and the film folded down over the side of the pavement shall be secured by a continuous bank of earth or other approved material. Plowing of this windrow into place will not be permitted. Use of polyethylene film holes and cuts are not acceptable.

(3) Membrane Curing. After final finish and immediately after the free surface moisture has disappeared, the concrete surface shall be sprayed uniformly with a curing compound in accordance with the requirements set forth in TxDOT Item 526, "Membrane Curing".

Special care shall be taken to insure that the sides of the tining grooves are coated with the curing compound.

360.12. PROTECTION OF PAVEMENT AND OPENING TO TRAFFIC.

The pavement shall be closed to all traffic, including vehicles of the Contractor, until the concrete is at least four (4) days old. This period of closure to all traffic may be extended if in the opinion of the City, weather or other conditions may require an extension of the time of protection. When Type II cement is used one (1) additional day shall be required for a total of five (5) days.

At the end of this period the pavement may be opened for use by vehicles of the Contractor provided the gross weight (vehicle plus load) of such vehicles and/or equipment does not exceed 14,000 pounds. Such opening, however, shall in no manner relieve the Contractor from his responsibility for overall safety of the traffic and the general public.

On those sections of the pavement to be opened to traffic, all joints shall first be sealed and the pavement cleaned. Unless otherwise shown on the Plans, stable material shall be placed against the pavement edges before permitting vehicles thereon.

After the concrete in any section of pavement is seven (7) days old, such section of pavement may be opened to all traffic as directed by the City. When Type II cement is used one (1) additional day shall be required for a total of eight (8) days. For those sections of the pavement to be opened to traffic, all joints shall first be sealed, the pavement cleaned, stable material placed against the pavement edges unless otherwise shown on the Plans and all other work performed as required for the safety of traffic. Such opening, however, shall in no manner relieve the Contractor from his responsibility for overall safety of the traffic and the general public.

360.13. MEASUREMENT.

This Item will be measured by one of the following methods:

(1) Measurement by the Square Yard. When provided under this item, concrete pavement will be measured by the square yard of surface area of completed and accepted work. When concrete pavement is to be measured by the square yard and monolithic curb is required, monolithic curb will be considered as part of pavement. Surface area of the pavement will be computed by measuring dimensions to the back of monolithic curb.

If curb and gutter is poured separately, it shall be measured and paid in accordance with Item 529, "Concrete Curb, Gutter, and combined Curb and Gutter". Gutter, Alley Apron, Driveways, and Sidewalks and will not be included in the area of concrete pavement.

(2) Measurement by the Cubic Yard. When provided by this Item, concrete pavement, including monolithic curb when required, will be measured by the cubic yard computed on the basis of design depth of concrete pavement shown on the Plans.

360.14. PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for this Item. This price shall be full compensation for furnishing concrete; for placing and adjusting forms; for furnishing and installing all reinforcing steel; for furnishing all materials for sealing joints and placing longitudinal, expansion and weakened-plane joints, including all steel dowel caps and load transmission devices required; for mixing, placing, finishing, curing and sawing concrete; for cleaning and sealing concrete joints; and for all manipulations, labor, tools, equipment and incidentals necessary to complete the work.

CITY OF SAN ANGELO**ITEM 400****EXCAVATION AND BACKFILL FOR STRUCTURES****400.1 DESCRIPTION.**

This Item shall govern for the excavation bedding, backfill and/or Portland Cement Stabilized Backfill required for the construction of all structures. This Item shall also govern for any necessary sloping, pumping or bailing, for drainage, and for all sheeting and bracing of excavation walls up to five feet in depth. Excavation greater than five (5) feet in depth shall be protected as specified in TxDOT Item 402, "Trench Excavation Protection" or TxDOT Item 403, "Temporary Special Shoring". Unless otherwise provided, the work included herein shall provide for the removal of old structures or portions thereof (abutments, wingwalls, piers, house foundations; old sewers, sewer appurtenances, etc.), trees and all other obstructions to the proposed construction, the blocking of the ends of abandoned sewers cut and left in place, and the protection of existing utilities. Also governed by this Item are the cutting and restoration of pavement and base courses, the construction and removal of any required cofferdams, the hauling and disposition of surplus materials and the bridging of trenches and other provisions for maintenance of traffic or access.

400.2 EXCAVATION.

(1) General. Excavation shall conform to the lines and grades shown on the Plans or as directed by the City.

When trench and/or negative projecting conditions for concrete pipe culverts are required by design, an excavation diagram will be shown on the Plans. These limits of excavation shall not be exceeded.

(a) Disposal of Excavation. All materials from excavation operations not required for backfilling and that are considered satisfactory, may be placed in embankment in accordance with Item 132, "Embankment". All excess material or material not satisfactory for use in Embankment will become the property of the Contractor. All surplus material shall be removed from the work site promptly following the completion of the portion of the structure involved and disposed of in a manner satisfactory to the City and by permit from the City.

Whenever excavation is made for installing structures across private property or beyond the limits of the embankment, the top soil removed in the excavation shall be kept separate and replaced, as nearly as feasible.

(b) Excavation in Streets. Where structures are installed in streets, highways or other paved areas, the work shall include the cutting of pavement and base to neat lines and the restoration of pavement structure after structural excavation and backfill are completed. The type and thickness of replacement materials shall be as shown on the Plans. Any work done or any damage to base

and/or pavement incurred outside the limits shown on the Plans or authorized by the City, will not be measured for payment, but shall be restored at the Contractor's expense. Maintenance and control of traffic shall be in accordance with the approved traffic control plan and Manual on Uniform Traffic Control Devices.

(c) Protection of Utilities. The Contractor shall conduct his work with a minimum disturbance of existing utilities and it shall be his responsibility to coordinate all work in or near the utilities with the utility owners. The Contractor shall inform utility owners sufficiently in advance of his operations to enable them to identify and locate, reroute, provide temporary detours, or to make other adjustments to utility lines in order that work may proceed with a minimum of delay. The Contractor shall cooperate with all utility owners concerned for any utility adjustments necessary.

Particular care shall be exercised to avoid the cutting or damaging of underground utility lines that are to remain in place. Such lines if damaged shall be restored promptly. When active sanitary sewer lines are cut during excavation operations temporary flumes shall be provided across the excavation, while open, and the lines shall be restored when the backfilling has progressed to the original bedding lines of the cut sewer.

(d) Removing Old or Abandoned Structures. When old or abandoned structures or foundations are encountered in the excavation, such obstructions shall be removed for the full width of the excavation and to a depth of one (1) foot below the bottom of the excavation. When old inlets or manholes are encountered and no plan provision is made for adjustment or connection to the new structures, such manholes and inlets shall be removed completely to a depth one (1) foot below the bottom of the excavation. In each instance, the bottom of the excavation shall be restored to grade by backfilling and compacting by the methods provided hereinafter for backfill. Where the excavation cuts through abandoned sewers, these sewers shall be removed as required to clear the new structure and plugged in a manner approved by the City.

(e) Dewatering of Excavation Area. Structures shall not be constructed or laid in the presence of water unless approved by the City. Setting of precast members, placement of concrete, or pipe placing operations shall be performed on a dry firm bed. This shall be accomplished by removal of water from the surface of the bed by bailing, pumping, wellpoint installation, deep wells, drench drains, or any other method approved by the City.

For foundations placed in the presence of water, when approved by the City, pumping or bailing from the interior of any foundation enclosure shall be done in a manner which precludes the possibility of movement of water through or alongside any concrete being placed. No pumping or bailing will be permitted during the placing of structural concrete or for a period of at least 36 hours thereafter, unless from a suitable sump separated from the concrete work. Pumping or bailing during placement of seal concrete shall be only to the extent necessary to maintain a static head of water within the cofferdam. Pumping or bailing to dewater a sealed cofferdam shall not be started until the seal has aged at least 36 hours.

In the event that the excavation cannot be dewatered to the point where the subgrade is free of mud, or it is difficult to keep the reinforcing steel clean in cast-in-place structures, a special material shall be used in the bottom of the excavation. Such special material shall be a minimum depth of three

inches and shall consist of a lean concrete mixture (not less than three (3) sacks of cement per cubic yard), or other material approved by the City.

(2) Bridge Foundations and Retaining Walls. To determine the adequacy of a proposed foundation, the City may require the Contractor to make soundings or take cores to determine the character of the subgrade materials. The maximum depth of soundings or cores will not exceed five (5) feet below the proposed footing grade.

Care shall be taken not to disturb the material below the bottom of footing grade. Backfilling in a foundation to compensate for excavation which has extended below grade will not be permitted. Such areas below grade shall be filled with concrete at the time the footing is placed. The additional concrete involved shall be at the Contractor's expense.

Unless otherwise required herein or on the Plans, rock or other hard foundation material shall be free from all loose material, clean, and cut to a firm surface which may be level, stepped, or serrated, as directed by the City. All seams shall be cleaned out and filled with concrete at the time the footing is placed.

When the material encountered at footing grade of a retaining wall, bridge bent or pier is found to be partially of rock or incompressible material and partially of a compressible material, the foundation shall not be placed until the City has inspected the footing and authorized necessary changes to provide a uniform bearing condition.

(3) Culverts. For all single and multiple box culverts, pipe culverts, pipe arch culverts, long span structural plate structures, box sewers, and pipe sewers where the soil encountered at established footing grade is an unstable or incompressible material, the following procedure shall be used unless other methods are called for on the Plans:

Unstable material shall be removed to a depth not to exceed two (2) feet below the footing of the structure unless additional depth is authorized by the City. All soil removed shall be replaced with stable material in uniform layers not to exceed eight (8) inches in depth (loose measurement). Each layer shall have sufficient moisture to be compacted by rolling or tamping as required to provide a stable foundation for the structure.

When it is not feasible to construct a stable footing as outlined above, the Contractor shall use special materials, such as flexible base, cement stabilized base, cement stabilized backfill or other material, as directed by the City. This work will be paid for as provided in Article 400.8. Special material used, or additional excavation made, for the Contractor's convenience to expedite the work, will be at the Contractor's expense.

When the material encountered at the footing grade of a structure is found to be rock, partially rock or other incompressible material, the incompressible material shall be removed to a depth of six (6) inches below the footing grade and backfilled with a compressible material approved by the City and compacted in accordance with Section 400.5.

(4) Trench. Unless otherwise shown on the Plans, all sewer pipe structures shall be constructed in an open cut with vertical sides to a point one (1) foot above the pipe. When site conditions or the Plans do not prohibit the sloping of the cut, the excavation one (1) foot above the pipe may be

stepped and/or the sides laid back to a stable slope. Required vertical sides shall be sheeted and braced when necessary to maintain the required vertical excavation throughout the construction period.

For all pipe sewers to be constructed in fill above natural ground, the embankment shall first be constructed to an elevation not less than one (1) foot above the top of the pipe, after which excavation for the pipe shall be made as noted above.

Unstable or incompressible material shall be removed in accordance with Section 400.2(3). For unstable trench conditions requiring outside forms, seals, sheeting and bracing, or where ground water is encountered, any additional excavation and backfill required shall be done at the Contractor's expense for trenches up to five (5) feet in depth.

400.3 COFFERDAMS.

The term cofferdam designates any temporary or removable structure constructed to hold the surrounding earth, water, or both out of the excavation, whether the structure is formed of soil, timber, steel, concrete, or a combination of these. The "cofferdam" shall also include the use of pumping wells or well points used for the same purpose. The cost of cofferdams shall be included in the price bid for excavation except where temporary special shoring is shown on the Plans to provide excavation protection.

For sheet pile or other types of cofferdams, which require internal bracing, the Contractor shall submit details and design calculations bearing the seal of a Registered Professional City for review. The maximum stresses shall not exceed 125 percent of the working allowable stresses used by the City Engineering Department for the design of structures. The interior dimensions of cofferdams shall provide sufficient clearance for the construction, inspection (inside and outside), and removal of any required forms and to permit pumping outside the forms. In general, sheet pile cofferdams shall extend well below the bottom of the footings and any concrete seal and shall be well braced and as watertight as practicable.

When the City judges it to be impractical to de-water a cofferdam and a concrete seal is to be placed around piling driven therein, the excavation shall be deep enough to allow for swell of the material during pile driving operations. After driving the piling, all swelling material shall be removed to the bottom of the seal grade. Where it is possible to de-water the cofferdam without placing a seal, the foundation material shall be removed to exact footing grades after piling are driven. Backfilling a foundation to compensate for excavation, which has been extended below grade, will not be permitted. Such areas below grade shall be filled with concrete at the time the seals or footings are placed. The additional concrete quantities necessary to compensate for excavation below grade shall be at the Contractor's expense.

Unless otherwise provided, the Contractor shall remove cofferdams after the completion of the substructure without disturbing or damaging the structure.

400.4 SHAPING AND BEDDING.

For precast pipe and box sections, the excavation shall be undercut a minimum depth sufficient to accommodate the class of bedding indicated on the Plans and conforming to the bedding requirement of this Item. Where cement stabilized backfill is indicated on the Plans, the excavation shall be undercut a minimum of four (4) inches and backfilled with stabilized material to support the pipe at the required grade.

Three classes of bedding for trench or embankment conditions are shown in Figures 1, 2, and 3. Bedding shall be in accordance with Class C bedding unless otherwise shown on the Plans. The City may require the use of a template to secure reasonably accurate shaping of the foundation material.

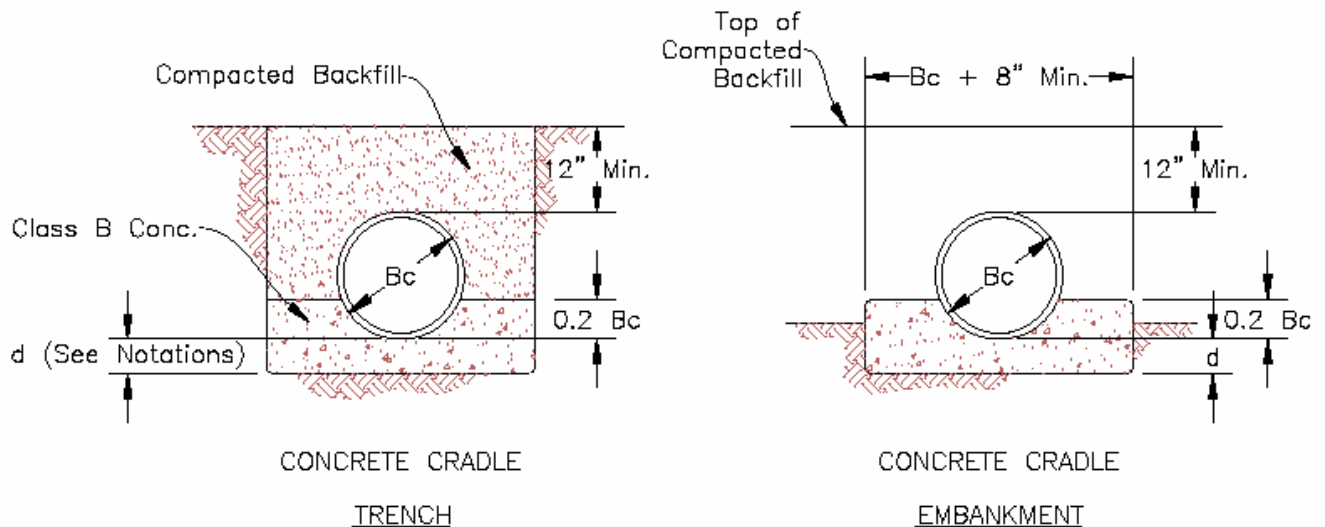
DEPTH OF BEDDING MATERIAL BELOW PIPE

D	d (Min.)
27" & smaller	3
30" to 60"	4
66" & larger	6

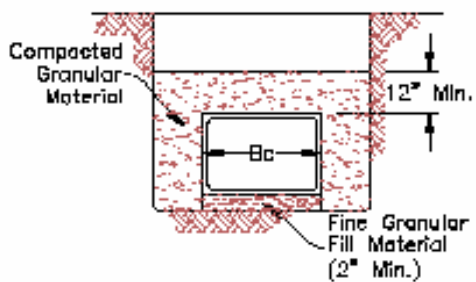
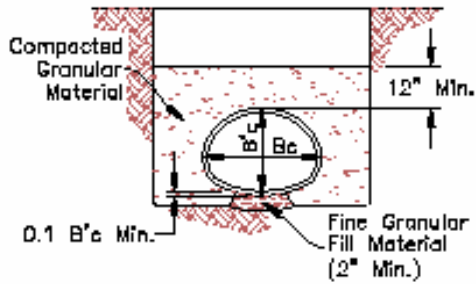
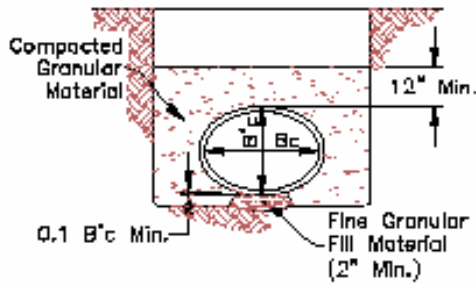
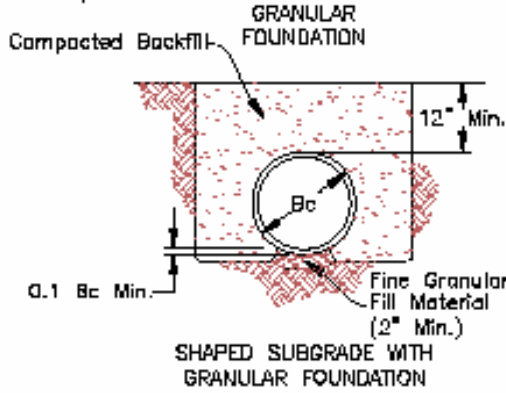
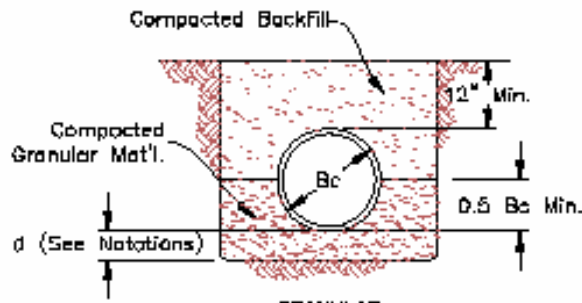
LEGEND

- Bc = Outside Diameter or Horizontal Dimension
- B'c = Vertical Dimension
- H = Backfill Cover Above Top of Pipe in Inches
- D = Inside Diameter of Pipe
- d = Depth of Bedding Material Below Pipe in Inches

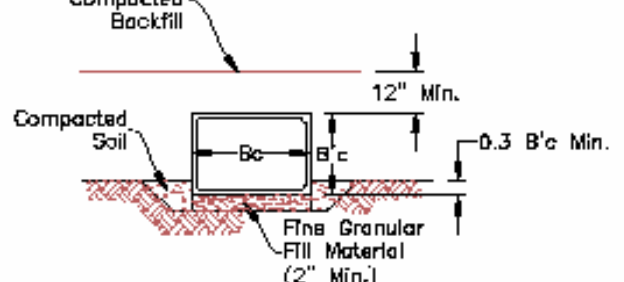
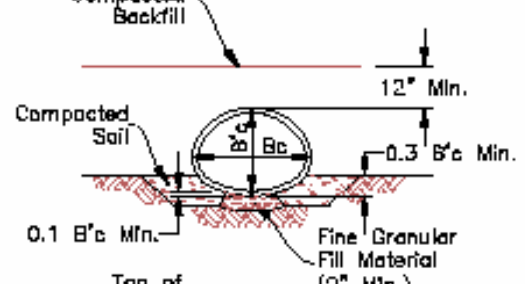
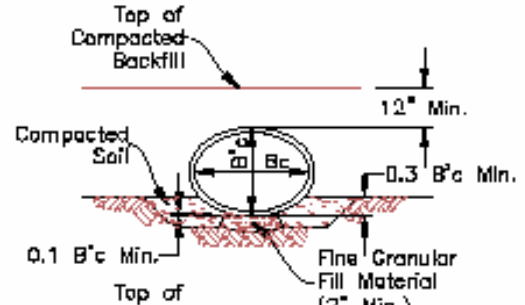
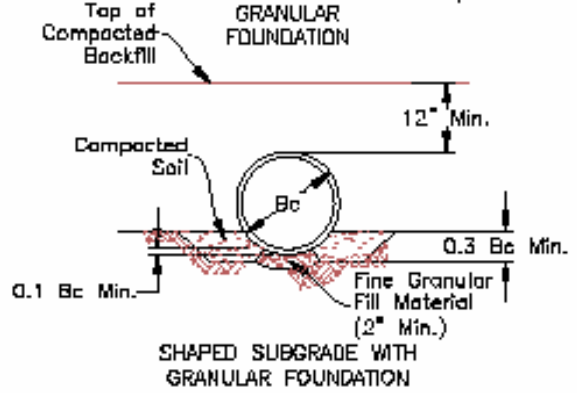
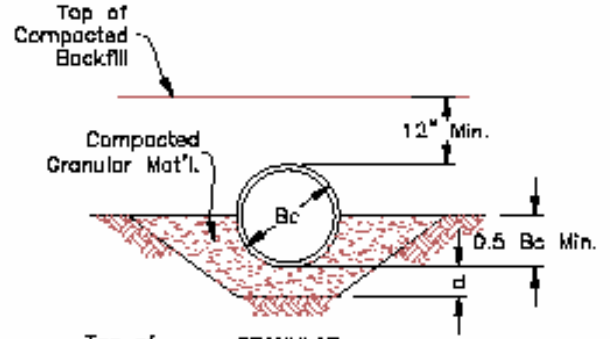
NOTATIONS FOR FIGURES 1, 2 & 3



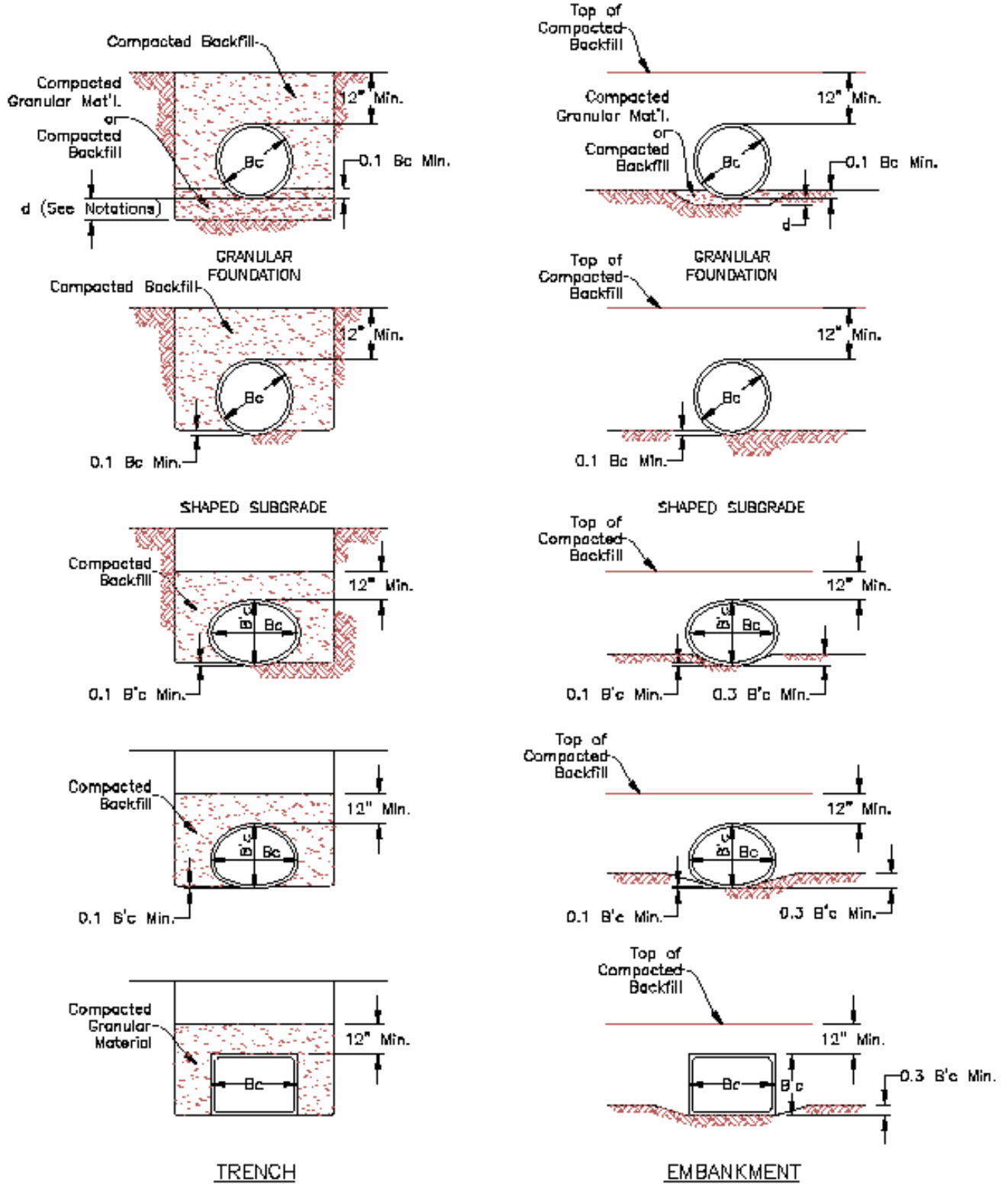
CLASS A
(Figure 1)



TRENCH



EMBANKMENT



CLASS C
(Figure 3)

400.5 BACKFILL.

(1) General. As soon as practical, all portions of the excavation not occupied by the permanent structure shall be backfilled. Backfill material may be obtained from excavation or from other sources. Backfill material shall be free from stones of such size as to interfere with compaction; free from large lumps which will not break down readily under compaction; and free from frozen lumps, wood, or other extraneous material.

Backfill which will not support any portion of the completed roadbed or embankment shall be placed in layers not more than ten (10) inches in depth (loose measurement). Backfill which will support any portion of the roadbed or embankment shall be placed in uniform layers not to exceed eight inches in depth (loose measurement). Each layer of backfill shall be compacted to a density comparable with the adjacent undisturbed soil or as shown on the Plans.

Each layer of backfill material, if dry, shall be wetted uniformly to the moisture content required to obtain a density comparable with the adjacent undisturbed soil or as shown on the Plans and shall be compacted to that density by means of mechanical tamps or rammers. The use of rolling equipment of the type generally used in compacting embankments will be permitted on portions which are accessible to such equipment.

When tamping equipment is furnished which, when proven to the satisfaction of the City, will adequately compact the backfill material to the density required, the eight (8) inch and ten (10) inch lifts (loose measurement) specified above may be increased to lifts not to exceed 12 inches.

Cohesionless materials, such as sand, may be used for general backfilling purposes. Compaction of cohesionless materials shall be done with vibratory equipment, water ponding or a combination thereof.

(2) Bridge Foundations, Retaining Walls, and Culverts. No backfill shall be placed against any structure until the concrete has reached the minimum flexural strength required in Item 421, "Portland Cement Concrete".

The material used for backfilling shall be free of any appreciable amount of gravel or stone particles more than four inches in greatest dimension and shall be of a gradation that permits thorough compaction. The use of rock or gravel mixed with soil will be permitted, provided the percentage of fines is sufficient to fill all voids and insure a uniform and thoroughly compacted mass of proper density.

When the excavation has been made through a hard material resistant to erosion, the City may require the backfill around piers and in front of abutments and wings to be of stone or lean concrete. Unless otherwise provided, such backfill will be measured and paid for as extra work in accordance with Part I, General Provisions – Division I, General Requirements and Covenants.

Embankment which is too close to a structure to permit compaction by the use of the blading and rolling equipment used on adjoining sections of embankment, shall be placed and compacted in

accordance with Section 400.5(1). Mechanical tamps or rammers shall be required when the structure being backfilled could sustain damage from other compacting operations.

Care shall be taken to prevent any wedging action of backfill against the structure, and the slopes bounding the excavation shall be stepped or serrated to prevent such action. Backfill placed around piers shall be deposited uniformly.

(3) Pipe. After the bedding and pipes have been installed as required, the selected backfill materials shall be brought to proper moisture condition, placed along both sides of the pipe equally, in uniform layers not exceeding eight (8) inches in depth (loose measurement), and each lift thoroughly compacted mechanically. Special care shall be taken to secure thorough compaction of the materials placed under the haunches of the pipe and to prevent damage or displacement of the pipe. Filling and/or backfilling shall be continued in this manner to the elevation of the top of the pipe. Backfill above the top of the pipe shall be placed and compacted in accordance with Subarticle 400.5(1). During construction, protection of the pipe shall be in accordance with the pertinent pipe item. Pipe damaged by the Contractor during construction shall be replaced at the Contractor's expense or repaired to the satisfaction of the City.

The City may reject any material containing more than 20 percent by weight of material retained on a three (3) inch sieve, or material excavated in such a manner as to produce large lumps not easily broken down or which cannot be spread in loose layers. In general, material excavated by means of a trenching machine will meet the requirements above, provided large stones are not present.

Where sewers extend beyond the toe of slope of the embankment and the depth of cover provided by backfill to the original ground level is less than the minimum required by the specifications for the type of pipe involved, additional material shall be placed and compacted, as herein specified for backfill outside the limits of the roadbed, until this minimum cover has been provided.

400.6 CEMENT STABILIZED BACKFILL.

When shown on the Plans, the excavation shall be backfilled to the elevations shown with cement stabilized backfill. Unless otherwise shown on the Plans, cement stabilized backfill shall contain aggregate, water and a minimum of seven (7) percent Portland Cement based on the dry weight of the aggregate, in accordance with Test Method Tex-120-E / ASTM D 1633. Aggregate shall be as shown on the Plans or as approved by the City.

Cement stabilized backfill below the top of sewers, manholes, inlets, or other structures shall be placed equally along all sides of the structure so as to prevent strain on or displacement of the structure. Cement stabilized backfill shall be placed in a manner that will completely fill all voids in the trench. Should compaction be required to fill all voids, hand operated tampers may be used.

400.7 MEASUREMENT.

Excavation and backfill will be measured by the cubic yard. Cutting and restoring of pavement will be measured by the square yard.

This is a Plans quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans, except as may be modified by General Conditions of Contract Documents. If no adjustment of quantities is required, additional measurements or calculations will not be required.

Unless otherwise shown on the Plans, structural excavation for pipe headwalls, inlets, manholes, culvert widening (extensions) 15 feet or less in length, bridge abutments, retaining walls and side road and private entrance pipe culverts will not be measured but shall be considered subsidiary to the various bid items.

For culvert widening (extensive) greater than 15 feet, quantities for structural excavation will be shown on the Plans.

Structural excavation will be measured by the cubic yard computed by the method of average end areas using the following limits to establish templates for measurement:

(1) For all excavation requiring measurement, except that required for the barrels of pipe culverts; for structural plate structures no material outside of vertical planes one (1) foot beyond the edges of the footings and parallel thereto will be included, unless otherwise shown on the Plans. When the Plans provide the Contractor the option of cast-in-place or precast boxes, measurement will be based on the cast-in-place option.

(2) For pipes 42 inches or less in nominal or equivalent diameter, no material outside of vertical planes one foot beyond the horizontal projection of the outside surfaces of the pipe and parallel thereto will be included. For pipes more than 42 inches in nominal or equivalent diameter, no material outside of vertical planes located two (2) feet beyond the horizontal projection of the outside surfaces of the pipe and parallel thereto will be included. Excavation for pipes shall be measured between the extreme ends of the completed structure, including any end appurtenances, as shown on the Plans and from centerline to centerline of inlets, manholes, etc., therein. When excavation for appurtenances is measured for payment, the limits of excavation for the pipes shall not overlap those of the appurtenances.

(3) For structural plate structures no material outside of vertical planes three (3) feet beyond the horizontal projection of the outside surfaces of the structure(s) and parallel thereto will be included. When the quality of the existing soil or embankment is less than that of the proposed backfill material, the excavation shall be extended for measurement to vertical planes located at one-half of the span beyond the horizontal projection of the outside surfaces of the structure(s) and parallel thereto.

(4) If a cofferdam is used, the limitations of Section 400.7(1) shall apply just as if no cofferdam were used. Excavation quantities for foundations shown on the Plans and in the proposal where

cofferdams are required shall be considered as final quantities and no further measurement will be made.

(5) Where excavation, in addition to that allowed for the footings, is required for other portions of the structure, such as for the cap, cross strut, or tie beam of a pier or bent or for the superstructure, measurement for such additional excavation will be limited laterally by vertical planes one (1) foot beyond the face of the member and parallel thereto and vertically to a depth of one (1) foot below the bottom of such member.

(6) No measurement will be made of any excavation necessary for placing forms or falsework except as allowed by the above conditions.

(7) At all structure sites except at culverts and trench excavations, the measurement of structural excavation will include only material below or outside the limits of the completed road or channel excavation.

Trench excavation in fill above natural ground, as specified in Section 400.2(4), will be measured for payment. Quantities will include that area as specified in Section 400.7(2) plus one (1) foot above the top of the pipe, regardless of the height of fill previously made.

(8) Excavation required for shaping the slopes of header banks which were built by prior contract and upon which riprap is to be placed will be measured as "Structural Excavation, (Riprap)".

(9) For all culverts, except for side road and private entrance culverts, all excavation within the limits of the structure and below or outside the limits of the completed roadway excavation, will be measured as culvert excavation. Where the overall normal width of the culvert is 12 feet or less, measurement will be as "Structural Excavation, culvert, Small". Where the overall normal width of the culvert exceeds 12 feet, measurement will be as "Structural Excavation, Culvert, Large".

(10) Where excavation diagrams are shown on the Plans, they shall take precedence over these provisions.

(11) Measurement will not include materials removed below footing grades to compensate for anticipated swelling due to pile driving, nor will it include material required to be removed due to swelling beyond the specified limits during pile driving operations.

(12) Measurement will not include additional volume caused by slips, slides, cave-ins, sitting, or fill material resulting from the action of the elements or the Contractor's operation.

(13) Where rock or other incompressible or unstable material is undercut to provide a suitable foundation for pipe or box sections, such material below grade, which is directed by the City to be removed, will be measured for payment.

(14) No allowance will be made for any variance from plan quantity incurred by an alternate bid.

(15) Additional measurement will be made of the volume of excavation involved in the lowering or raising of the elevation of a footing, foundation, or structure unit, when such grade change is authorized by the City.

(16) Cement stabilized backfill will be measured in accordance with the backfill diagram shown on the Plans. The quantity of "Cement Stabilized Backfill" shown on the Plans shall be considered as final quantities and no further measurement will be required. Changes in alignment or grade as authorized by the City will be measured for payment.

(17) The work to be done in the cutting and restoring of pavement will be measured in accordance with the dimensions shown on the Plans. The excavation below the pavement and/or base shall be measured as structural excavation of the pertinent type.

400.8 PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Structural Excavation", "Structural Excavation (Bridge)", "Structural Excavation (Culvert, Small)", "Structural Excavation (Culvert, Large)", "Structural Excavation (Trench)", "Structural Excavation (Riprap)", "Cement Stabilized Backfill" and "Cutting and Restoring Pavement".

Payment for removal and replacement of unstable or incompressible material below the footing grades of culverts as provided for in Section 400.2.(3) will be made as follows:

When the Plans specify or when the City directs the use of special materials such as flexible base, cement stabilized base, cement stabilized backfill or other special material, payment for excavation below the footing grades shall be made at the unit price bid for "Structural Excavation" of the pertinent type. Payment for furnishing, hauling, placing and compacting the flexible base, cement stabilized base, cement stabilized backfill or other special materials will be made at the unit price bid for these items in the contract or in accordance with General Requirements and Covenants, in cases where the required material is not a bid item.

Where special materials are not required or specified, payment for the removal and replacement of unstable and/or incompressible material will be made at a price equal to 200 percent of the unit price bid per cubic yard for "Structural Excavation" of the pertinent type. This price shall be full compensation for removing the unstable or incompressible material, furnishing, hauling, placing and compacting suitable replacement material and for all labor, equipment, tools, and incidentals necessary to complete the work.

If no direct method of payment is provided in the contract for culvert excavation and no special materials are required or specified, the removal and replacement of unstable or incompressible material, when such work is authorized by the City, will be measured and paid for at fifteen dollars (\$15.00) per cubic yard.

Should the City deem it necessary to lower a bridge foundation to an elevation below the grade shown on the Plans, such over excavation below plan will be paid for as "Structural Excavation" at an adjusted unit price as defined herein. Payment will be made at a unit price equal to 115

percent of the contract unit price bid for all over excavation where the revised footing grade does not vary from plan grade by more than five feet.

Payment will be made at a unit price of 125 percent of the contract unit price bid for all over excavation where the revised grade varies from plan grade by more than five (5) feet but not in excess of ten (10) feet. In cases where the revised footing grade varies from plan grade by more than ten (10) feet, a supplemental agreement shall be prepared to establish a unit price with which to make payment for the over excavation.

No direct payment will be made for backfilling ground structures. Payment for the backfilling and compacting of areas which were removed as structural excavation shall be included in the unit price bid for "Structural Excavation".

Unless otherwise shown on the Plans, structural excavation, which has been completed to the satisfaction of the City, but not backfilled, a partial payment of 50 percent of the price bid, will be made. The remaining amount will be paid upon the satisfactory completion of the backfilling.

This price shall be full compensation for all excavation, bedding, and backfill including placing, sprinkling and compaction of material; all soundings; cleaning and filling seams; constructing all cofferdams; all de-watering; and for furnishing all materials, hauling, labor, equipment, tools, sheeting and/or bracing of excavations up to and including five feet in depth, pumps, drills, explosives, disposition of surplus material, cutting pavement and base to neat lines; and for incidentals necessary to complete the work..

CITY OF SAN ANGELO
ITEM 420
CONCRETE STRUCTURES

420.1. DESCRIPTION.

This Item shall govern for the construction of all types of structures involving the use of cast-in-place concrete. All structures shall be constructed in accordance with the details shown on the Plans and this Item.

420.2. MATERIALS.

(1) **Concrete.** All concrete shall conform to the provisions of Item 421, “Portland Cement Concrete”.

The class of concrete for each type of structure or unit shall be as shown on the Plans, or by pertinent governing specifications.

(2) **Reinforcing Steel.** All reinforcing steel shall conform to the provisions of Item 440, “Reinforcing Steel”.

(3) **Expansion Joint Material.** The following materials shall conform to the requirements of Item 433, “Joint Sealants and Fillers”.

(a) Preformed Fiber Material. Preformed fiber expansion joint material shall conform to the dimensions shown on the Plans. Unless otherwise specified, “Preformed Bituminous Fiber Material” shall be used.

(b) Joint Sealing Material. Unless shown otherwise, the sealer shall be a “Low Modulus Silicone Sealant”.

(c) Asphalt Board. Asphalt board shall conform to the dimensions shown on the Plans.

(d) Rebonded Neoprene Filler. Rebonded neoprene filler shall conform to the dimensions shown on the Plans.

(4) **Waterstop.**

(a) Rubber waterstop or polyvinyl chloride (PVC) waterstop shall be in conformance with TxDOT Item 435, “Elastomeric Materials”.

(b) Other types shall be as shown on the Plans.

(5) **Curing Materials.**

(a) Membrane curing shall conform to TxDOT Item 526, “Membrane Curing”.

(b) Cotton mats shall consist of a filling material of cotton “bat” or “bats” (min. twelve (12) oz. per sq. yd.); covered with unsized cloth (min. six (6) oz. per sq. yd.); tufted or stitched to maintain stability; shall be free from tears; and shall be in good general condition.

(c) Polyethylene sheeting shall be four (4) mil. minimum thickness and free from visible defects. It shall be clear or opaque white except when the temperature during the curing period does not exceed 60°F or when applicable to control temperature during mass pours.

(d) Burlap-polyethylene mats shall be made from burlap impregnated on one (1) side with a film of opaque white pigmented polyethylene and free from visible defects.

(e) Laminated mats shall have not less than one layer of an impervious material such as polyethylene, vinyl plastic or other acceptable material (either as a solid sheet or impregnated into another fabric) and shall be free of visible defects.

(6) **Admixtures.** Concrete admixtures shall comply with the requirements of TxDOT Item 437, “Concrete Admixtures”.

(7) **Epoxy.** Unless otherwise specified, epoxy materials shall conform to TxDOT Item 575, “Epoxy”.

420.3. GENERAL REQUIREMENTS.

Before starting work, the Contractor shall inform the City fully of the construction methods he proposes to use, the adequacy of which shall be subject to the approval of the City.

Concurrence on the part of the City of any proposed construction methods, approval of equipment, or of form and falsework Plans does not relieve the Contractor of the responsibility for the safety or correctness of the methods, the adequacy of his equipment or from carrying out the work in full accordance with the contract.

Unless otherwise shown on the Plans, the time sequence in which construction operations may be carried on and in which completed structures may be opened to traffic shall be governed by the following:

(1) Superstructure members, forms, falsework, or erection equipment shall not be placed on the substructure before the concrete therein has attained a flexural strength of 425 psi.

(2) Storage of materials on completed portions of a structure will not be permitted until all curing requirements for those particular portions have been met.

(3) A minimum flexural strength of 340 psi will be required for the following:

(a) Forms erected on concrete footings supported by piling or drilled shafts.

(b) Forms on individual drilled shafts.

Such work may begin on spread footings and culvert footings, after the concrete therein has aged at least two curing days. Concrete may be placed as soon as the forms and reinforcing steel are approved.

(4) The support of tie beam and/or cap forms by falsework placed on previously placed tie beams will be permissible provided such beams have attained 425 psi flexural strength, curing requirements are completed, and the member is properly supported to eliminate stresses not provided for in the design.

(5) Bridges and direct traffic culverts shall not be opened to construction traffic or to the traveling public until authorized by the City in accordance with the following:

After the last slab concrete has been in place at least 14 days, authorization may be given for construction traffic on structures not to exceed three quarter (3/4) ton vehicles.

After the last slab concrete has been in place at least 21 days, authorization may be given for other construction traffic or for the traveling public when necessary. Vehicles exceeding the legal load limit will be allowed in accordance with TxDOT Item 6, "Control of Materials".

(6) Box culverts in fills may be opened to backfilling and compaction equipment when the concrete in the top slab has attained 425 psi flexural strength, and may be opened to other traffic as soon as sufficient backfill and/or embankment has been placed over the top to protect the culverts against damage from heavy construction equipment. The Contractor shall repair, at his expense, any damage inflicted on the culvert by construction traffic.

420.4. DRAINS.

Weep holes and roadway drains shall be installed and constructed as shown on the Plans.

420.5. EXPANSION JOINTS.

Joints and devices to provide for expansion and contraction shall be constructed in accordance with plan details and the requirements of this Item.

The bearing area under the expansion ends of concrete slabs and slab and girder spans shall be given a steel trowel finish, and finished to the exact grades required.

Bridging of concrete or mortar around expansion joint material in bearings and expansion joints shall be prevented.

All open joints and joints to be filled with expansion joint material shall be constructed using forms adaptable to loosening or early removal. To avoid expansion or contraction damage to the adjacent concrete, these forms shall be loosened as soon as possible after final concrete set to permit free movement of the span without requiring full form removal.

When a "Type A" joint is shown on the Plans, preformed fiber joint material shall be used in the vertical joints of the roadway slab, curb, median, or sidewalk and the top one (1) inch thereof shall be filled with the joint sealing material shown herein or shown on the Plans.

The sealer shall be installed in accordance with TxDOT Item 438, "Cleaning and/or Sealing Joints and Cracks (Portland Cement Concrete)", and the manufacturer's recommendations.

Where preformed fiber joint material is used, it shall be anchored to the concrete on one side of the joint by light wire or nails.

Finished joints shall conform to the plan details with the concrete sections completely separated by the specified opening or joint material.

Soon after form removal and again where necessary after surface finishing, all concrete shall be removed from within the joint opening to insure full effectiveness of the expansion joint.

420.6. CONSTRUCTION JOINTS.

The joint formed by placing plastic concrete in direct contact with concrete that has attained its initial set shall be deemed a construction joint. The term monolithic placement shall be interpreted to mean that the manner and sequence of concrete placing shall not create a construction joint.

Construction joints shall be of the type and at the locations shown on the Plans. Construction joints other than those shown on the Plans will not be permitted in bridge slabs. Additional joints in other members will not be permitted without written authorization from the City. When additional joints are authorized, they shall have details equivalent to those shown on the Plans for joints in similar locations.

Unless otherwise provided, construction joints shall be square and normal to the forms. Bulkheads shall be provided in the forms for all vertical joints.

Construction joints requiring the use of joint sealing material shall be as shown on the Plans.

A concrete placement terminating at a horizontal construction joint shall have the top surface roughened thoroughly as soon as practicable after initial set is attained.

The hardened concrete surface shall be thoroughly cleaned of all loose material, laitance, dirt or foreign matter and saturated with water. All free water shall be removed and the surface shall be in a moist condition when concrete and/or bonding grout is placed against it.

Forms shall be drawn tight against the existing concrete to avoid mortar loss and offsets at joints.

When shown on the Plans or in other specifications, the joint surface shall be coated with bonding mortar, grout or other specified material.

When shown on the Plans, Type V epoxy material shall be used for bonding fresh concrete to hardened concrete. The bonding epoxy shall be placed on a clean, dry surface and shall be tacky when the fresh concrete is placed.

420.7. SEAL FOR FOUNDATIONS.

Concrete for foundation seals, unless otherwise specified, shall be in accordance with Item 400, "Excavation and Backfill for Structures".

420.8. FALSEWORK.

Falsework shall be designed and constructed to safely carry the maximum anticipated loads, including wind loads, and to provide the necessary rigidity. Details of falsework construction shall be subject to review and approval by the City in accordance with the provisions of Section 420.3.

For evaluating the adequacy of job fabricated falsework, a weight of 150 pounds per cubic foot shall be assumed for concrete, and a live load allowance of 50 pounds per square foot of horizontal surface of the form work shall be included. The maximum stresses shall not exceed 125 percent of the allowable stresses used by the Department for the design of structures.

Commercially produced structural units used in falsework shall not exceed the manufacturer's maximum allowable working load for moment, and shear or end reaction. The maximum allowable working load shall include an allowance of 35 pounds per square foot of horizontal form surface and sufficient details and data shall be submitted to the City for approval.

All timber used in falsework shall be sound, in good condition, and free from defects which would impair its strength.

When wedges are used to adjust falsework to desired elevations, the wedges shall be used in pairs to insure even bearing. The use of wedges to compensate for incorrectly cut bearing surfaces will not be permitted. Wedges shall be hardwood or metal.

Sills or grillages shall be large enough to support the superimposed load without settlement, and unless founded on solid rock, shale or other hard materials, precautions shall be taken to prevent yielding of the supporting material.

Falsework, which cannot be founded on a satisfactory spread footing, shall be placed on piling or drilled shafts having a bearing capacity sufficient to support the superimposed load without settlement. Falsework piling shall be driven to the required resistance determined by the applicable formula given in TxDOT Item 404, "Driving Piling". Drilled shafts for falsework shall be designed to carry the superimposed load using both skin friction and point bearing.

Welding, when used, shall conform to the requirements of TxDOT Item 448, "Structural Field Welding". Each falsework bent shall be securely braced to provide the stiffness required with the bracing securely fastened to each pile or column it crosses.

The falsework shall be removed when no longer required. Falsework piling shall be pulled or cut off not less than six (6) inches below finished ground level. Falsework, piling or drilled shafts in a stream, lake, or bay shall be completely removed to a point specified by the City to prevent any obstruction to the waterway.

420.9. FORMS.

All forms shall be constructed in accordance with the following:

(1) General. Except where otherwise specified, forms may be of either timber or metal.

Forms for round columns exposed to view shall be of steel, except that other materials will be allowed with written permission of the City.

Studs, joists, wales or other devices used for form supports shall be of sufficient section and rigidity to withstand undue bulging or settling of the forms. Any device or method used for form support shall be subject to the approval of the City.

Forms shall be designed for the pressure exerted by a liquid weighing 150 pounds per cubic foot. The rate of placing the concrete shall be taken into consideration in determining the depth of the equivalent liquid. Job fabricated forms shall be designed for an additional live load of 50 pounds per square foot of horizontal surface. The maximum unit stresses shall not exceed 125 percent of the allowable stresses used by the City Engineering Department for the design of structures.

Commercially produced structural units used in formwork shall not exceed the manufacturer's maximum allowable working load for moment, shear or end reaction. The maximum working load shall include a live load of 35 pounds per square foot of horizontal form surface and sufficient details and data shall be submitted to the City for review.

Forms shall be practically mortar-tight, rigidly braced and strong enough to prevent bulging between supports and shall be maintained to the proper line and grade during concrete placement. Forms shall be maintained in a manner to prevent warping and shrinkage.

Offsets at form joints shall not exceed 1/16 inch. Form supports for slabs shall not be welded to the top flange of I-beams or girders except in accordance with the provisions of Section 420.3.

Deflections due to cast-in-place slab concrete and railing shown in the dead load deflection diagram shall be taken into account in the setting of slab forms.

All forms and footing areas shall be cleaned of any extraneous matter before placing concrete.

Permission to place concrete will not be given until all preparatory work is complete to the satisfaction of the City.

If, at any stage of placement, the forms show signs of bulging or sagging, the portion of the causing such condition shall be removed immediately, if necessary, and the forms shall be reset and securely braced against further movement.

(2) Timber Forms. Lumber for forms shall be properly seasoned, of good quality, and free from imperfections which would affect its strength or impair the finished surface of the concrete

Forms or form lumber to be reused shall be maintained clean and in good condition. Any lumber which is split, warped, bulged, marred, or has defects that will produce inferior work shall not be used and shall be promptly removed from the work.

Form lining will be required for all formed surfaces, except for the inside of culvert barrels, inlets, manholes and box girders; the bottom of bridge decks between beams or girders; surfaces that are subsequently covered by backfill material or are completely enclosed and any surface formed by a single finished board. Lining will not be required when plywood forms are used.

Form lining shall be of an approved type such as Masonite or plywood. Thin membrane sheeting such as polyethylene sheets shall not be used for form lining.

Commercial form liners used to imprint a pattern or texture on the surface of the concrete shall be as shown on the Plans and/or as approved by the City.

Forms may be constructed of plywood not less than one-half (1/2) inch in thickness. The grain of the face plies on plywood forms shall be placed parallel to the span between the supporting studs or joists.

Plywood used for forming surfaces which remain exposed shall be equal to that specified as B-B Plyform Class I or Class III Exterior of the U.S. Department of Commerce, National Institute of Standards and Technology, U.S. Product Standard, latest edition.

Studs and joists shall be spaced so that the facing form material remains in true alignment under the imposed loads.

Wales shall be spaced close enough to hold forms securely to the designated lines and scabbed at least four feet on each side of joints to provide continuity. A row of wales shall be placed near the bottom of each placement.

Facing material shall be placed with parallel and square joints and securely fastened to supporting studs.

Forms for surfaces receiving only an ordinary finish and exposed to view shall be placed with the form panels symmetrical, i.e., long dimensions set in the same direction. Horizontal joints shall be continuous.

Molding for chamfer strips or other uses shall be made of materials of a grade that will not split when nailed and which can be maintained to a true line without warping. Wood molding shall be mill cut and dressed on all faces. Unless otherwise provided herein or shown on the Plans, forms shall be filleted at all sharp corners and edges with triangular chamfer strips measuring three fourth (3/4) inch on the sides.

Except at structures where railing is to be attached, culvert headwall heights shall be adjusted as necessary to provide a maximum projection of three (3) inches above the roadway slope unless otherwise directed by the City. At the entrance of all culverts, a three-inch chamfer shall be provided along the bottom edge of the top slab. Reinforcing steel shall be adjusted as necessary to

provide a minimum 1-1/4 clear cover. No changes will be made in quantities and no additional compensation will be allowed for this work.

All forms shall be constructed to permit their removal without marring or damaging the concrete. The forms may be given a slight draft to permit ease of removal.

Metal form ties of an approved type or a satisfactory substitute shall be used to hold forms in place and shall be of a type that permits ease of removal of the metal as hereinafter specified.

All metal appliances used inside of forms for alignment purposes shall be removed to a depth of at least one-half (1/2) inch from the concrete surface. The appliances shall be made so the metal may be removed without undue chipping or spalling of the concrete, and when removed, shall leave a smooth opening in the concrete surface. Burning off of rods, bolts or ties will not be permitted.

Any wire ties used shall be cut back at least one-half (1/2) inch from the face of the concrete.

Devices holding metal ties in place shall be capable of developing the strength of the tie and adjustable to allow for proper alignment.

Metal and wooden spreaders, which are separate from the forms, shall be removed entirely as the concrete is being placed.

Adequate clean-out openings shall be provided for narrow walls and other locations where access to the bottom of the forms is not readily attainable.

The facing of all forms shall be treated with bond breaking coating of such composition that would not discolor or otherwise injuriously affect the concrete surface. Care shall be exercised to prevent coating of the reinforcing steel.

(3) Metal Forms. The foregoing requirements for timber forms regarding design, mortar-tightness, filleted corners, beveled projections, bracing, alignment, removal, reuse and wetting shall also apply to metal forms, except that these will not require lining, unless specifically noted on the Plans.

The thickness of form metal shall be as required to maintain the true shape without warping or bulging. All bolt and rivet heads on the facing sides shall be countersunk. Clamps, pins or other connecting devices shall be designed to hold the forms rigidly together and to allow removal without injury to the concrete. Metal forms which do not present a smooth surface or which line up improperly shall not be used. Metal shall be kept free from rust, grease or other foreign materials.

(4) Form Supports for Overhang Slabs. Form supports which transmit a horizontal force to a steel girder or beam, or to a prestressed concrete beam will be permitted, providing a satisfactory structural analysis has been made of the effect on the girder or beam and approval is granted by the City.

When overhang brackets are used on prestressed concrete beam spans with slab overhangs not exceeding three (3) feet six (6) inches, bracing requirements shall conform to the details shown on the Plans.

For spans in which the overhang exceeds three feet six inches, additional support will be required for the outside beams regardless of the type beam used. Details of the proposed support system shall be submitted by the Contractor for approval.

Holes in steel members for support of overhang brackets may be punched or drilled full size or may be torch cut to one-fourth (1/4) inch under size and reamed full size. In no case shall the holes be burned full size. The hole shall be left open unless otherwise shown on the Plans. The holes shall never be filled by welding.

420.10. PLACING REINFORCEMENT.

Reinforcement shall be placed as provided in Item 440, "Reinforcing Steel". Reinforcing steel supports shall not be welded to I-beams or girders or to reinforcing steel except where shown on the Plans to be permissible.

Post tensioning ducts shall be placed in accordance with TxDOT Item 426, "Prestressing". The Contractor shall maintain all ducts free of obstructions until all post tensioning operations are complete.

420.11. PLACING CONCRETE-GENERAL.

The Contractor shall give the City sufficient advance notice before placing concrete in any unit of the structure to permit the inspection of forms, reinforcing steel placement and other preparations.

The sequence of placing concrete shall be as shown on the Plans or as required herein.

Concrete placement will not be permitted when impending weather conditions would impair the quality of the finished work. If conditions of wind, humidity, and temperature are such that concrete cannot be placed without cracking, concrete placement shall be done in the early morning or at night. When concrete mixing, placing, and finishing is done in other than daylight hours, provisions shall be made to adequately light the entire placement site. The City will approve the adequacy of such lighting before operations are begun.

Where work has been started and changes in weather conditions require protective measures, the Contractor shall furnish adequate shelter to protect the concrete against damage from rainfall, or from freezing temperatures as outlined in Section 420.12. If necessary to continue operations during rainfall, the Contractor shall also provide protective coverings for the material stockpiles. Aggregate stockpiles need to be covered only to the extent necessary to control the moisture conditions in the aggregates.

After concrete has achieved initial set, at least one (1) curing day shall elapse before placing strain on projecting reinforcement in order to prevent damage to the concrete.

(1) Placing Temperature. The temperature of all concrete at the time of placement shall be not less than 50° F.

The temperature of cast-in-place concrete in bridge slabs and top slabs of direct traffic structures shall not exceed 85° F when placed. Concrete diaphragms, parapets, concrete portions of railing, curbs, and sidewalks, unless monolithically placed with the slab, will not be subject to the above maximum. Other portions of structures, when shown on the Plans, shall require the temperature control specified.

For mass concrete placements, as defined in Section 420.11 (10), the concrete temperature at the time of placement shall not exceed 75° F.

(2) Transporting Time. The maximum time interval between the addition of cement to the batch and the placing of concrete in the forms shall conform to the requirements in Table 1.

TABLE 1
TEMPERATURE TIME REQUIREMENTS

Concrete Temperature (at point of placement)	Max. Time (No Retarding Agent) Minutes	Max. Time (1) (With Retarding Agent) Minutes
Non-Agitated Concrete		
Above 80°F	15	30
80°F and Below	30	45
Agitated Concrete		
Above 90°F	45	75
Above 75°F thru 90°F	60	90
75°F and Below	90	120

(1) Normal dosage of retarder

(3) Transporting Equipment. The method and equipment used to transport concrete to the forms shall be capable of maintaining the rate of placement shown on the Plans or required by the City. Concrete may be transported by buckets, chutes, buggies, belt conveyors, pumps, or other methods.

When belt conveyors or pumps are used, sampling for testing should be done at the discharge end. When in the opinion of the City, it is deemed impractical to sample at the discharge end, sampling may be done at the mixer provided that correlation testing is performed and documented to ensure specification requirements are met at the discharge end.

Concrete transported by conveyors shall be protected from sun and wind, if necessary, to prevent loss of slump and workability. Pipes through which concrete is pumped shall be shaded and/or wrapped with wet burlap, if necessary, to prevent loss of slump and workability. Concrete shall not be transported through aluminum pipes, tubes, or other aluminum equipment.

Pump lines shall conform to the following:

For Grade 2 coarse aggregate and smaller, the minimum size pump line shall be five (5) inches ID.

For Grade 1 coarse aggregate, the minimum size pump line shall be eight (8) inches ID.

Chutes, troughs, conveyors or pipes shall be arranged and used so that the concrete ingredients will not be separated. When necessary to prevent segregation, such equipment shall terminate in vertical downspouts. Open troughs and chutes shall extend, if necessary, down inside the forms or through holes left in the forms.

All transporting equipment shall be kept clean and free from hardened concrete coatings. Water used for cleaning shall be discharged clear of the concrete.

(4) Forms. Openings in forms shall be provided, if needed, for the removal of laitance or foreign matter.

All forms, prestressed concrete panels, T-beams, and concrete box beams on which concrete is to be placed shall be wetted thoroughly prior to placing concrete thereon. Any remaining puddles of excess water shall be removed. The top of such members shall be in moist surface dry condition when concrete is placed on them.

(5) Handling, Placing, and Consolidation. The method of handling, placing, and consolidation of concrete shall minimize segregation of the concrete and displacement of the reinforcement. A uniform dense compact mass shall be produced.

(a) Handling and Placing. Concrete shall not have a free fall of more than five (5) feet, except in the case of thin walls such as in culverts or as specified in other items. Any hardened concrete splatter ahead of the plastic concrete shall be removed.

Each part of the forms shall be filled by depositing concrete as near its final position as possible. Depositing large quantities at one (1) point and running or working the concrete along the forms will not be allowed.

Concrete shall be deposited in the forms in layers of suitable depth but not more than 36 inches in thickness, unless otherwise directed by the City.

Cold joints in a monolithic placement shall be avoided. The sequence of successive layers or adjacent portions of concrete shall be such that they can be vibrated into a homogeneous mass with the previously placed concrete. Not more than one (1) hour shall elapse between adjacent or successive placements of concrete except as otherwise required by an approved placing procedure when revibration of the concrete is shown on the Plans or specifications. This time requirement may be extended by one-half (1/2) hour when the concrete contains not less than a normal dosage of retarding admixture.

An approved retarding agent shall be used to control stress cracks and/or cold joints in placements where differential settlement and/or setting time may induce stress cracking.

(b) Consolidation. All concrete shall be well consolidated and the mortar flushed to the form surfaces with immersion type vibrators. Vibrators which operate by attachment to forms or reinforcement will not be permitted, except on steel forms. At least one (1) stand-by vibrator shall be provided for emergency use in addition to those required for placement.

The concrete shall be vibrated immediately after deposit. A systematic spacing of the points of vibration shall be established to insure complete consolidation and thorough working of the concrete around the reinforcement, embedded fixtures, and into the corners and angles of the forms. The vibrator may be inserted in a sloping or horizontal position in shallow slabs. The entire depth of each lift shall be vibrated, allowing the vibrator to penetrate several inches into the preceding lift. Concrete along construction joints shall be thoroughly consolidated by operating the vibrator along and close to but not against the joint surface. The vibration shall continue until thorough consolidation and complete embedment of reinforcement and fixtures is produced, but not long enough to cause segregation. Vibration may be supplemented by hand spading or rodding, if necessary, to insure the flushing of mortar to the surface of all forms.

(6) Slabs. Unless otherwise shown on the Plans or other specifications, slab concrete shall be mixed in a plant located off the structure. Carting or wheeling concrete batches over completed slabs will not be permitted until the slabs have aged at least four (4) full curing days. For the remainder of the curing period, timber planking will be required for carting of the concrete. Carts shall be equipped with pneumatic tires. Curing operations shall not be interrupted for the purpose of wheeling concrete over finished slabs.

The storing of reinforcing or structural steel on completed roadway slabs generally shall be avoided and, when permitted, shall be limited to quantities and distribution that will not induce excessive stresses.

A longitudinal screed may be placed directly on previously placed concrete slabs for the purpose of checking and grading of an adjacent slab after the previously placed slab has aged not less than 24 hours. Actual screeding may be done after the previously placed slabs have aged at least 48 hours.

(7) Continuous Placements. For continuous placement of the deck on steel units, the initial set of the concrete shall be retarded sufficiently to insure that the concrete remains plastic in not less than three (3) spans immediately preceding the slab being placed. For simple spans, retardation shall be required only if necessary to complete finishing operations or as required by Section 420.13.

(8) Fogging and Interim Curing. From the time of initial strike off of the concrete until finishing is completed and required interim curing is in place, the unformed surfaces of slab concrete in bridge decks and top slabs of direct traffic culverts shall be fogged when necessary to replace water loss due to evaporation.

Fogging equipment shall be capable of applying water in a fine mist, not a spray. The fog shall be produced using equipment, which pumps water or water and air under high pressure through a suitable atomizing nozzle. The equipment shall be hand operated and sufficiently portable for use in the direction of any prevailing wind. It shall be adaptable for intermittent use as directed by the City to prevent excessive wetting of the concrete.

Interim curing will be required for slab concrete in bridge decks and top slabs of the direct traffic culverts, immediately upon completion of final finish. Type 1-D membrane curing compound (Resin Base Only) will be required. Water curing will be required in accordance with Section 420.20 and shall be commenced as soon as possible without damaging the surface finish.

(9) Installation of Dowels and Anchor Bolts. Dowels and anchor bolts may be cast-in-place or installed by grouting with grout, epoxy or epoxy mortar. Holes for grouting may be formed or drilled.

(a) General. Holes for anchor bolts shall accommodate the bolt embedment required by the Plans. Holes for dowels shall be a minimum of 12 inches deep unless otherwise shown on the Plans. When grout or epoxy mortar is used, the diameter of the hole shall be not less than twice the dowel or bolt diameter or more than the diameter plus 1 1/2 inches. When using epoxy, the hole diameter shall be one-sixteenth (1/16) inch to one-fourth (1/4) inch greater than the dowel or bolt diameter.

Holes shall be thoroughly cleaned of all loose material, oil, grease, or other bond breaking substance and blown clean with filtered compressed air. Holes shall be in a surface dry condition when epoxy type material is used. Holes shall be in a surface moist condition when Portland cement grout is used. The Contractor shall develop and demonstrate a procedure for cleaning and preparing the holes for installation of the dowels and anchor bolts that is satisfactory to the City. The void between the hole and dowel or bolt shall be completely filled with grouting material.

(b) Cast-in-Place or Grouted Systems. Portland cement grout, epoxy, epoxy mortar, or other prepackaged grouts as approved by the City may be used.

Portland cement grout shall conform to the pertinent provisions of Item 421, "Portland Cement Concrete". Epoxy

(Type V) and Epoxy Mortar (Type VIII) shall conform to TxDOT Item 575, "Epoxy". Grout, epoxy or epoxy mortar may be used as the binding agent unless otherwise indicated on the Plans.

(c) Other Anchor Systems. These systems shall be in accordance with the Plans and approved by the City.

(10) Mass Placements. Unless otherwise shown on the Plans, for monolithic mass placements having a least dimension greater than five (5) feet, the Contractor shall develop a plan to assure that during the heat dissipation period, the temperature differential between the central core of the placement and the exposed concrete surface does not exceed 35°F.

A detailed plan, along with an analysis of the associated heat generation and dissipation (heat flow analysis) shall be submitted to the City for approval. No concrete shall be placed until this plan is approved.

This plan may include a combination of the following:

1. Selection of concrete ingredients to minimize heat of hydration.

2. Using ice or cooling concrete ingredients.
3. Controlling rate of concrete placement.
4. Using insulation to control heat loss.
5. Using supplemental heat to control heat loss.
6. Use of fly ash.

The Contractor shall furnish and install two sets of strip chart temperature recording devices or approved equivalent at locations designated by the City. These devices shall be accurate to within +/- 2°F within the range of 32°F to 212°F and shall be used to simultaneously measure the temperature of the concrete at the core and the surface.

420.12. PLACING CONCRETE IN COLD WEATHER.

The Contractor is responsible for the protection of concrete placed under any and all weather conditions. Permission given by the City for placing during cold weather will not relieve the Contractor of the responsibility for producing concrete equal in quality to that placed under normal conditions. Should concrete placed under such conditions prove unsatisfactory, it shall be removed and replaced.

Concrete may be placed only when the atmospheric temperature is greater than 35°F. Concrete shall not be placed in contact with any material coated with frost or having a temperature less than 32°F.

Aggregates shall be free from ice, frost and frozen lumps. When required, in order to produce the minimum specified concrete temperature, the aggregate and/or the water shall be heated uniformly, in accordance with the following:

The water temperature shall not exceed 180°F, nor shall the aggregate temperature exceed 150°F. The heating apparatus shall heat the mass of aggregate uniformly. The temperature of the mixture of aggregates and water shall be between 50°F and 85°F before introduction of the cement.

The Contractor shall provide and install recording thermometer(s) or other suitable temperature measuring device(s) to verify that all concrete is effectively protected as follows:

(a) The temperature of all unformed surfaces of bridge decks and top slabs of direct traffic culverts shall be maintained at 50°F or above for a period of 72 hours from time of placement and above 40°F for an additional 72 hours.

(b) The temperature at the surface of all concrete in bents, piers, culvert walls, retaining walls, parapets, wingwalls, bottom of slabs, and other similar formed concrete shall be maintained at 40°F or above for a period of 72 hours from time of placement.

(c) The temperature of all concrete, including the bottom slabs (footings) of culverts placed on or in the ground, shall be maintained above 32°F for a period of 72 hours from the time of placement.

Protection shall consist of providing additional covering, insulated forms or other means, and if necessary, supplementing such covering with artificial heating. Curing as specified under Section 420.20 shall be provided during this period until all requirements for curing have been satisfied.

When impending weather conditions indicate the possibility of the need for such temperature protection, all necessary heating and covering material shall be on hand and ready for use before permission is granted to begin placement.

Sufficient extra test specimens will be made and cured with the placement to ascertain the condition of the concrete as placed to form removal and acceptance.

420.13. PLACING CONCRETE IN HOT WEATHER.

Unless otherwise directed by the City, when the temperature of the air is above 85°F, an approved retarding agent will be required in all concrete used in superstructures and top slabs of direct traffic culverts.

420.14. PLACING CONCRETE IN WATER.

Concrete shall be deposited in water only when shown on the Plans or with the written permission of the City. The forms or cofferdams shall be sufficiently tight to prevent any water current passing through the space in which the concrete is being deposited. Pumping of water will not be permitted during the concrete placing, nor until it has set for at least 36 hours.

The concrete shall be placed with a tremie, or other approved method, and shall not be permitted to fall freely through the water nor shall the concrete be disturbed after being placed. The concrete surface shall be kept approximately level during placement.

The tremie shall consist of a watertight tube of a diameter which will permit adequate placement of the concrete, but not greater than 14 inches. The tremie shall be constructed so that the bottom can be sealed and opened after the tremie is in place and fully charged with concrete. The tremie shall be supported so that it can be easily moved horizontally to cover all the work area and vertically to control the concrete flow. The lower end of the tremie shall be submerged in the concrete at all times.

The placing operations shall be continuous until the work is complete.

Unless otherwise specified, all classes of concrete placed under water, except Class E and Class SS, shall be redesigned to contain an additional sack of cement per cubic yard more than the mix design being used. Pilot beam tests may be waived by the City for this redesign.

420.15. PLACING CONCRETE IN SUPERSTRUCTURE.

Unless otherwise shown on the Plans, simple span bridge slabs shall be placed without transverse construction joints by using either a mechanical longitudinal screed or a self propelled transverse finishing machine. For small placements or for unusual conditions, the City may waive the mechanical screed requirement and permit the use of manually operated screeding equipment. The screed shall be adequately supported on a header or rail system sufficiently stable to withstand the longitudinal or lateral thrust of the equipment. Unless otherwise shown on the Plans, temporary intermediate headers will be permitted for placements exceeding 50 feet in length for the longitudinal screed, provided the rate of placement is rapid enough to prevent a cold joint, and that these headers are designed for early removal to permit satisfactory consolidation and finish of the concrete at their locations.

Unless otherwise shown on the Plans, slabs on continuous units shall be placed in one (1) continuous operation without transverse construction joints using a mechanical longitudinal screed or a self propelled transverse finishing machine. For unusual conditions, such as widening, variable cross slopes or transitions, the City may waive the mechanical screed requirement and permit the use of manually operated screeding equipment. Rails for transverse finishing machines, which are supported from the beams or girders, shall be installed so that the supports may be removed without damage to the slab. Bond between removable supports and the concrete shall be prevented in a manner acceptable to the City. Rail support parts, which remain embedded in the slab, shall not project above the upper mat of reinforcing steel. Rail or screed supports attached to I-beams or girders shall be subject to the requirements of Section 420.3.

Unless otherwise shown on the Plans, for transverse screeding, the minimum rate of concrete placement shall be 30 linear feet of bridge deck per hour. The Contractor shall furnish personnel and equipment capable of placing, finishing and curing the slab at an acceptable rate to insure compliance with the specifications.

The profile gradeline may require adjustment, due to variation in beam camber and other factors, to obtain the required cover over the slab reinforcement. Beams shall be set in a sufficient number of spans so that when adjustment is necessary, the profile gradeline can be adjusted over suitable increments and the revised gradeline will produce a smooth riding surface.

One or more passes shall be made with the screed over the bridge deck segment prior to the placement of concrete thereon to insure proper operation and maintenance of grades and clearances.

Slab concrete shall be deposited between the exterior beam and the adjacent beam prior to placing concrete in the overhang portion of the slab.

For transverse screeding, concrete shall be placed in transverse strips. Additionally, on profile grades greater than 1-1/2 percent, placement shall begin at the lowest end.

For longitudinal screeding, concrete shall be placed in longitudinal strips starting at a point in the center of the segment adjacent to one (1) side, except as provided herein, and the strip completed by placing uniformly in both directions toward the ends, except that for spans on a grade of 1-1/2 percent or more placing shall start at the lowest end.

The width of strips shall be such that the concrete therein will remain plastic until the adjacent strip is placed. Where monolithic curb construction is specified, the concrete shall be placed therein in proper sequence to be monolithic with the adjacent longitudinal strips of the slabs.

An approved system of checking shall be used to detect any vertical movement of the forms or falsework. Forms for the bottom surface of concrete slabs, girders and overhangs shall be maintained to the required vertical alignment during concrete placing.

Unless otherwise shown on the Plans, girders, slab and curbs of slab and girder spans shall be placed monolithically. Concrete girder stems shall be filled first and the slab concrete placed within the time limits specified in Section 420.11.

Construction joints, when permitted for slab placements on steel and prestressed concrete beams, shall be as shown on the Plans. Where Plans permit segmental placing without specifying a particular order of placement, any logical placing sequence which will not result in the overstressing of any of the supporting members will be permitted subject to the approval of the City.

Any falsework under steel girder or truss spans shall be released and the spans swung free on their permanent supports before placing any slab concrete thereon.

When the curb forms are filled, the top of curb and sidewalk section shall be brought to the correct camber and alignment and finished as described in Sections 420.18 and 420.23.

420.16. PLACING CONCRETE IN BOX CULVERTS.

Where the top slab and walls are placed monolithically in culverts more than four feet in clear height, an interval of not less than one (1) or more than two (2) hours shall elapse before placing the top slab to allow for settlement and shrinkage in the wall concrete.

The footing slab shall be accurately finished at the proper time to provide a smooth uniform surface. Top slabs, which carry direct traffic, shall be finished as specified in Section 420.19. Top slabs of fill-type culverts shall be given a float finish.

420.17. PLACING CONCRETE IN FOUNDATION AND SUBSTRUCTURE.

Concrete shall not be placed in footings until the depth and character of the foundation has been inspected by the City and permission has been given to proceed.

Placing of concrete footings upon seal concrete will be permitted after the cofferdams are free from water and the seal concrete cleaned. Any necessary pumping or bailing during the concreting operation shall be done from a suitable sump located outside the forms.

All temporary wales or braces inside the cofferdams shall be constructed or adjusted as the work proceeds to prevent unauthorized construction joints.

When footings can be placed in a dry excavation without the use of cofferdams, forms may be omitted, if approved by the City, and the entire excavation filled with concrete to the elevation of

the top of footing. In this case, measurement for payment will be based on the footing dimensions shown on the Plans.

Concrete in columns shall be placed monolithically between construction joints unless otherwise provided. Columns and caps and/or tie beams supported thereon may be placed in the same operation. To allow for settlement and shrinkage of the column concrete, it shall be placed to the lower level of the cap or tie beam and placement delayed for not less than one (1) hour nor more than two (2) before proceeding.

420.18. TREATMENT AND FINISHING OF HORIZONTAL SURFACES EXCEPT ROADWAY SLABS.

All unformed upper surfaces shall be struck off to grade and finished. The use of mortar topping for surfaces under this classification will not be permitted.

After the concrete has been struck off, the surface shall be floated with a suitable float. Bridge sidewalks shall be given a wood float or broom finish or may be striped with a brush, as specified by the City.

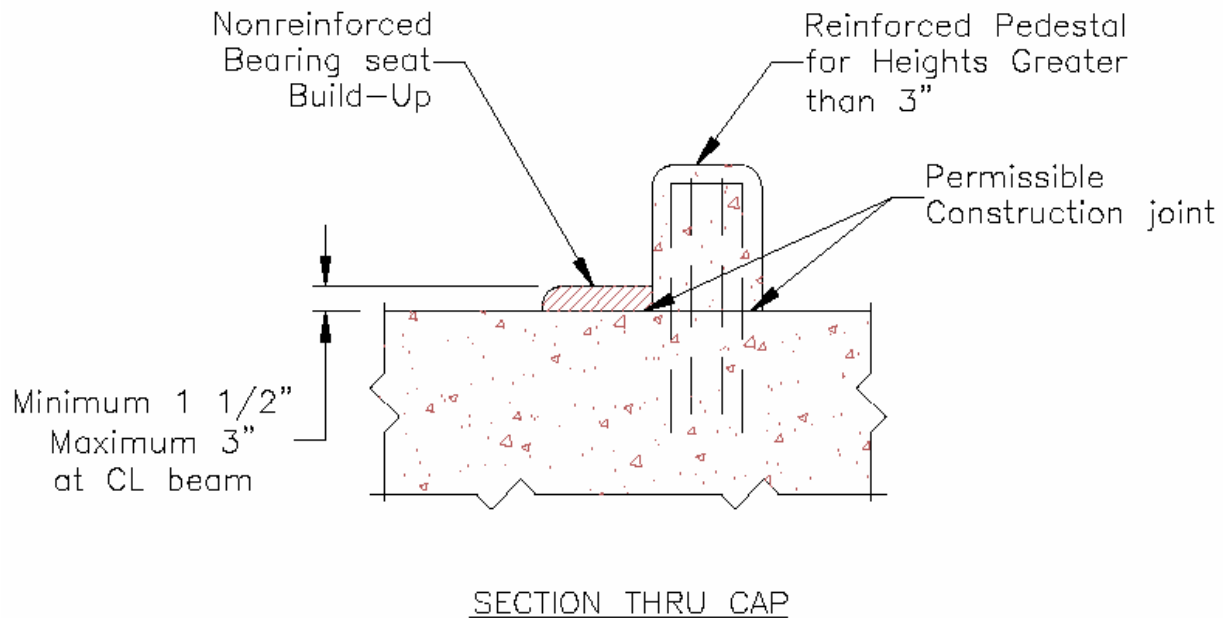
The tops of caps and piers between bearing areas shall be sloped slightly from the center toward the edge, and the tops of abutments and transition bents sloped from the backwall to the edge, as directed by the City, so that the water drains from the surface. The concrete shall be given a smooth trowel finish. When shown on the Plans, the top of caps and piers shall be coated with Type X epoxy material except for areas under shoes and bearing pads. Unless otherwise shown on the Plans, the color shall be concrete gray. The color of the epoxy may be adjusted to concrete gray by the use of a black universal type tinting paste. Bearing areas for steel units shall be constructed in accordance with TxDOT Item 441, "Steel Structures".

Bearing seat build-ups or pedestals for concrete units may be cast integrally with the cap or with a construction joint as follows:

The bearing seat build-ups shall be constructed of a latex-based mortar or an epoxy mortar, mixed in accordance with the manufacturer's recommendation. Pedestals shall be constructed of Class "C" concrete, reinforced as shown on the Plans.

Bearing areas under elastomeric pads or non-reinforced bearing seat build-ups shall be given a textured, wood float finish.

FIGURE 1



420.19. FINISH OF ROADWAY SLABS.

In all roadway slab-finishing operations, camber for specified vertical curvature and transverse slopes shall be provided.

For concrete slab or concrete slab girder spans cast in place on falsework, an additional amount of camber shall be provided to offset the initial and final deflections of the span. The additional amount of camber shall be determined from the dead load deflection diagram shown on the Plans. When dead load deflection is not shown on the Plans, the additional amount of camber shall be one-eighth (1/8) inch per ten (10) foot of span length but not to exceed one-half (1/2) inch. For pan girder spans, the additional camber for initial and final deflections shall be approximately one-half (1/2) inch for 30-foot spans and five-eighths (5/8) inch for 40-foot spans unless otherwise directed by the City.

Roadway slabs supported on prestressed concrete, steel beams or girders shall receive no additional camber, except that for slabs without vertical curvature, the longitudinal camber shall be approximately one-fourth (1/4) inch.

Dead load deflection shall be taken into account in setting the grades of headers and rail systems.

Work bridges or other suitable facilities shall be provided by the Contractor from which to perform all finishing operations and check measurements for slab thickness and reinforcement cover.

As soon as the concrete has been placed and vibrated in a section of sufficient width to permit working, the surface shall be approximately leveled, struck off and screeded, carrying a slight

excess of concrete ahead of the screed to insure filling of all low spots. The screed shall be rigid enough to hold true to shape and shall have sufficient adjustments to provide for the required camber or section. A vibrating screed may be used if heavy enough to prevent undue distortion. The screeds, except those of the roller drum type, shall be provided with metal cutting edges.

Longitudinal screeds shall be moved across the concrete with a saw-like motion while their ends rest on headers or templates set true to the roadway grade or on the adjacent finished slab.

The surface of the concrete shall be screeded a sufficient number of times and at such intervals to produce a uniform surface, true to grade and free of voids.

If necessary, the screeded surface shall be worked to a smooth finish with a long handled wood or metal float, or hand floated from bridges over the slab.

When required by the City, the Contractor shall perform sufficient checks with a long handled ten 10 foot straightedge on the plastic concrete to insure that the final surface will be within the tolerances specified below. The check shall be made with the straightedge parallel to the centerline. Each pass thereof shall lap half of the preceding pass. All high spots shall be removed and all depressions over one-sixteenth ($1/16$) inch in depth shall be filled with fresh concrete and floated. The checking and floating shall be continued until the surface is true to grade and free of depressions, high spots, voids or rough spots.

Rail support holes shall be filled with concrete and finished to match the top of the slab.

Unless otherwise shown on the Plans, when no additional wearing course is to be placed, the bridge deck surface shall be given a grooved steel tine finish. The grooves shall be approximately one-eighth ($1/8$) to three-sixteenth ($3/16$)-inch deep, approximately one-eighth ($1/8$) inch wide. The tines shall be randomly spaced approximately three-fourths ($3/4$) to one (1) inch apart. The grooves shall run perpendicular to the structure centerline when a longitudinal screed is used. Areas, which receive insufficient texture depth, shall receive additional texturing, when directed by the City, by saw grooving in accordance with the procedure given below.

At the option of the Contractor, or when shown on the Plans, the surface shall be given its final texture by saw grooving to meet the above requirements. Saw grooving may be done a minimum of four days after the slab concrete has been placed. If saw grooving is done prior to the completion of curing, the curing shall be continued after sawing to provide the minimum curing time required.

When shown on the Plans that a concrete overlay is to be placed on the slab (new construction), or on prestressed concrete box beams or other precast elements, the slab or the top surface of shear key and diaphragm concrete shall be given a broom finish. The finish shall have an average texture depth of approximately 0.035 inches with any individual test, not falling below 0.020 inches unless otherwise shown on the Plans, when tested in accordance with Test Method Tex-436-A. Should the texture depth fall below that intended, the finishing procedures should be revised to produce the desired texture.

When the Plans require that an asphaltic seal, with or without overlay, on the slab (new construction), or on prestressed concrete box beams or other precast elements, the slab or top surface of shear key and diaphragm concrete shall be given a lightly textured broom finish having

an average texture depth of approximately 0.025 inches when tested in accordance with Test Method Tex-436-A.

Straightedge requirements will be required on slabs (new construction) to be overlaid.

After the concrete slab has attained final set, the City may require that the finished surface be tested with a standard ten (10) foot straightedge. The straightedge shall be used parallel to the centerline of the structure to bridge any depressions and touch high spots. Ordinates of the irregularities, measured from the face of the straightedge to the surface of the slab, should normally not exceed one-eighth (1/8) of an inch, making proper allowances for camber. Vertical curve and surface texture; however, occasional variations exceeding this will be acceptable if, in the opinion of the City, the variations will not produce unacceptable riding qualities.

When directed by the City, irregularities exceeding the above shall be corrected. Areas that are corrected to produce satisfactory riding qualities shall be provided with an acceptable surface texture in a manner approved by the City.

420.20. CURING CONCRETE.

The Contractor shall inform the City of the methods proposed for curing; shall provide the proper equipment and material in adequate amounts; and shall have the proposed methods, equipment and material approved prior to placing concrete.

Unless otherwise noted herein or shown on the Plans, the choice of curing methods shall be at the option of the Contractor, except that the City may require the same curing methods for like portions of a single structure.

Inadequate curing and/or facilities shall be cause for the City to delay all concrete placement on the job until remedial action is taken.

All concrete shall be cured for a period of four curing days except as noted herein.

**TABLE 2
EXCEPTION TO 4 DAY CURING**

Description	Type of cement	Required Curing Days
Upper Surfaces of bridge slabs, top slab of direct traffic culverts, and concrete overlays	I or III	8
	III or I/III*	10
	All types with fly ash	10
Concrete Piling Build-ups	All	6

*Meets the requirements of both Type I and Type III

When the air temperature is expected to drop below 40°F, the concrete shall be covered with polyethylene sheeting, burlap-polyethylene blankets, mats or other acceptable materials to provide the protection required by Section 420.12.

A curing day is defined as a calendar day when the temperature, taken in the shade away from artificial heat, is above 50°F for at least 19 hours, or on colder days if satisfactory provisions are made to maintain the temperature of all surfaces of the concrete above 40°F for the entire 24 hours. The required curing period shall begin when all concrete therein has attained its initial set.

The following methods are permitted for curing concrete subject to the requirements of Table 3 and the following additional requirements for each method of curing:

(1) Form Curing. When forms are left in contact with the concrete, other curing methods will not be required except for exposed surfaces and for cold weather protection.

(2) Water Curing. All exposed surfaces of the concrete shall be kept wet continuously for the required curing time. The water used for curing shall meet the requirements for concrete mixing water as specified in Item 421, "Portland Cement Concrete". Seawater will not be permitted. Water, which stains or leaves an unsightly residue, shall not be used.

(a) Wet Mat Curing. This curing method shall consist of keeping the concrete continuously wet by maintaining wet cotton mats in direct contact with the concrete for the required curing time. Damp burlap blankets made from nine (9) ounce stock may be placed on the damp concrete surface for temporary protection prior to the application of cotton mats. The cotton mats may then be placed dry and wetted down immediately after they are placed. The mats shall be weighted down adequately to provide continuous contact with all concrete where possible. Surface, which cannot be cured by direct contact, shall be covered with mats forming an enclosure well anchored to the forms or ground so that outside air cannot enter the enclosure. Sufficient moisture shall be provided inside the enclosure to keep all surfaces of the concrete wet. Wet mat curing will be required for Part A in Table 3 when the anticipated ambient temperature is expected to remain above 40° F for the first 72 hours of the curing period.

Polyethylene sheeting, burlap-polyethylene blankets, laminated mats or insulating curing mats placed in direct contact with the slab will be required when the air temperature is expected to drop below 40°F during the first 72 hours of the curing period. These curing materials shall be weighted down with dry mats to maintain direct contact with the concrete and to provide insulation against cold weather. Supplemental heating or insulation may be required in cold and/or wet weather if the insulating cotton mats become wet or if the concrete drops below the specified curing temperature.

(b) Water Spray. This curing method shall consist of overlapping sprays or sprinklers that keep all unformed surfaces continuously wet.

(c) Ponding. This curing method requires the covering of the surfaces with a minimum of two inches of clean granular material, kept wet at all times, or a minimum of one (1) inch depth of water. Satisfactory provisions shall be made to provide a dam to retain the water or saturated granular material.

(3) Membrane Curing. Unless otherwise provided herein or shown on the Plans, either Type 1-D or Type 2, membrane curing compound may be used where membrane curing is permitted, except that Type 1-D (Resin Base Only), will be required for bridge slabs and top slabs of direct traffic culverts. All other surfaces which may require a higher grade of surface finish.

**TABLE 3
CURING REQUIREMENTS**

STRUCTURE UNIT DESCRIPTION	REQUIRED		PERMITTED	
	Water for Complete Curing	Membrane for Interim Curing	Water for Complete Curing	Membrane for Interim Curing
A. Upper surfaces of Bridge Roadway, Median and Sidewalk slabs of Direct Traffic Culverts	X	X (Resin Base)		
B. Top Surface of any Concrete Unit upon which Concrete is to be placed and bonded at a later interval (Stub Walls, Risers, etc.).	X			
C. All Substructure Concrete, Culverts, Box Sewers, Inlets, Manholes, Retaining Walls, Riprap, Railing			*X	*X
All other concrete	As specified in other items.			

*Polyethylene Sheeting, Burlap-Polyethylene Mats or Laminated Mats in close intimate contact with the concrete surfaces will be considered equivalent to water or membrane curing.

For substructure concrete only one type of curing compound will be permitted on any one structure. Material requirements and construction methods shall be as required by TxDOT Item 526, "Membrane Curing", except as changed herein.

Membrane curing shall not be applied to dry surfaces, but shall be applied just after free moisture has disappeared. Formed surfaces and surfaces, which have been given a first rub, shall be dampened and shall be moist at the time of application of the membrane.

When membrane is used for complete curing, the film shall remain unbroken for the minimum curing period specified. Membrane, which is damaged, shall be corrected immediately by reapplication of membrane. Unless otherwise noted herein or shown on the Plans, the choice of membrane type shall be at the option of the Contractor.

420.21. REMOVAL OF FORMS AND FALSEWORK.

Except as herein provided, forms for vertical surfaces may be removed when the concrete has aged not less than 12 hours, provided the removal can be done without damage to the concrete.

Forms for inside curb faces may be removed at such time the removal can be done without damage to the curb.

Weight supporting forms and falsework for all bridge components and culvert slabs, except as noted herein, shall remain in place a minimum of four curing days. The forms then may be removed if the concrete has attained a flexural strength of 425 psi, as evidenced by strength tests using test beams made from the same concrete and cured under the same conditions as the portion of the structure involved. Forms for other structural components may be removed as specified by the City.

Inside forms (walls and top slabs) for box culverts and sewers may be removed after concrete has aged not less than one day (24 hrs.) and has acquired a flexural strength of not less than 255 psi, provided an overhead support system, approved by the City, is used to transfer the weight of the top slab to the walls of the box culvert or sewer before the support provided by the forms is removed.

When all test beams made for the purpose of form removal have been broken without attaining the required strength, forms shall remain in place for a total of 14 curing days.

The above provisions relative to form removal shall apply only to forms or parts thereof which are constructed to permit removal without disturbing forms or falsework required to be left in place for a longer period on other portions of the structure.

All forms and falsework shall be removed unless otherwise approved by the City.

420.22. DEFECTIVE WORK.

Any defective work shall be repaired as soon as possible.

Any defect, which in the opinion of the City cannot be repaired satisfactorily to the extent required by the City, shall be removed and replaced at the expense of the Contractor.

420.23. FINISHING EXPOSED SURFACES.

A Surface Finish shall be applied to all concrete surfaces and shall be in accordance with TxDOT Item 427, "Surface Finishes for Concrete".

420.24. MEASUREMENT.

The quantities of concrete of the various classifications which will constitute the completed and accepted structure or structures in place will be measured by the cubic yard, each, square foot, square yard, or linear foot as shown on the Plans. Measurement will be as follows:

(1) General.

(a) All concrete quantities will be placed on the dimensions shown on the Plans or those established in writing by the City. Diaphragm concrete, when required, will be included in the slab measurement.

(b) In all determining quantities, no deductions will be made for chamfers less than two (2) inches, embedded portions of structural steel or prestressed concrete beams, piling, anchor bolts, reinforcing steel, drains, weep holes, junction boxes, electrical or telephone conduit, conduit and/or voids for prestressed tendons or for embedded portions of light fixtures.

(c) For pan girder spans, a quantity will be included for the screed setting required providing proper camber in the roadway surface after form removal.

(d) For slabs on steel and prestressed beams, a quantity for the haunch between the slab and beams will be included when required. No measurement will be made during construction for variation in the amount of haunch concrete due to deviation from design camber in the beams.

(e) For slabs on panels, T-beams, or box beams, the combination of span length, theoretical camber in beams, computed deflections, and plan vertical curve will be taken into account in determining the quantity for the slab.

Additional concrete, which may be required by an adjustment of the profile grade line during construction, to insure proper slab thickness, will not be measured for payment.

(f) Variation in concrete headwall quantity incurred when an alternate bid for pipe is permitted will not be measured for payment.

(g) Quantities revised by a change in design, measured as specified herein, will be increased or decreased, as the case may be, and included for payment.

(2) Plan Quantity. For structure elements designated in Table 4, and when measured by the cubic yard, this is a Plans quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans. If no adjustment of quantities is required, additional measurements or calculations will not be required.

When the quantity for a complete structure element has been erroneously included or omitted from the Plans, the quantity shown on the Plans for that element will be added to or deducted from the plan quantity and included for payment. A complete structure element will be the smallest portion of a total structure for which a quantity is included on the Plans.

When the plan quantity for a complete structure element is in error by five (5) percent or more, a recalculation will be made and the corrected quantity included for payment.

(3) Measured in Place. For those items not measured for plan quantity payment, measurement will be made in place.

TABLE 4
PLAN QUANTITY PAYMENT
(CUBIC YARD MEASUREMENT ONLY)

Culverts and Wingwalls	Slabs on Steel Spans
Headwalls for pipe	Slabs on Prestressed Spans
Retaining Walls	Pan Girder Spans
Inlets and Manholes	Pile Bent Caps
Slab Spans	Shear Key Concrete
Slab and Girder Spans	Abutments

Note: Other structure elements may be paid for as “plan quantity”, including pier and bent concrete, when shown on the Plans.

For those portions of structures not listed in Table 4, the concrete quantities, measured as provided in Sub Section 420.24.(1) will be paid for at the unit price bid per “Cubic Yard”, per “Each”, per “Square Foot”, per “Square Yard”, or per “Linear Foot”, in place, for the various classifications of concrete shown.

420.25. PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under “Measurement” will be paid for at the unit price bid for the various structure elements specified of the various classes of concrete. This price shall be full compensation for furnishing, hauling and mixing all concrete materials; for furnishing, bending, fabricating, splicing, welding and placing the required reinforcement; for all clips, blocks, metal spacers, ties, wire or other materials used for fastening reinforcement in place; for placing, finishing and curing all concrete; for all grouting and pointing; for furnishing and placing drains; for furnishing and placing metal flashing strips; for furnishing and placing expansion-joint material required by this Item; and for all forms and falsework, labor, tools, equipment and incidentals necessary to complete the work.

Concrete which fails to meet minimum strength requirements may be rejected or a structural review may be made by the City. Such concrete which is proven structurally adequate may be accepted at an adjusted price based on the following formula:

$$A = 0.10 \times Bp + 0.75 \times (Sa/Ss)^2 \times Bp$$

Where:

A = Amount to be paid per unit of measure

Bp = Unit bid price

Sa = Actual strength from beams or cores (average value if more than one test taken)

Ss = Minimum required strength (specified)

CITY OF SAN ANGELO**ITEM 421****PORTLAND CEMENT CONCRETE****421.1. DESCRIPTION.**

This Item shall govern for Portland Cement Concrete to be used in concrete pavement, concrete structures and other concrete construction.

421.2. MATERIALS.

The concrete shall be composed of Portland Cement, (with or without) fly ash, fine and coarse aggregates and water.

(1) Cement. Portland Cement shall conform to TxDOT Item 524, "Hydraulic Cement".

(2) Mixing Water. Water for use in concrete and for curing shall be free from oils, acids, organic matter or other deleterious substances and shall not contain more than 1000 parts per million of chlorides as Cl nor more than 1000 parts per million of sulfates as SO₄.

Water from municipal supplies approved by the State Health Department will not require testing, but water from other sources will be sampled and tested before use in concrete. Tests shall be made in accordance with AASHTO T26.

Water used in white Portland Cement Concrete shall be free from iron and other impurities which may cause staining or discoloration.

(3) Coarse Aggregate. Coarse aggregate shall be washed and shall consist of durable particles of gravel, crushed blast furnace slag, crushed stone, or combinations thereof and shall be free from frozen material or injurious amounts of slat, alkali, vegetable matter, or other objectionable material either free or as an adherent coating. When white Portland cement is specified, the coarse aggregates used in the concrete shall be light colored. Quality shall be reasonably uniform throughout. Coarse aggregate shall not contain more than 0.25 percent by weight of clay lumps, nor more than one (1) percent by weight of shale, nor more than five (5) percent by weight of laminated and/or friable particles when tested in accordance with Test Method Tex-413-A. Coarse aggregate from each source shall have a wear of not more than 40 percent when tested in accordance with Test Method Tex-410-A.

When tested in accordance with Test Method Tex-401-A, the coarse aggregate, including combinations of aggregates when used, shall conform to the gradation requirements shown in Table 1.

TABLE 1
COARSE AGGREGATE GRADATION CHART

Aggregate Grade No.	Nominal Size Inches	Percent Retained on Each Sieve								
		2-1/2 in.	2 in.	1-1/2 in.	1 in.	3/4 in.	1/2 in.	3/8 in.	No. 4	No. 8
1	2	0	0-20	15-50		60-80			95-100	
2	1-1/2		0	0-5		30-65		70-90	95-100	
3	1-1/2		0	0-5		10-40	40-75		95-100	

The loss by decantation in accordance with Test Method Tex-406-A plus the allowable weight of clay lumps, shall not exceed one (1) percent, or the value shown on the Plans, whichever is smaller. In the case of aggregates made primarily from the crushing of stone, if the material finer than the 200 sieve is definitely established to be the dust of fracture, essentially free from clay or shale, as established by Part III of Test Method Tex-406-A, the percent may be increased to 1.5.

(4) Fine Aggregate. Fine aggregate shall be washed and consist of clean, hard, durable and uncoated particles of natural or manufactured sand or a combination thereof, with or without a mineral filler. When white Portland Cement is specified the fine aggregate used in the concrete shall be light colored. It shall be free from frozen material or injurious amounts of salt, alkali, vegetable matter or other objectionable material and it shall not contain more than 0.5 percent by weight of clay lumps. When the aggregate is subjected to the color test for organic impurities in accordance with Test Method Tex-408-A, the test result shall not show a color darker than standard.

Unless otherwise shown on the Plans, the acid insoluble residue of fine aggregate used in concrete subject to direct traffic shall be not less than 60 percent by weight when tested in accordance with Test Method Tex-612-J.

When tested in accordance with Test Method Tex-401-A, the fine aggregate or combinations of aggregates, including mineral filler, shall conform to the gradation requirements shown in Table 2.

TABLE 2
FINE AGGREGATE GRADATION CHART

Aggregate Grade No.	Percent Retained on Each Sieve							
	3/8 in.	No. 4	No. 8	No. 16	No. 30	No. 50	No. 100	No. 200
1	0	0 to 5	0 to 20	15 to 50	35 to 75	65 to 90	90 to 100	97 to 100

Where manufactured sand is used in lieu of natural sand, the percent retained on the No. 200 sieve shall be 94 to 100.

Where the sand equivalent value is greater than 85, the retainage on the No. 50 sieve may be 65 to 94 percent.

Fine aggregate will be subjected to the Sand Equivalent Test (Test Method Tex-203-F / ASTM D 2419). The sand equivalent shall not be less than 80 unless otherwise shown on the Plans.

For all classes of concrete, the fineness modulus shall be between 2.30 and 3.10 as determined by Test Method Tex-402-A.

(5) Mineral Filler. Mineral filler shall consist of stone dust, clean crushed sand, or other approved inert material. When tested in accordance with Test Method Tex-401-A, it shall conform to the following gradation:

Retained on No. 30 Sieve	0 percent
Retained on No. 200 Sieve	0-35 percent

(6) Admixtures. Admixtures and their use shall conform to the requirements of TxDOT Item 437, "Concrete Admixtures". Calcium chloride will not be permitted.

(7) Mortar and Grout. Unless otherwise specified or approved by the City, mortar and grout shall consist of one part Portland cement, two parts finely graded sand and sufficient water to provide the desired consistency. Mortar may contain admixtures.

Post tensioning grout shall be in accordance with TxDOT Item 426, "Prestressing".

Mortar shall have a consistency such that the mortar can be easily handled and spread by trowel. Grout shall have a consistency such that the grout will flow into and completely fill all voids.

When required to prevent color difference, white cement shall be added to produce the color required. When shown on the Plans or in the specifications, or when required by the City, latex adhesive conforming to the requirements of TxDOT Departmental Material Specification D-9-8110 shall be added to the mortar.

421.3. STORAGE OF MATERIALS.

(1) Cement, and Mineral Filler. All cement and mineral filler shall be stored in well-ventilated weatherproof buildings or approved bins, which will protect them from dampness or absorption of moisture. Each shipment of packaged cement shall be kept separated to provide easy access for identification and inspection.

(2) Aggregates. The method of handling and storing concrete aggregates shall prevent contamination with foreign materials. If the aggregates are stored on the ground, the sites for the stockpiles shall be clear of all vegetation and shall be level. The bottom six-inch layer of aggregate shall not be disturbed or used without re-cleaning.

(3) **Admixtures.** Admixtures shall be stored in accordance with TxDOT Item 437, “Concrete Admixtures”.

421.4. MEASUREMENT OF MATERIALS.

Except as noted below, the measurement of materials used in batches of concrete shall be by weight.

Water may be measured by volume or by weight.

Cement shall be weighed separately from other materials. Weighing of sacked cement will not be required. When sacked cement is used, the quantity of cement per batch shall be based upon using full bags of cement. Batches involving use of fractional bags will not be permitted except for small hand mixed batches of approximately five (5) cubic feet or less and when an approved method of volumetric measurement is used.

Where two (2) or more sizes or types of aggregates are used, each type and/or size shall be measured separately.

When determining aggregate batch weights, proper allowance shall be made for the water content in the aggregate (free water and/or absorption).

Admixtures shall be measured and dispensed in accordance with TxDOT Item 437, “Concrete Admixtures”.

Measuring materials by volumetric methods may be used where permitted by the specifications. When a mixer using volumetric batching of materials is used, an accurate method of measuring by volume shall be provided. Continuous volumetric mixers shall be calibrated to assure correct measurement of materials.

The amount of each ingredient in the batch shall be measured to within plus or minus one percent of required amount except that water shall be measured to within plus or minus one gallon and admixture tolerances shall be in accordance with TxDOT Item 437, “Concrete Admixtures”.

421.5. EQUIPMENT.

(1) **Weighing and Measuring Equipment.** Weighing and measuring equipment shall conform to TxDOT Item 520, “Weighing and Measuring Equipment”.

(2) Mixing Equipment.

(a) General. All equipment, tools, and machinery used for hauling materials and performing any part of the work shall be maintained in such condition as to insure completion of the work under way without excessive delays for repairs or replacement.

The mixer shall be of an approved type and size that will produce uniform distribution of the material throughout the mass and shall be capable of producing concrete meeting the requirements of these specifications.

For all mixers, an adequate water supply and an accurate method of measuring the water shall be provided.

Delivery of concrete to the work site and the discharge from the hauling equipment, agitating, or non-agitating, shall be in accordance with the requirements shown on the Plans or in the governing specifications.

Specific requirements for batch plants, mixers and other equipment shall be in accordance with Item 360, "Concrete Pavement", or TxDOT Item 522, "Portland Cement Concrete plants", or other specifications.

421.6. MIXING.

(1) General. Mixed concrete, which does not conform to specification requirements, shall not be placed. Mixing shall be in accordance with TxDOT Item 522, "Portland Cement Concrete plants", except that mixing with continuous volumetric mixers will be in accordance with Section 421.6.(2) and except as set out in Section 421.6.(3).

(2) Continuous Volumetric Mixers. Mixing shall be in accordance with mixer manufacturer's recommendations unless otherwise revised by the City.

(3) Mixing of concrete by hand methods or by the use of a small motor driven mixer will be permitted for small placements of approximately two (2) cubic yards or less when authorized by the City. Hand mixed batches shall not exceed a two-sack batch in volume. For such placements the mix may be proportioned by approved volumetric methods.

421.7. CLASSIFICATION AND MIX DESIGN.

The Contractor shall furnish the mix design, using a coarse aggregate factor acceptable to the City, for the class(s) of concrete specified, to conform with the requirements contained herein and in accordance with Construction Bulletin C-11. The Contractor shall perform, at his entire expense, the work required to substantiate the design. Sampling and testing of concrete will be the responsibility of the City. Complete concrete design data shall be submitted to the City for approval.

The Contractor shall determine and measure the batch quantity of each ingredient, including all water, not only for batch designs but also for all concrete produced for the project. The mixes shall conform to these specifications and other requirements shown on the Plans.

The Contractor may accept a design from the City; however, this acceptance will not relieve the Contractor of the responsibility of providing concrete meeting the requirements of these specifications.

Mix designs from previous or concurrent jobs may be used without trial batches if it is shown that no substantial change in any of the proposed ingredients has been made.

No charge will be made for existing designs furnished by the City. The cost to the City of preparing a new mix design will be charged to the Contractor and deducted from the payment for the work.

Concrete for pneumatically placed concrete shall be in accordance with Item 431, “Pneumatically Placed Concrete”.

The Contractor shall have the option of using chemical admixtures with all classes of concrete in accordance with Item 437, “Concrete Admixtures”, except where the use of specific admixtures is required or prohibited in this or other items. For example, a mid-range water reducer can help maintain the water/cement ratio below the maximum allowed and provide a better workability to the concrete mix than if a mid-range water reducer was not used.

When a retarding admixture is required for hot weather concreting, the amount to be used will be as required in TxDOT Item 437, “Concrete Admixtures”, subject to change by the City when required. When used for extended retardation, the amount to be used will be established by several trial batches with varying retarder content and simulating the placing conditions to be encountered and tested in accordance with Tex-440-A.

When entrained air is required, the concrete shall be designed to entrain five (5) percent air when Grade 1 or 2 coarse aggregate is used, six (6) percent when Grade 3 or 4 coarse aggregate is used, and seven (7) percent for Grades 5,6 or 7 unless otherwise specified by the City. Concrete as placed shall contain the proper amount of entrained air as required herein with a tolerance of plus or minus 1-1/2-percentage points. Acceptance of concrete with occasional variations between 1-1/2 and three (3) percentage points over the specified amount will be based on strength tests as required by the City. When the quantity of entrained air is found to be more than three (3) percentage points over or two (2) percentage points under those values given herein, the concrete will be rejected.

Entrained air will be required for bridge slabs, top slabs of direct traffic culverts, concrete pavement, dense and regular concrete overlays, piers, bents, precast piling (non-prestressed), drilled shafts placed in water, bridge railing, concrete traffic barrier and for other items of work as may be specified, on the Plans or in other specifications.

**TABLE 3
SLUMP REQUIREMENTS**

Concrete Designation	Desired Slump Inches	Max Slump Inches
A. Structural Concrete		
(1) All drilled shafts	6	7
(2) Thin-Walled Section (9" or less)	4	5
(3) Slabs, Concrete Overlay, Caps, Columns, piers, Wall sections over 9", etc.	3	4
(4) Prestressed Concrete Members	4	5
(5) Concrete traffic Barrier (cast-in-place or precast), Concrete Bridge Railing	4	5
(6) Dense concrete overlay	¾	1
(7) Concrete placed underwater	6	7
(8) Concrete with High Range Water Reducer	-	8
B. Concrete Pavement	1-1/2	3 max 1 min
C. Riprap, curb, gutter, slipformed and extruded concrete	As Approved by the City	

Note: No concrete will be permitted with a slump in excess of the maximums shown. When high-range water reducing admixtures are used, the slump shall not exceed eight (8) inches.

421.8. QUALITY OF CONCRETE.

The concrete shall be uniform, workable and of a consistency acceptable to the City. The cement content, maximum allowable water/cement ratio, the desired and maximum slump, the proper amount of entrained air and the strength requirement for all classes of concrete shall be the responsibility of the Contractor to provide concrete meeting these requirements.

During the progress of the work, a certified testing laboratory technician will cast test cylinders and/or beams, perform slump and entrained air tests and will make temperature checks, as required, to ensure compliance with the specifications.

Unless otherwise shown on the Plans, the Contractor shall furnish and properly maintain all test molds. The test molds shall meet the requirements of Test Methods Tex-418-A / ASTM C 39 and Tex-448-A / ASTM C 78 and, in the opinion of the City, must be satisfactory for use at the time of use. In addition, the Contractor shall be responsible for furnishing personnel to remove the test specimens from the molds and transport them to the proper curing location at the schedule designated by the City and in accordance with the governing specification. For all concrete items

the Contractor shall have a wheelbarrow, or other container acceptable to the City, available to use in the sampling of the concrete.

All labor and equipment furnished by the Contractor will be considered subsidiary to the various bid items and will not be paid for directly.

A strength test is defined as the average of the breaking strength of two (2) cylinders or two (2) beams as the case may be. Each specimen will be tested in accordance with Test Methods Tex-418-A / ASTM C 39 or Tex-448-A / ASTM C 78.

Slump tests will be performed in accordance with Test Method Tex-415-A / ASTM C 143. Entrained air tests will perform in accordance with Test Method Tex-416-A / ASTM C 231.

If the required strength or consistency of the class of concrete being produced cannot be secured with the minimum cement specified or without exceeding the maximum water/cement ratio, the Contractor will be required to furnish different aggregates, use a water reducing agent, an air entraining agent or increase the cement content in order to provide concrete meeting these specifications.

All test specimens, beams or cylinders, representing tests for removal of forms and/or falsework shall be cured using the same methods and under the same conditions as the concrete represented.

“Design Strength” beams and cylinders shall be cast and cured in accordance with Test Method Tex-447-A / ASTM C 31.

When the specified concrete strength is by 28-day compressive strength tests, job control testing will be by 7-day compressive strength tests. The minimum strength requirement for 7-day tests will be 70 percent of the specified minimum 28-day compressive strength. If the required 7-day strength is not obtained with the quantity of cement specified in Table 4, changes in the batch design will be made as specified in this article. For an occasional failure of the 7-day compressive test, the concrete may be tested at 28-days for final evaluation. Strength test requirements for Type II cement will govern when Type I/II cement is used.

TABLE 4
CLASSES OF CONCRETE

Class of Concrete	Cement per C.Y. Min. (sacks)	Min. Comp. Strength 28 day psi	Min. Flexural Strength 7 day psi	Max. Water / Cement Ratio Gal/sk	Coarse Aggregate Grade No.	General Usage (information only)
A	5.0	3,000	425 390 (c)	6.5	1-2- 3-4- 8 (a) (d)	Drilled Shafts; Culverts, except Top Slab of Direct Traffic Culverts; Inlets; Manholes, Headwalls; Approach Slabs; Curb; Gutter, Conc. Retards; Sidewalks; Driveways; Conc. Pavement; Back-up Walls; Anchors
B	4.0	2,000	280	8.0	2-3-4-5- 6-7	Riprap, Small Roadside Signs and Anchors
C	6.0	3,600	510 470 (c)	6.0	1-2-3-4- 5 (d)	Drilled Shafts; Bridge Substructure; Bridge Railing; Culverts, except Top Slab of Direct Traffic Culverts; Wing Walls; Approach Slab; Concrete Traffic Barrier (cast-in-place)
D	2.0 to 3.0	1,000 to 1,500	215±	11.0±	2-3-4-5- 6-7	Riprap Fill Voids Fills
E	6.0	3,000	425	6.0	2-3-4-5	Seal Concrete
S	6.5	4,000	570 525 (c)	5.0	2-3-4-5	Bridge Slab; Top Slab of Direct Traffic Culvert; Bridge Substructure
P	5.0	3,500	555 (b)	6.25	2-3	Concrete Pavement
DC	8.75	5,500	720	3.6	6	Dense Concrete Overlay
CO	7.0	4,600	640	4.5	6	Concrete Overlay

- (a) Grade 8 aggregate for use in extruded curbs, unless the City approves a larger size.
- (b) Minimum running average for concrete pavement (in accordance with Construction Bulletin C-II).
- (c) When Type II or Type I/II cement is used.
- (d) Unless otherwise permitted by the City, Grade I coarse aggregate may be used only in massive foundations with four (4) inch minimum clear spacing between reinforcing steel bars. Grade I aggregate may be used in drilled shafts.

421.9. MEASUREMENT AND PAYMENT.

The work performed, materials furnished and all labor, tools, equipment and incidentals necessary to complete the work under this Item will not be measured or paid for directly, but will be considered subsidiary to the various bid items of the contract.

CITY OF SAN ANGELO
ITEM 433
JOINT SEALANTS AND FILLERS

433.1. DESCRIPTION.

This Item shall govern the material requirements for joint sealants, backing materials and joint fillers.

433.2. MATERIALS.

The materials for this item shall conform to the following:

(1) Joint Sealant Materials. Joint sealant material shall be the class indicated on the Plans or in the governing specifications. The various classes of sealant described herein shall be in accordance with TxDOT Material Specification D-9-6310. Copies of specification D-9-6310 are available from TxDOT, Director of Materials and Tests, 125 E. 11th Street, Austin, TX 78701-2483.

(2) Storage. Class 1 and 2 sealants shall be stored at temperatures between 40°F and 100°F. Class 4 and 5 sealants shall be stored in sealed containers at a temperature of 100°F or below and the material must be used within two (2) months of receipt on the project.

(3) Classes of Joint Sealants.

(a) Class 1. Two Component, Synthetic Polymer, Non-sag. The components shall be proportioned and mixed in accordance with the manufacturer's recommendations.

(b) Class 2. Two Component, Synthetic Polymer, Self-leveling. The components shall be proportioned and mixed in accordance with the manufacturer's recommendations.

(c) Class 3. Hot Poured Rubber. This sealant shall be a rubber asphalt compound which when heated shall melt to the proper consistency for pouring and shall solidify on cooling to ambient temperatures.

(d) Class 4. Non-sag Low Modulus Silicone. The material shall be a single component formulation not requiring addition of a catalyst.

(e) Class 5. Self-leveling Low Modulus Silicone. The material shall be a single component formulation not requiring addition of a catalyst.

(f) Class 6. Preformed Joint Sealant (PJS). The preformed joint sealant shall be an extruded elastomeric material having a multi-channeled shape.

(g) Class 7. Self-leveling, Rapid Curing, Low Modulus Silicone. The material shall be a two component, rapid curing, self-leveling, low modulus formulation. The components shall be proportioned and mixed in accordance with the manufacturer's recommendations.

The size shown on the Plans shall be the nominal width of the sealant. The uncompressed depth of the seal shall be equal to or greater than the width.

All preformed joint sealants installed by the Contractor shall have been pre-qualified for compliance with the requirements. Each size and configuration of seal produced by a manufacturer must be approved by the City prior to use on City of San Angelo projects. For a sealant manufacturer to pre-qualify and obtain approval of a sealant, detailed dimensions and configuration of each size of sealant and certified test results indicating compliance with TxDOT Material Specification D-9-6310 and any requirements shown on the Plans and specifications shall be submitted to the City.

Submission shall be done sufficiently in advance of work to allow for testing and evaluation of the material.

The City will confirm by visual inspection that the sealant proposed for installation is the same size, configuration and manufacture as shown on Plans. The City will examine the sealant for any undue distortions, such as dissymmetry, warping, thick webs or uneven width, which are likely to impair the performance of the joint. If the magnitudes of the distortions are sufficient to create doubt as to the performance of the sealant, the City may direct that the sealant be replaced or that samples representing the worst of the lot be subjected to further testing to verify their performance.

(4) Backer Rods and Backing Materials. These materials shall be capable of holding the fluid sealant in open joints in place. In all cases these materials shall be of such a type that will not bond to the sealant. The backing materials shall meet the requirements of the sealant manufacturer. They shall be compressible type materials, such as closed-cell, resilient foam or sponge rubber stock of vinyl, butyl or neoprene, or expanded polyethylene or polyurethane.

The diameter of the backer rod shall be at least 25 percent larger than the joint reservoir width.

(5) Joint Fillers. Joint fillers shall be of the size, shape and type indicated on the Plans and shall conform to the following requirements.

(a) Timber Boards. Timber boards shall be obtained from redwood, cypress, gum, southern yellow pine or Douglas fir timber. They shall be sound heartwood and shall be free from sapwood, knots, clustered bird's eye, checks and splits. Occasional sound or hollow bird's-eye, when not in clusters, will be permitted provided the board is free from any other defects that will impair its usefulness as a joint filler. All boards, except redwood and cypress, shall have a creosote or pentachlorophenol treatment conforming to TxDOT Item 492 "Timber Preservative and Treatment", Table 1. When oven dried at 230°F to a constant weight, the weight of the board per cubic foot (minus treatment) shall not be less than 20 pounds or more than 35 pounds.

(b) Asphalt Boards. Asphalt boards shall consist of two (2) suitable asphalt-impregnated liners filled with a mastic mixture of asphalt and vegetable fiber and/or mineral fiber. Asphalt boards shall be smooth, flat and sufficiently rigid to permit installation. When tested in accordance

with Test Method Tex-524-C, the horizontal deflection shall not be more than one inch in 3-1/2 inches.

(c) Preformed Fiber Material.

i. Preformed Bituminous Fiber Material. Preformed bituminous fiber material shall meet the requirements of ASTM D1751.

ii. Preformed Non-bituminous Fiber Material. Preformed non-bituminous fiber material shall meet the requirements of ASTM D1751, except that the requirements pertaining to bitumen content, density and water absorption shall be voided.

(d) Rebonded Neoprene Filler. Rebonded neoprene filler shall consist of ground closed-cell neoprene particles, rebonded and molded into sheets of uniform thickness of the dimensions shown on the Plans, meeting the requirements of ASTM D1752, Type 1. Certification that the material meets these requirements shall be furnished to the City.

433.3. MEASUREMENT.

The work performed, materials furnished and all labor, tools, equipment and incidentals necessary to complete the work.

433.4. PAYMENT.

This Item will be paid for by the linear feet as measured in place.

CITY OF SAN ANGELO**ITEM 437****CONCRETE ADMIXTURES****437.1 DESCRIPTION.**

This Item shall govern for the admixtures used in Portland Cement Concrete, in accordance with the Plans, specifications and requirements herein.

437.2 MATERIALS.

Admixtures shall be in a liquid state and conform to the following requirements:

Chemical Admixtures – ASTM C494
Air Entraining Admixtures – ASTM C260

437.3 APPROVAL OF ADMIXTURES.

The City reserves the right to perform any or all of the tests required by ASTM C260 and ASTM C494 as a check on the tests reported by the manufacturer. In case of any variance, the City test will govern. Any change in formulation of an admixture shall require retesting, and shall be approved by the City.

A change in formulation discovered by any of the tests prescribed herein, or other means, and not reported and retested, may be just cause to remove a manufacturer from the pre-certified list for City projects.

All documentation and correspondence shall be submitted to the City Engineer, San Angelo, Texas.

437.4 DISPENSING EQUIPMENT.

Each admixture shall be measured and dispensed by a separate readily adjustable dispenser. When set to a predetermined volume, the dispenser shall fill to the preset amount and hold it without leakage until the operator releases the content by some positive means. Unless otherwise shown on the Plans, completely automatic dispensing will not be required, except for use with a fully automatic Plant.

The calibrated container shall be constructed in such a manner that the level of the admixture is visible at all times. A strip gauge with one (1) ounce increments for air-entraining admixtures and ten (10) ounce increments for water reducing and/or retarding admixtures, shall be attached securely to the measuring apparatus. This strip shall be a material possessing weather-resistant qualities. The accuracy required for these systems shall be plus or minus three (3) percent. The equipment shall visibly show the total amount to be dispensed for ready check by the City.

For individual concrete placements of less than two (2) cubic yards at the placement site, the City may waive the requirements for mechanical dispensing equipment.

437.5 CONSTRUCTION USE OF ADMIXTURES.

When used in accordance with the governing specification, the Contractor will be allowed to use any admixture, which has been approved. The Contractor shall submit to the City one (1) copy of the invoice showing the admixture or admixtures to be used on the project. Prior to using an admixture in the work, trial mixes shall be made and tested in the field using the materials and equipment to be used on the project. If more than one (1) admixture is used, they shall be used in such manner that the desirable effects of each are realized.

Mix designs from previous or concurrent jobs may be used without trial batches if it is shown that no substantial change in any of the proposed ingredients has been made.

The volume of liquid admixtures shall be taken into account when determining the water/cement ratio of the mix.

The dosage rate for air-entraining admixtures shall be adjusted as necessary to produce the required air content in the concrete. The air content shall be in accordance with Item 421, "Portland Cement Concrete", unless otherwise shown on the Plans.

When a retarding admixture is required for extended retardation, the amount to be used shall be established by several trial batches with varying retarder content and simulating the placing conditions to be encountered. When water reducing or retarding agents are used at the option of the Contractor, reduce dosage of the admixture will be permitted.

Accelerators will be used only to meet special requirements and will require the written approval of the City on each specific project. Accelerating admixtures will not be permitted in bridge decks, top slab of direct traffic culverts, nor when Type II cement is specified.

All accelerating admixtures dosages will be based on trial mixes and approved by the City.

High range water reducers will be used only to meet special requirements and will require written approval of the City on each specific project. A Work Plan for control shall be submitted by the Contractor for approval and an evaluation of the concrete containing the admixture will be performed by the City. Recommended guidelines for developing a Work Plan are shown in TxDOT Construction Bulletin C-11.

Suitable measures shall be taken to prevent admixtures from freezing. Admixtures shall be agitated as required to prevent separation or sedimentation of solids. Air agitation of entraining agents will not be permitted.

Air entraining agents shall be charged into the mixer with the first one-third (1/3) of the mix water. Retarding or water reducing admixtures, except for high-range water reducers, shall be charged into the mixer during the last one-third of the mix water. Each admixture shall be dispensed separately but at the same time as the mixing water. No admixture shall be dispensed on dry aggregates.

Alternate charging sequences based on trial batches may be used subject to approval by the City.

High range water reducing admixtures shall be used and/or dispensed in accordance with the approved Work Plan.

Should the desired effects of an admixture not be achieved in the concrete, the City may take a sample of the admixture being used for further testing. Further use of the admixture will not be allowed until the results of such test confirm that the admixture has not been changed or altered in any way.

437.6 MEASUREMENT AND PAYMENT.

The work performed, materials furnished and all labor, tools, equipment, and incidentals necessary to complete the work under this Item will not be measured or paid of directly, but will be considered subsidiary to the various bid Item of the contract.

CITY OF SAN ANGELO**ITEM 440****REINFORCING STEEL****440.1. DESCRIPTION.**

This Item shall govern for the furnishing and placing of deformed and smooth reinforcing steel, of the sizes and details shown on the Plans and in accordance with this Item. All reinforcing steel to be epoxy coated will be designated on the Plans. Epoxy coating of reinforcing steel shall be in accordance with “Epoxy Coating of Reinforcing Steel” of this Item.

440.2. MATERIALS.

Unless otherwise shown on the Plans or specified herein, the reinforcing steel shall be Grade 60 and all bar reinforcement shall be deformed, conforming to one of the following:

- (1) ASTM A615, Grades 40 or 60, open hearth, basic oxygen, or electric furnace new billet steel.
- (2) ASTM A617, Grades 40 or 60, axle-steel.
- (3) ASTM A616, Grade 60, rail steel will be permitted in concrete pavement only. ASTM A616 bars shall be furnished as straight bars only and bending is prohibited. Bend tests will not be required.
- (4) ASTM A706, Grade 60, weldable reinforcing steel.
- (5) Smooth Bars. Smooth bars for concrete pavement shall have a minimum yield strength of 60 ksi.

All other smooth bars, larger than No. 4, may be steel conforming to the above or may be furnished in any steel that meets the physical requirements of ASTM A36.

- (6) Spiral reinforcement shall be either smooth or deformed bars, or wire, of the minimum size or gage shown on the Plans, or as specified herein.

Bars for spiral reinforcement shall comply with ASTM A675, Grade 80 (reference to ASTM A29 is voided) A615 or A617, Grade 40, unless otherwise shown on the Plans. Smooth wire shall comply with ASTM A82 and deformed wire shall comply with ASTM A496.

In cases where the provisions of the Item are in conflict with the provisions of the ASTM Specification, the provisions of this Item shall govern.

Reinforcing steel to be structurally welded shall comply with ASTM A706 or shall have a carbon equivalency (C.E.) of not more than 0.55%. A report of chemical analysis, showing the percentages of all elements necessary to establish the carbon equivalency, will be required for all reinforcing steel that is to be structurally welded. The above requirements do not pertain to miscellaneous welds on reinforcing steel as defined in Item 448, “Structural Field Welding”.

Carbon equivalency will be calculated using the following formula:

$$\text{C.E.} = \%C + \frac{\%MN}{6} + \frac{\%Cu}{40} + \frac{\%Ni}{20} + \frac{\%Cr}{10} - \frac{\%Mo}{50} - \frac{\%V}{10}$$

The nominal size, area and weight of reinforcing steel bars covered by this specification are as follows:

Bar Size Number	Nominal Diameter In.	Nominal Area Sq. In.	Weight Per Linear Foot
2	0.250	0.05	0.167
3	0.375	0.11	0.376
4	0.500	0.20	0.668
5	0.625	0.31	1.043
6	0.750	0.44	1.502
7	0.875	0.60	2.044
8	1.000	0.79	2.670
9	1.128	1.00	3.400
10	1.270	1.27	4.303
11	1.410	1.56	5.313
14	1.693	2.25	7.65
18	2.257	4.00	13.60

Smooth round bars shall be designated by size number through No. 4. Smooth bars above No. 4 shall be designated by diameter in inches.

(7) Wire for fabric reinforcement shall conform to ASTM A82 or A496. Wire fabric shall conform to ASTM A185 or A497.

When wire is ordered by size numbers, the following relation between size number, diameter in inches and area shall apply unless otherwise specified. Where deformed wire is required, the size number shall be preceded by "D", and for smooth wire the prefix shall be "W".

Size Number	Nominal DAMETER IN.	Nominal Area Sq. In.
31	0.628	0.310
30	0.618	0.300
28	0.597	0.280
26	0.575	0.260
24	0.553	0.240
22	0.529	0.220
20	0.505	0.200
18	0.479	0.180
16	0.451	0.160
14	0.422	0.140
12	0.391	0.120
10	0.357	0.100
8	0.319	0.080
7	0.299	0.070
6	0.276	0.060
5.5	0.265	0.055
5	0.252	0.050
4.5	0.239	0.045
4	0.226	0.040
3.5	0.211	0.035
3	0.195	0.030
2.5	0.178	0.025
2	0.160	0.020
1.5	0.138	0.015
1.2	0.124	0.012
1	0.113	0.010
0.5	0.080	0.005

Note: Fractional sizes between the sizes listed above are also available and acceptable for use.

Welded wire fabric will be designated as shown in the following example:

6 x 12 - W16 x W8; indicated six (6) inch longitudinal wire spacing and 12 inch transverse wire spacing with smooth Number 16 wire longitudinally and smooth Number 8 wire transversely.

(8) Epoxy Coating. The epoxy coating material and the material used for the repair of the coating shall comply with the TxDOT Departmental Materials Specification D-9-8130, "Epoxy Powder Coating For Reinforcing Steel". Copies of the Departmental Materials Specifications are available from the TxDOT, Division of Materials and Tests, 125 East 11th Street, Austin, Texas 78701-2483. An eight-ounce sample of epoxy powder and manufacturer's certifications will be required for each lot of epoxy powder used to coat materials for City projects.

440.3. BENDING.

The reinforcement shall be bent cold, true to the shapes shown on the Plans. Fabrication shall preferably be done in the shop. Field fabrication, if permitted, shall be done with equipment approved by the City. Misfabricated, damaged or broken bars shall be rejected and replaced at the Contractor's expense. Damaged or broken bars imbedded in a previous concrete placement may be repaired with the approval of the City.

Unless otherwise shown on the Plans, the inside diameter of bar bends, in terms of the nominal bar diameter (d), shall be as follows:

Bends of 90° and greater in stirrups, ties and other secondary bars that enclose another bar in the bend shall be:

#3, #4, #5 4d

#6, #7, #8 6d

All bends in main bars and in secondary bars not covered above shall be:

#3 thru #8 6d

#9, #10, #11 8d

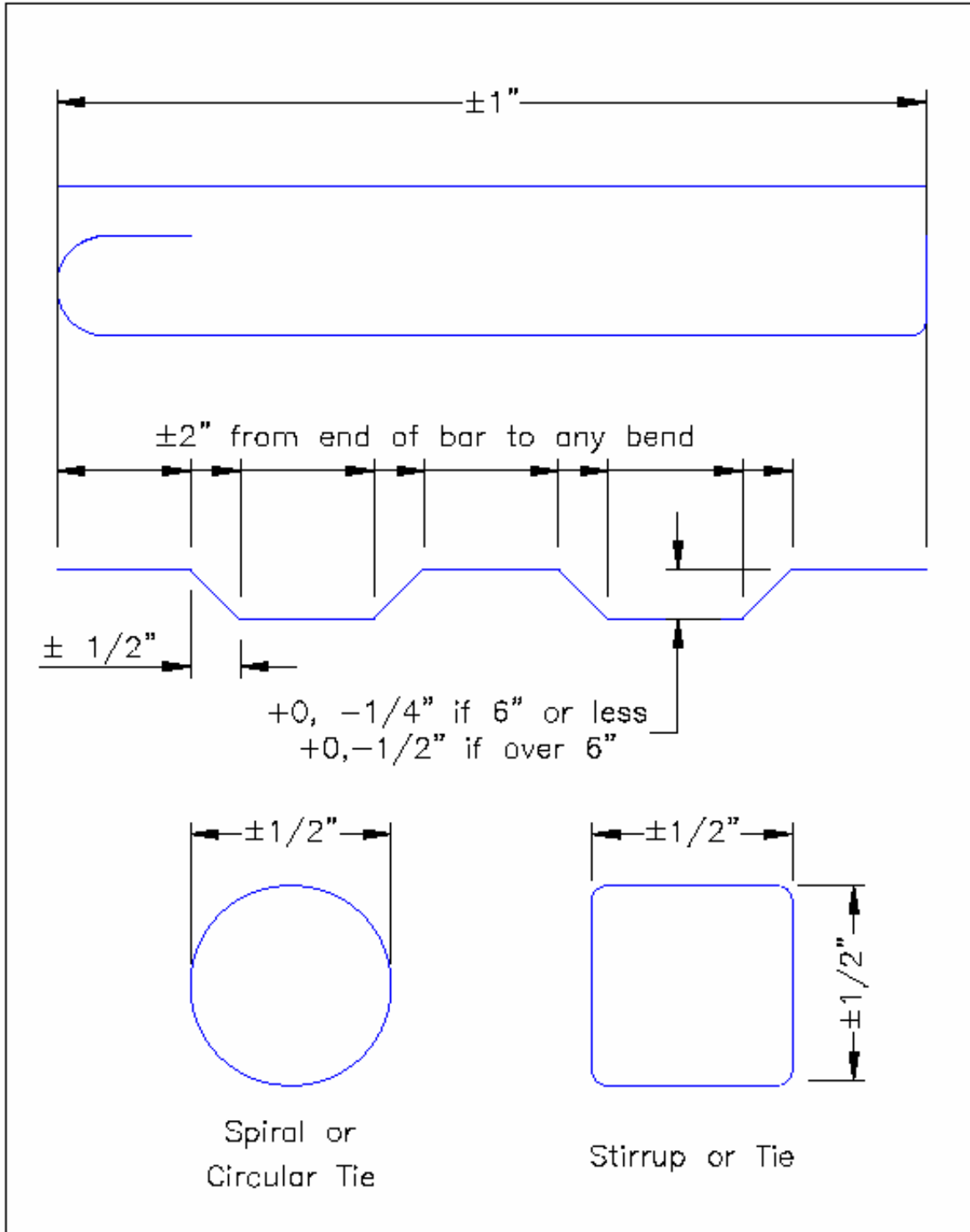
#14, #18 10d

Where bending of Grade 60 bars, sizes No. 14 or No. 18 is required, bend testing shall be performed on representative specimens as described for smaller bars in the applicable ASTM Specification. The required bend shall be 90° degrees around a pin having a diameter of ten (10) times the nominal diameter of the bar.

440.4. TOLERANCES

Fabricating tolerances for bars, from Plan dimensions, shall not be greater than shown in Figure 1.

FIGURE 1



440.5. STORING.

Steel reinforcement shall be stored above the surface of the ground upon platforms, skids, or other supports and shall be protected from damage and deterioration as approved by the City. When placed in the work, reinforcement shall be free from dirt, paint, grease, oil, or other foreign materials. Reinforcement shall be free from defects such as cracks and laminations. Rust, surface seams, surface irregularities or mill scales will not be cause for rejection, provided the minimum cross-sectional area of a hand wire brushed specimen meets the requirements for the size of steel specified.

440.6. SPLICES.

Splicing of bars, lap spliced or welded, shall be as shown on the Plans or specified herein. Additional splices will require written approval of the City.

Splices not provided for on the Plans will be permitted in slabs 15 inches or less in thickness, columns, walls and parapets, but will not be included for measurement, subject to the following:

Unless otherwise approved by the City, splices will not be permitted in bars 30 feet or less in Plan length. For bars exceeding 30 feet in Plan length, the distance center to center of splices shall not be less than 30 feet minus one splice length, with no more than one individual bar length less than ten (10) feet. The specified concrete cover and proper spacing shall be maintained at such splices and the lap spliced bars placed in contact and securely tied together.

TABLE 1
MINIMUM LAP REQUIREMENTS FOR BAR SIZES THROUGH NO. 11

SIZE	LAP LENGTH	
	UNCOATED	COATED
No. 3	1'-0"	1'-6"
No. 4	1'-6"	2'-3"
No. 5	1'-10"	2'-9"
No. 6	2'-3"	3'-4"
No. 7	3'-0"	4'-6"
No. 8	3'-9"	5'-7"
No. 9	4'-8"	7'-0"
No. 10	5'-7"	8'-4"
No. 11	6'-7"	9'-10"

Spiral steel shall be lapped a minimum of one turn.

Bar sizes No. 14 and No. 18 may not be lapped.

Welded splices shall conform to the requirements of the Plans and TxDOT Item 448, "Structural Field Welding". End preparation for butt welding reinforcing bars shall be done in the field. Delivered bars shall be of sufficient length to permit weld preparation.

Welded wire fabric shall be spliced using a lap length that will include the overlap of a minimum of two (2) cross wires plus two (2) inches on each sheet or roll. Splices using bars, which develop equivalent strength and lapped in accordance with Table 1 will be permitted.

For box culvert extensions with less than one (1) foot of fill, the existing longitudinal bars shall have a lap with the new bars as shown in Table 1. For extensions with more than one (1) foot of fill, a minimum of six-inch lap will be required.

440.6. MECHANICAL COUPLERS.

(1) General. When shown on the Plans, mechanical splices may be made in the reinforcing steel bars using one of the following types:

- Sleeve-Filler Type
- Sleeve-Threaded Type
- Sleeve-Swaged Type
- Sleeve-Wedge Type

A pre-qualified manufacturer shall produce all couplers furnished by the Contractor. Pre-qualification shall be in accordance with TxDOT Departmental Material Specification D-9-4510. Sleeve-wedge type couplers will not be permitted on coated reinforcing.

(2) Project Samples. For purposes of sampling couplers for use on an individual project, a lot of couplers shall be defined as 500 couplers, or fraction thereof, for each size and type. Prior to use on the project, three (3) test specimens shall be assembled using couplers selected at random from each lot received on the project. All test specimens shall be assembled from materials consigned to the project and shall be assembled in the presence of the City. A test specimen shall consist of a coupler connecting two (2) 21 inch, or longer, bars using the same splice materials, position, equipment and procedures to be used to make splices in the work. The assembled test specimens shall be submitted to the Division of Materials and Tests for testing. Each lot of couplers shall be identified with tags or markings identifying the lot from which the samples were taken.

(3) Testing. Project samples will be tested to 125% of specified yield strength and for total slip requirements. When a test representing a lot of couplers fails to meet the requirements, four (4) additional couplers from that lot will be tested. If all four (4) tests meet the requirements, the lot will be accepted for use in the work. If any of the four (4) tests fail to meet the requirements, that lot of couplers will be rejected and not used in the work.

(4) Construction Methods. All coupling devices shall be installed in accordance with the manufacturer's recommendations. Protection of threaded male or female connections shall be provided and the threaded connections shall be clean when making the connection. Damaged threads shall not be repaired.

(5) **Alternate Equivalent Strength.** Alternate equivalent strength arrangements to be accomplished by substituting larger bar sizes, or more bars, will be considered if approved by the City, in writing, prior to the fabrication of the systems.

440.7. PLACING.

Unless otherwise shown on the Plans, dimensions shown for reinforcement are to be the centers of the bars. Reinforcement shall be placed as near as possible in the position shown on the Plans. In the plane of the steel parallel to the nearest surface of concrete, bars shall not vary from Plan placement by more than one-twelfth (1/12) of the spacing between bars. In the Plane of the steel perpendicular to the nearest surface of concrete, bars shall not vary from Plan placement by more than 1/4-inch. Cover of concrete to the nearest surface of steel shall meet the above requirements but shall never be less than one (1) inch.

For bridge slabs, the clear cover tolerance for the top mat of reinforcement shall be -0, + 1/2 inch.

The reinforcement shall be accurately located in the forms, and firmly held in place, before and during concrete placement, by means of bar supports, adequate in strength and number in order to prevent displacement and to keep the steel at the proper distance from the forms. Bars shall be supported by standard bar supports with plastic tips, plastic bar supports approved by the City or precast mortar or concrete blocks when supports are in contact with removable or stay-in-place forms. Bright basic bar supports may be used to support reinforcing steel placed in slap overlays on concrete panels or on existing concrete slabs. Bar supports in contact with soil or subgrade shall be as approved by the City.

For bar supports with plastic tips, the plastic protection shall have a minimum thickness of 3/32 of an inch and extend upward on the wire to a point at least one-half (1/2) inch above the formwork.

All accessories such as tie wires, bar chairs, supports or clips used with epoxy coated reinforcement shall be of steel, fully coated with epoxy or plastic. Plastic supports approved by the City may also be used with epoxy coated reinforcement.

Mortar or concrete blocks shall be cast to uniform dimensions with adequate bearing area. A suitable tie wire shall be provided in each block for anchoring to the steel. The blocks shall be accurately cast to the thickness required in molds approved by the City. The surface placed adjacent to the form shall be a true Plane, free of surface imperfections. The blocks shall be cured by covering with wet burlap or mats for a period of 72 hours. Mortar for blocks shall contain approximately one (1) part Portland Cement to three (3) parts sand. Concrete for blocks shall contain nine (9) sacks of Portland Cement per cubic yard of concrete.

Individual bar supports shall be placed in rows at four (4) foot maximum spacing in each direction. Continuous type bar supports shall be placed at four (4) feet maximum spacing. Continuous bar supports will be required when permanent metal deck forms are used.

Individual bar supports shall be placed in rows at four-foot maximum spacing in each direction. Continuous type bar supports shall be placed at four (4) feet maximum spacing. Continuous bar supports will be required when permanent metal deck forms are used.

The exposure of the ends of longitudinals, stirrups and spacers used to position the reinforcement in concrete pipe and precast box culverts or sewers shall not be cause for rejection.

Reinforcing steel for bridge slabs, top slabs or direct traffic culverts and the top slabs of prestressed box beams shall be tied at all intersections except that where the spacing is less than one (1) foot in each direction, alternate intersections only need to be tied. For reinforcing steel cages for other structural members, the steel shall be tied at a sufficient number of intersections to provide a rigid cage of steel. Mats of wire fabric shall be fastened securely at the ends and edges.

Before concrete placement, all mortar, mud, dirt, etc., shall be cleaned from the reinforcement. Concrete shall not be placed until authorized by the City.

If the reinforcement is not adequately supported or tied to resist settlement, floating upward, overturning of truss bars, or movement in any direction during concrete placement, concrete placement will be halted until corrective measures are taken.

440.8. EPOXY COATING OF REINFORCING STEEL.

(1) General. When shown on the Plans, coating with epoxy of reinforcing bars, plain wire, deformed wire or welded wire fabric to be used as reinforcement for concrete shall conform to the requirements herein.

(2) Surface Preparation. The reinforcing steel shall be free of surface contaminants such as oil, grease or paint when received at the manufacturer's Plant and prior to cleaning and coating. The surface of steel to be coated shall be cleaned by abrasive blast cleaning. All traces of grit and dust from the blast cleaning shall be removed prior to coating. Other methods of cleaning may be submitted to the City for approval.

(3) Application of Coating. The applicator shall notify the City at least 30 days before the date of production. The coating shall be applied as recommended by the manufacturer of the coating material.

The coating shall be applied to the cleaned surface as soon as possible after cleaning and before oxidation of the surface discernible to the unaided eye occurs. The coating shall be a smooth uniform coat and shall have a thickness of from 7 to 12 mils, after curing. The thickness of the coating shall be measured using magnetic thickness testing gages in accordance with Test Method Tex-728-I.

The coating film shall be fully cured. Sufficient checks shall be made to assure that each coated production lot is supplied in a fully cured condition.

(4) Continuity of Coating. The applicator shall check the coating for continuity after curing. The coating shall be free from holes, voids, cracks, contamination and damaged areas discernible to the unaided eye.

For reinforcing bars a 67-1/2 volt D.C. in-line holiday detector, such as Tinker and Razor Model M-1 or approved equivalent, shall be used to check the coating for holidays. There shall be no more

than two (2) holidays (pinholes not visually discernible) in any linear foot of a coated reinforcing bar.

Holiday checks to determine acceptability of wire or welded wire fabric shall be made at the manufacturer's Plant with a 67-1/2 volt D.C. in-line holiday detector. For wire, there shall not be more than an average of two (2) holidays per linear foot of wire. For welded wire fabric, there shall not be more than an average of four (4) holidays per linear foot of wire in welded wire fabric when the wire spacings are four (4) inches or more, or six (6) holidays per linear foot of wire when the spacings are less than four (4) inches. Uncoated areas at cut ends shall not be counted, nor shall sharp edges (weld spurs) at intersections be counted as holidays. When measuring the number of holidays, at least one-half (1/2) inch of wire must be included on each side of the intersections being checked.

(5) Repair of Coating. Material for repair of the coating shall comply with the requirements in "Epoxy Coating" of this Item. Repairs shall be made in accordance with procedures recommended by the manufacturer of the epoxy coating powder. Areas to be patched shall receive at least the same coating thickness as required for the original coating.

All visible damage to the coating shall be repaired.

Sawed and sheared ends, cuts, breaks and/or other damage shall be repaired promptly before additional oxidation occurs. Areas to be repaired shall be clean and free from surface contaminants. Repairs shall be made in the shop or in the field as required.

The acceptable amount of patched area at the applicator shall not exceed one-quarter (1/4) inch total length in any linear foot.

(6) Sampling and Testing. Sampling and testing of coated reinforcement shall be in accordance with Test Method Tex-739-I.

(7) Identification and Documentation. Identification of all reinforcing shall be maintained throughout the coating and fabrication process and until delivery to the project site.

For all production of coated reinforcing steel to be used on City projects, the manufacturer shall furnish to the City two copies of a written certification that the coated reinforcing steel meets the requirements of this specification and two copies of the manufacturer's control tests.

(8) Handling. All systems for handling coated reinforcement shall have padded contact areas. Bundling bands shall be padded or suitable banding shall be used to prevent damage to the coating. Bundles of coated reinforcement shall be lifted with a strong back, spreader bar, multiple supports, or a platform bridge. The bundled reinforcement shall be transported with care and stored on protective cribbing. The coated reinforcement shall not be dropped or dragged.

(9) Construction Methods. Flame cutting will not be permitted on coated reinforcement. Saw or shear cutting will be permitted with permission of the City. Cut ends shall be coated as specified in "Repair of Coating" of this Item.

Welding or mechanical coupling of coated reinforcing steel will not be permitted except where specifically shown on the Plans. The epoxy coating shall be completely removed a minimum of six (6) inches beyond the weld limits prior to welding and two (2) inches beyond the limits of the coupler prior to assembly. After welding or coupling, the steel shall be cleaned of all oil, grease, moisture, dirt, welding contamination (slag and/or acid residue) and rust to a near white finish. The existing epoxy shall be checked for damage. Any damaged or loose epoxy shall be removed back to sound epoxy coating.

After proper cleaning, the splice area shall be coated with epoxy repair material to a thickness of 7 to 12 mils. A second application of repair material shall be applied to the bar and coupler interfaces to insure complete sealing of the joint.

440.9. MEASUREMENT AND PAYMENT.

Except as specified below, the work performed, materials furnished, and all labor, tools, equipment and incidentals necessary to complete the work under this Item will not be measured or paid for directly, but will be considered subsidiary to the various bid items of the contract.

The quantities of reinforcing steel shown on the Plans are estimates and are for the Contractor's information.

Compensation for adjustment of reinforcing steel quantities will be as follows:

(1) When the reinforcing steel quantity for a complete structure element has been erroneously included in or omitted from the quantities shown on the Plans, the quantity for that element will be added or deducted for payment. A complete structure element will be the smallest portion of a total structure for which a corresponding quantity of concrete is included on the Plans. Additional payment or reduction in payment for quantities revised in this manner will be made accordingly, in accordance with TxDOT Item 4.3.

(2) When the Plan quantity for reinforcing steel for a complete structure element is in error by five (5) percent or more, a recalculation will be made and payment will be increased or reduced accordingly in accordance with contract documents..

(3) When quantities for reinforcing steel are revised by a change in design, the change in quantities will be calculated. Additional payment or reduction in payment for quantities revised in this manner will be made accordingly, in accordance with contract documents.

The party to the contract requesting the adjustment shall present to the other one (1) copy of the description and location, together with calculations of the quantity for the structure element involved. When this quantity is certified correct by the City, it will become the basis for additional or reduced payment.

CITY OF SAN ANGELO**ITEM 531****SIDEWALKS****531.1. DESCRIPTION.**

This Item shall govern for Portland Cement Concrete sidewalks constructed on approved subgrade, foundation material or finished surface in accordance with the lines and grades established by the City and in conformance with details shown on the Plans.

531.2. MATERIALS.

All materials shall conform to the pertinent requirements of the following items:

- Item 360, "Concrete Pavement"
- Item 420, "Concrete Structures"
- TxDOT Item 437, "Concrete Admixtures"
- Item 421, "Portland Cement Concrete"
- Item 440, "Reinforcing Steel"
- TxDOT Item 526, "Membrane Curing"

Concrete used in conventionally formed and slipformed construction shall be Class "A" concrete or concrete as specified for concrete pavement. Unless otherwise shown on the Plans, concrete for extruded construction shall be Class "A", except that the coarse aggregates shall meet the requirements of grade 8. Other grades of coarse aggregate may be substituted, if approved by the City.

531.3. CONSTRUCTION METHODS.

(1) Conventionally Formed Concrete. Prior to sidewalk construction, the subgrade, foundation or pavement surface shall be shaped to the line, grade and cross section shown on the Plans, and if considered necessary by the City, hand tamped and sprinkled. When directed by the City, the subgrade or foundation material shall be sprinkled lightly immediately before concrete is deposited thereon.

Forms, where needed, shall be of wood or metal, of a section satisfactory to the City, straight, free of warp and of the depth required. They shall be securely staked to line and grade and maintained in a true position during the placing of concrete.

The reinforcing steel shall be placed in position as shown on the Plans. Care shall be exercised to keep all steel in its proper location during concrete placement.

Sidewalks shall be constructed in sections of the lengths shown on the Plans. Unless otherwise shown on the Plans or approved by the City, no section shall be less than eight (8) feet or more than 40 feet in length. Sections shall be separated by premolded or board joint of the thickness shown on

the Plans and placed vertically and at right angles to the longitudinal axis of the sidewalk. Where the sidewalk abuts a curb or retaining wall, approved expansion material shall be placed along their entire length. Similar expansion material shall be placed around all obstructions protruding through sidewalks. Sidewalks shall be marked at spacing equal to sidewalk width by the use of approved jointing tools. Each day's production will be terminated at an expansion joint.

(2) Extruded or Slipformed Concrete. Concrete shall be placed with equipment approved by the City. When placement is directly on subgrade or foundation materials, the foundation shall be hand tamped and sprinkled if considered necessary by the City. If the concrete is placed directly on the surface material or pavement, such surface shall be thoroughly cleaned. Concrete shall be fed into the machine in such a manner and at such consistency that the finished work will present a well-compacted mass with a surface free from voids and honeycomb, and true to the required shape, line and grade.

Any additional surface finishing specified and/or required shall be performed immediately after extrusion or slipforming. Joints shall be constructed at such locations as directed by the City and conforming to the details shown on the Plans.

(3) General. The completed work shall be cured for a period of not less than 72 hours by one of the methods specified in Item 420, "Concrete Structures".

Regardless of the method of construction, hand finishing will be permitted. All exposed surfaces shall be brushed to a smooth and uniform surface.

Wheelchair ramps shall be in conformance with the details shown on the Plans.

531.4. MEASUREMENT.

Sidewalks, including any required wheelchair ramps, will be measured by the linear foot or by the square foot of surface area, complete in place.

531.5. PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Concrete Sidewalks" of the width shown on the Plans. This price shall be full compensation for furnishing all materials required; surface preparation of base; and for all manipulations, labor, tools, equipment, and incidentals necessary to complete the work.

CITY OF SAN ANGELO**ITEM 900****REFLECTORIZED PAVEMENT MARKINGS****900.1 DESCRIPTION.**

This Item shall govern for furnishing and placing reflectORIZED pavement markings of the types, colors, shapes, sizes, widths and thickness shown on the Plans.

900.2 MATERIALS.

(1) **Type I Marking Materials.** Type I markings are thermoplastic type materials that require heating to elevated temperatures for application. Type I marking materials shall conform to TxDOT Departmental Materials Specification D-9-8220. Each container of Type I marking material shall be clearly marked to indicate the color, weight, type of material, manufacturer's name and the lot/batch number.

(2) **Type II Marking Materials.** Type II markings are paint type materials that are applied at ambient or slightly elevated temperatures. Type II marking materials shall conform to TxDOT Departmental Materials Specification D-9-8220, YPT-10 and/or WPT-10 and D-9-8290

(3) **Source of Supply.** All Type I marking materials shall be purchased on the open market.

All glass traffic beads and Type II paint shall be purchased on the open market.

900.3 EQUIPMENT REQUIREMENTS.

Equipment used to place pavement markings shall:

(1) Be maintained in satisfactory operating condition.

(2) Be considered in satisfactory operating condition if it can repeatedly produce a stripe acceptable to the City.

(3) Meet or exceed the material handling at elevated temperatures requirements of the National Fire Underwriters and the Texas Railroad Commission.

(4) Have production capabilities considered satisfactory by the City when used to place markings other than solid or broken lines.

(5) Be capable of producing a center-line and a no-passing barrier line configuration consisting of one (1) broken line with two (2) solid lines at the same time to the alignment and spacing shown on the Plans.

(6) Be capable of placing lines with clean edges and of uniform cross-section. All lines shall have a tolerance of $\pm 1/8$ inch per four (4) inch width.

(7) Have an automatic cut-off device with manual operating capabilities to provide clean, reasonably square marking ends to the satisfaction of the City, and to provide a method of applying broken line in an approximate stripe-to-gap ration of 10 to 30. The length of the stripe shall not be less than 10 feet or more than 10.5 feet. The total length of any stripe-gap cycle shall not be less than 39.5 feet or more than 40.5 feet.

(8) Provide continuous mixing and agitation of the pavement marking material. The use of pans, aprons or similar appliances which the die overruns will not be permitted for longitudinal striping applications.

(9) Apply beads by an automatic bead dispenser attached to the pavement marking equipment in such a manner that the beads are dispensed uniformly and almost instantly upon the marking as the marking is being applied to the road surface. The bead dispenser shall have an automatic cut-off control, synchronized with the cut-off of the pavement marking equipment.

When Type I markings are to be placed, the Contractor shall have a hand-held thermometer on the project. The thermometer shall be capable of measuring the temperature of the pavement marking material to be placed.

900.4 CONSTRUCTION METHODS.

(1) **General.** When required by the City, the Contractor and the City shall review the sequence of work to be followed and the estimated progress schedule.

Markings may be placed on roadways either free of traffic or open to traffic. On roadways already open to traffic, the markings shall be placed under traffic conditions that exist with a minimum of interference to the operation of the facility. Traffic control shall be as shown on the Plans or as approved by the City in writing. All markings placed under open-traffic conditions shall be protected from traffic damage and disfigurement. On roadways open to traffic, with three (3) lanes of travel in one direction, all markings shall be placed from the outside lanes only unless otherwise approved in writing by the City.

Guides to mark the lateral location of pavement markings shall be established as shown on the Plans or as directed by the City. The Contractor shall establish the pavement marking guides and the City will verify the location of the guides.

Markings shall be placed in proper alignment with the guides. The deviation rate in alignment shall not exceed one (1) inch per 200 feet of roadway. The maximum deviation shall not exceed two (2) inches nor shall any deviation be abrupt.

Markings shall essentially have a uniform cross-section. The density and quality of markings shall be uniform throughout their thickness. The applied markings shall have no more than five (5) percent, by area, of holes or voids and shall be free of blisters.

Markings, in place on the roadway, shall be reflectORIZED both internally and externally. Glass beads shall be applied to the materials at a uniform rate sufficient to achieve uniform and distinctive retroreflective characteristics when observed in accordance with TxDOT Test Method Tex-828-B or as otherwise shown on the contract Plans.

The Contractor's personnel shall be sufficiently skilled in the work of installing pavement markings.

Markings placed that are not in alignment or sequence, as shown on the Plans or as stated in this specification, shall be removed by the Contractor at the Contractor's expense. Removal shall be in accordance with Item 901, "Eliminating Existing Pavement Markings and Markers", except for measurement and payment. Guides placed on the roadway for alignment purposes shall not establish a permanent marking on the roadway.

Unless otherwise shown on the Plans, pavement markings may be applied by any method that will yield markings meeting the requirements of this specification.

(2) Surface Preparation. New portland cement concrete surfaces shall be cleaned in accordance with TxDOT Item 678, "Pavement Surface Preparation for Markings" to remove curing membrane, dirt, grease, loose and/or flaking existing construction markings and other forms of contamination.

Older portland cement concrete surfaces and asphaltic surfaces that exhibit loose and/or flaking existing markings shall be cleaned in accordance with TxDOT Item 678, "Pavement Surface Preparation for Markings", to remove all loose and flaking markings.

Pavement to which material is being applied shall be completely dry. Pavements shall be considered dry if, on a sunny day after observation for 15 minutes, no condensation occurs on the underside of a one (1) foot square piece of plastic that has been placed on the pavement and weighted on the edges.

(3) Application of Type I Markings. New portland cement concrete surfaces shall be further prepared for Type I markings, after cleaning, by placing a Type II marking as a sealer in accordance with this Item. When placing Type I markings in new locations on asphaltic surfaces three (3) years old or older or any portland cement concrete surfaces, a Type II marking shall be used as a sealer. Unless otherwise shown on the Plans, existing Portland Cement concrete and asphaltic surfaces to be striped will not require Type II markings as a sealer; existing markings may be used as a sealer in lieu of Type II markings. Type II markings shall be placed a minimum of two (2) and a maximum of thirty (30) calendar days in advance of placing Type I markings. Type II markings, which become dirty due to inclement weather or road conditions shall be cleaned by washing, brushing, compressed air or other means approved by the City, prior to application of Type I markings. If washing is used, the surface of Type II markings shall become thoroughly dry before placing Type I markings. Color, location and configuration of Type II markings shall be the same as that of Type I markings.

Type I pavement marking material shall be applied within temperature limits recommended by the material manufacturer. Application of Type I pavement markings shall be done only on

clean, dry pavement having a surface temperature above 50° F. Pavement temperature shall be measured in accordance with TxDOT Test Method Tex-829-B.

When Type I pavement marking application is by spray, and operation ceases for five (5) minutes or more, the spray head shall be flushed by spraying pavement marking material into a pan or similar container until the pavement marking material being sprayed is at the proper temperature for application.

Unless otherwise directed by the City in writing, Type I pavement marking materials shall not be placed on roadways between September 30 and March 1, subject to temperature and moisture limitations specified herein.

Unless otherwise shown on the Plans, Type I marking minimum thickness shall be 0.060 inch (60 mil) for edgeline markings and 0.090 inch (90 mil) for stop-bars, legends, symbols, gore and center-line/no-passing barrier-line markings, when measured in accordance with TxDOT Test Method Tex-854-B. The maximum thickness of all Type I markings shall be 0.180 inch (180 mil).

The thickness of Type I markings at the time of placement will be measured above the Plane formed by the pavement surface. The City will supply a device to measure the thickness of the applied markings. The markings shall be of uniform thickness throughout their lengths and widths.

(4) Application of Type II Markings. The application of Type II marking materials shall be done only on surfaces with a minimum surface temperature of 50°F.

The application rate for Type II marking material shall be: between 15 and 20 gallons per mile of solid four (4) inch line and between 30 and 40 gallons per mile for solid eight (8) inch line except that, for new surface treatment projects, the application rate shall be between 25 and 30 gallons per mile of solid four (4) inch line and between 40 and 50 gallons per mile for solid eight (8) inch line.

Pavement markings for new surface treatment projects shall be applied in two (2) applications, each approximately one-half the application rate. The first application shall not contain glass beads. The interval between the first and second applications shall be a minimum of one (1) hour.

When, in the case of impending inclement weather, and the City directs the Contractor to apply water-based traffic paint, the markings are damaged by subsequent rain, sleet, hail, etc., the Contractor will be paid for the initial placement and the replacement markings. However, if the Contractor places the markings at his option, the Contractor is responsible for all costs associated with the replacement markings.

900.5 PERFORMANCE PERIOD FOR TYPE I MARKINGS.

Type I pavement markings shall meet all the requirements of this specification for a minimum of 15 calendar days after installation. Pavement markings that fail to meet all the requirements of this specification shall be removed and replaced by the Contractor at the Contractor's expense.

The Contractor shall replace all pavement markings failing the requirements of this specification within 30 calendar days following notification by the City of such failing. All replacement markings shall also meet all requirements of this specification for a minimum of 15 calendar days after installation.

900.6 MEASUREMENT.

This Item will be measured by the linear foot, by each of the various words, symbols or shapes, or by any other unit as shown on the Plans.

Where double stripes are placed, each stripe will be measured separately.

This is a Plans quantity measurement Item and the quantity to be paid for will be that quantity shown in the proposal and on the "Estimate and Quantity" sheet of the contract Plans, except as may be modified by contract documents. If no adjustment of quantities is required, additional measurements or calculations will not be required.

Type II pavement markings requiring two (2) applications on new surface treatments will be measured as one (1) marking.

Type II pavement marking materials, when used as a sealer for Type I markings, will be measured as Type II markings.

900.7 PAYMENT.

The work performed and materials furnished in accordance with this Item and measured as provided under "Measurement" will be paid for at the unit price bid for "Reflectorized Pavement Markings" of the various types, colors, shapes, sizes, widths and thickness (Type I markings only) specified. This price shall be full compensation for furnishing all materials; for application of pavement markings; and for all other labor, tools, equipment and incidentals necessary to complete the work, except as shown below.

Surface preparation, when shown on the Plans, will be subsidiary to this Bid Item.

Final work zone pavement markings (paint and beads) which will be used as a sealer for Type I pavement markings will be paid for under this Item.

When replacement Type II markings are required due to damage to the original markings from rain, sleet, hail, etc., and the original markings were placed at the direction of the City, the Plan quantity requirements under "Measurement" do not apply to the original and replacement markings. The Contractor will be paid for the actual quantity of original and replacement markings at the unit price bid for that item.

SECTIONAL PRECAST CONCRETE BUILDING

1.0 SCOPE

This specification covers the construction and placing of a sectional precast concrete building.

2.0 SPECIFICATIONS

ASTM C33	Concrete Aggregates
ASTM C39	Method of Test for Compressive Strength of Cylindrical Concrete Specimens
ASTM C94	Standard Specification for Ready-Mixed Concrete
ASTM C143	Method of Test for Slump of Concrete
ASTM C150	Standard Specification for Portland Cement
ASTM C172	Standard Practice for Sampling Freshly Mixed Concrete
ASTM A185	Standard Specification for Steel Welded Wire Reinforcement, Plain, or Concrete
ASTM C192	Method of Making and Curing Test Specimens in the Laboratory
ASTM C231	Standard Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method
ASTM C309	Standard Specifications for Liquid Membrane-Forming Compounds for Curing Concrete
ASTM C494	Standard Specification for Chemical Admixtures for Concrete
ASTM A615	Standard Specification for Deformed and Plain Carbon-Steel Bar for Concrete Reinforcement
ASTM C618	Standard Specification for Coal Fly Ash and Raw or Calcine Natural Pozzolan for Use in Concrete
ASTM C979	Standard Specification for Pigments for Integrally Colored Concrete
ASTM D1557	Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort
ACI 211.1	Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete
ACI 306	Cold Weather Concreting
ACI 318	Building Code Requirements Structural Concrete and Commentary (includes Errata)
PCI MNL 116	Quality Control for Plants and Production of Precast Prestressed Concrete Products

3.0 MANUFACTURER CRITERIA

The manufacturer supplying the requested precast concrete flush facility must meet the following:

- A. Manufacturer must be ISO 9001 certified at the time of bid.
- B. Manufacturing plant must be PCI certified at the time of bid.
- C. Manufacturer must not have defaulted on any contract within the last five (5) years.
- D. Manufacturer must provide stamped, engineered drawings prior to acceptance.
- E. Manufacturer must be pre-approved prior to bidding.
- F. Manufacturer must show four (4) examples of precast concrete flush facilities produced, installed and in use as an example of their ability to perform this contract.

- G. Manufacturer shall provide a warranty against defects in material and workmanship for a period of twenty (20) years on all concrete components. The warranty is valid only when concrete is used within the specified loadings. Furthermore, said warranty includes only the related material necessary for the construction and fabrication of said concrete components.
- H. UL 752 Bullet Resistance on 4" thick concrete samples.

Manufacturer meeting these criteria is:

CXT Incorporated
Spokane Industrial Park
3808 North Sullivan Road, Building 7
Spokane, WA 99216
Phone 800-696-5766

Other manufacturers meeting the criteria are able to submit their qualifications to the Engineer for consideration.

4.0 DESIGN CRITERIA

The flush building has been designed to individually meet the following criteria. Calculations and engineer's stamped drawings are available, for standard buildings, upon request by the customer and are for their sole and specific use only. The design criteria are to ensure that the flush building not only will withstand the forces of nature listed below, but to provide protection from vandalism and other unforeseen hazards. Building's structural and foundation design will be relevant to the region and properties associated with its final placement. Design will also meet all applicable accessibility and building code requirements. Buildings will also meet various structural loads such as below, but not limited to/or restricted by them.

- A. Roof Snow Load
 - 1. The building shall be designed to withstand a 50 PSF snow load.
- B. Floor Load
 - 1. The building shall designed to withstand 400 PSF floor load.
- C. Wind Load
 - 1. The building will withstand the effects of 150 miles per hour (3-second gust) wind exposure
- D. Earthquake
 - 1. The building will withstand the effects of a seismic group 1 design category E earthquake.
- E. Additional Design Standards
 - 1. The building is designed to meet the accessibility requirements put forth by federal, state, and local statutes.
 - 2. The building is an all concrete design with a minimum 3/12 roof pitch. The flush building shall have a minimum 4" wall, 4½" roof, and 5" floor thickness.
 - 3. All wall to floor interior surface seams shall have a minimum 1" radius coving made of high strength grout.
 - + Recycled Material
 - + LED Lighting

5.0 MATERIALS

A. Concrete – General

1. The concrete mix design is designed to ACI 211.1 to produce concrete of good workability.
2. Concrete will contain a minimum of 675 pounds of cementitious material per yard. Cement is a low alkali type I/II or III conforming to ASTM C-150.
3. Coarse aggregates used in the concrete mix design will conform to ASTM C33 with the designated size of coarse aggregate #67.
4. Maximum water/cement ratio will not exceed .45.
5. Air-entraining admixtures will conform to ASTM C260. Water reducing admixtures will conform to ASTM C494, Type A.
6. If Self Compacting Concrete (SCC) is used, it must conform to ASTM C1611.

B. Colored Concrete

1. Color additives will conform to ASTM C979. A 12" x 12" x 1" color sample is available for customer approval.
2. The following will contain colored concrete:
 - a. Toilet building roof panels.
 - b. Building walls.
 - c. Screen panels.
3. The same brand and type of color additive are used throughout the manufacturing process.
4. All ingredients are weighed and the mixing operation are adequate to ensure uniform dispersion of the color.

C. Concrete Reinforcement

1. All reinforcing steel will conform to ASTM A615. All welded wire fabric will conform to ASTM A185.
2. All reinforcement is new, free of dirt, oil, paint, grease, loose mill scale and loose or thick rust when placed.
3. Details not shown on drawings or specified are to ACI318.
4. Steel reinforcement is centered in the cross-sectional area of the walls and will have at least 1¼" of cover on the under surface of the floor.
5. The maximum allowable variation for center-center spacing of reinforcing steel is ½".
6. Full lengths of reinforcing steel are used when possible. When splices are necessary on long runs, splices are alternated from opposite sides of the components for adjacent steel bars.
 - a. Lap bars under #4 a minimum of 12" bar diameters.
 - b. Lap bars larger than #4 a minimum of 24" bar diameters.
7. Reinforcing bars are bent cold. No bars partially embedded in concrete are field bent unless approved by the customer.

D. Caulking, Grout, Adhesive and Sealer

1. Caulking service temperatures from -40°F to +194°F.
2. Interior and exterior joints are caulked with a paintable polyurethane sealant.

3. Grout is a non-shrink type and are painted to match the color of surrounding concrete as nearly as possible.
 4. Cement base coating is formulated with a very fine aggregate system and is a built-in bonding agent.
- E. Paint
1. All paints and materials will conform to all federal specifications or be similar "top-of-the-line-components."
 2. Type of paints for toilets.
 - a. Inside concrete surfaces.
 - i. Interior floors will be a chemical resistant urethane. The color will be gray.
 - ii. Interior walls and ceilings will be a modified acrylic, water repellent penetrating stain. The color will be white followed by a clear acrylic anti-graffiti sealer.
 - b. Metal surfaces both inside and out.
 - i. DTM ALKYD.
 - c. Exterior concrete surfaces.
 - i. Exterior slab will be clear sealer.
 - ii. Exterior walls and roof will be a water repellent penetrating stain in the same color as the walls or roof followed by a clear acrylic anti-graffiti sealer.
- F. Grab Bars
1. Grab bars will be 18-gauge, type 304 stainless steel with 1½" clearance. Grab bars will each be able to withstand 300-pound top loading.
- G. Toilet Paper Dispenser
1. Dispenser will be constructed of ¼" thick, type 304 stainless steel. Dispenser will be capable of holding three (3) standard rolls of toilet paper. Toilet paper holder fastening system will be able to withstand 300-pound top loading.
- H. Steel Doors
1. Doors will be flush panel type 1¾" thick, minimum 16-gauge galvanized steel, top painted with DTM ALKYD.
 2. Door frames will be knockdown or welded type, single rabbet, minimum 16-gauge prime coated steel top painted with DTM ALKYD, width to suit wall thickness. Three (3) rubber door silencers will be provided on latch side of frame.
- I. Door Hinges
1. Door hinges will be three (3) per door with dull chrome plating 4½" x 4½", adjustable tension, and automatic closing for each door.
- J. Lockset
1. Lockset will meet ANSI A156.2 Series 4000, Grade 1 cylindrical lockset for exterior door.
 2. Lever handle both inside and out.
 3. U.S. 26D finish.
- K. Dead Bolt
1. Certified ANSI/BHMA A156.5-2001 Grade 1.
 2. Heavy duty tamper resistant.

3. 2³/₄" backset.
 4. U.S. 26D finish.
- L. Doorstop
1. Doorstop will be a dome style stop meeting ANSI 156.16.
- M. Double Coat Hook
1. Coat hook will be 304 stainless steel 16-gauge (1.5mm), formed construction with a satin finish and have ³/₁₆" x ⁷/₈" nail in anchor. Upper hook will extend at least 2¹/₂" from the wall. Lower hook will extend at least 1¹/₄" from the wall.
- N. Door Sweep
1. Door sweep will be provided at the bottom of door and will be an adjustable brush type.
- O. Wall Vent
1. Wall vent will be crank operated allowing the unit to be opened or closed. Crank will be removable. Vent cover will be 14-gauge 304 stainless steel and anchored into the concrete wall with high strength anti-rust tap con fasteners. Vent to come with insect screen. Cover to be recessed a minimum ³/₄" on exterior walls with a 45-degree bevel. Interior to be flush mounted. Wall vent will not protrude from the wall.
- P. Signs
1. Signs to have raised pictograms, letters, and braille to meet ADA.
- Q. Windows
1. Window frames will be constructed from steel.
 2. Window glazing will be ³/₁₆" thick translucent pebble finished mar-resistant Lexan.
 3. Windows to have ³/₄" recess with 45-degree bevel.
 4. Window frames to have vandal resistant fasteners.
- R. Mirrors
1. Mirror to be 18" x 36" frameless 430 18-gauge stainless steel with #8 bright polish.
- S. Stall Doors
1. Stall doors to be solid HDPE in matching white color.
 2. Where applicable stalls to be made of concrete and match wall finish.
- T. Shower Section Benches
1. Shower benches to be heavy duty, type 304 satin finish stainless steel with phenolic slats.
- U. Concession Section Window
1. Concession window to be heavy duty vandal resistant locking aluminum roll up window with serving counter and serving windows.
- V. Plumbing Flush Section/Room
1. All fixtures to meet ANSI A112.19.2.
 2. Plumbing will be concealed in the service area.
 3. *Flush valve* – Concealed closet flush-o-meter constructed of rough brass. Furnish valve with integral vacuum breaker and wall mounted push button. Valve will be of a water saver type. Water closet with a flow of 1.6 gallons per flush. Urinal with a flow of .5 gallons per flush.
 4. *Hammer arrester* – Installed on water line.

5. *Hose bib* – Available in the chase area.
 6. *Lavatory* – Vitreous china with back splashguard, front overflow opening, equipped with brass trap and drainpipe without stopper. Sink will be 20" wide x 18" front to back x 5¾" deep with ADA trap cover. Optional stainless steel fixtures available.
 7. Main shut-off valve and drain.
 8. *Toilet* – Constructed of vitreous china, wall hung, with siphon jet action. Toilet will have a back spud for a concealed flush valve connection and will be mounted with the top of the seat 18" above the finished floor. Seat will be heavy duty solid plastic with an open front. Optional stainless steel fixtures available.
 9. *Urinal* – Constructed of vitreous china, wall hung, with a siphon jet action. Urinal will have a back spud for concealed flush valve connection and will be mounted at a proper height per code. Optional stainless steel fixtures available.
 10. Trap primer distribution unit.
 11. *Waste and vent material* – ABS or PVC plastic and will be plumbed to meet Uniform Building Codes.
 12. *Water material* – Copper tubing Type L, hard drawn. A gate valve will be provided at the inlet end of the water line. All water lines will be of a size to provide proper flushing action based on a nominal water pressure of 40 psi.
 13. *Water valve* – Self-closing water set with indexed push button.
 14. Optional water heater available.
- W. Plumbing Shower Section/Room
1. *Shower control unit* – 14-gauge, type 304 stainless steel recessed shower panel with 2.5 gpm flow rate, pressure balancing valve, recessed soap dish and integral stainless steel shower head.
 2. *ADA shower control unit* – 14-gauge, type 304 stainless steel recessed shower panel with 2.5 gpm flow rate, pressure balancing valve, recessed soap dish, high low diverter valve, and high low integral stainless steel shower heads.
 3. High efficiency commercial water heater provided.
 4. Optional coin operated controls available.
- X. Plumbing Concession Section/Room
1. Three (3) compartment, 304 18-gauge tub and top stainless steel sink with 9" back splash and 16-gauge galvanized steel legs.
 2. Faucet to be double jointed, chrome plated swing spout.
 3. Optional water heater available.
 4. Optional mop sink available.
 5. Optional lavatory available.
- Y. Electrical
1. All components are UL listed.
 2. *Breaker panel* – Sized to meet load requirements and mounted to meet electrical code.

3. *Interior lighting* – Vandal resistant fixtures with built-in occupancy sensor, energy efficient LED lights, and lifetime warranty.
4. *Exterior lighting* – Vandal resistant fixtures with built-in photoelectric switch, energy efficient LED lights.
5. *Exhaust fans* – All wet location motion activated with speed control in chase area to control CFM.
6. *Wiring* – Conduit, surface mounted in the service area and concealed in the user compartments. All wire will be copper.
7. GFI outlets provided per code requirements.
8. Optional warm air, ADA compliant, vandal resistant hand dryers available.

6.0 MANUFACTURE

A. Finishing Concrete

1. All exterior building walls and exterior screen walls will be any one of the available textures.
2. All exterior surfaces of the roof panels will be cast to simulate any one of the available textures. The underside of the overhang will have a smooth finish.

B. Cracks and Patching

1. Cracks in concrete components which are judged to affect the structural integrity of the building will be rejected.
2. Small holes, depressions, and air voids will be patched with a suitable material. The patch will match the finish and texture of the surrounding surface.
3. Patching will not be allowed on defective areas if the structural integrity of the building is affected.

7.0 FINISHING AND FABRICATION

A. Structural Joints

1. Wall components will be joined together with two (2) welded plate pairs at each joint. Each weld plate will be 6" long and located one (1) pair in the top quarter and one (1) pair in the bottom quarter of the seam. Weld plates will be anchored into the concrete panel and welded together with a continuous weld.
2. The inside seams will be a paintable caulk. The outside seams will use a caulk in a coordinating building color or clear.
3. Walls and roof will be joined with weld plates, 3" x 6" at each building corner.
4. The joint between the floor slab and walls will be joined with a grout mixture on the inside, a matching colored caulk on the outside and two (2) weld plates 6" long per wall.

B. Painting/Staining

1. An appropriate curing time will be allowed before paint is applied to concrete.
2. Schedule of finishes.
 - a. Inside concrete surfaces.
 - i. Inside floors will be one (1) coat of 1-part water based chemical resistant urethane.
 - ii. Interior walls and ceilings will be two (2) coats of a modified acrylic, water repellent penetrating stain, followed by one (1) coat of clear sealer.

- b. Metal surfaces both inside and out.
 - i. Two (2) coats of DTM ALKYD.
- c. Exterior concrete surfaces.
 - i. Exterior walls will be two (2) coats of water repellent penetrating stain in the same color as the walls or roof followed by one (1) coat of clear acrylic anti-graffiti sealer.

8.0 TESTING

The following tests will be performed on concrete used in the manufacture of toilets. All testing will be performed in a PCI certified laboratory. Testing will only be performed by qualified individuals who have been certified ACI Technician Grade 1. Sampling will be in accordance with ASTM C172.

- A. The air content of the concrete will be checked per ASTM C231 on the first batch of concrete. The air content will be in the range of 5.0% +/- 2.0%.
- B. The compressive strength of the cylinders will be tested to ASTM C39. We will make one (1) cylinder for release, one (1) for seven (7) days and one (1) for 28 days. The release must be a minimum strength of 2500 psi, the 7-day must be a minimum of 4500 psi and the 28-day must be a minimum of 5000 psi.
- C. A copy of all test reports will be available to the customer as soon as 28-day test results are available.

9.0 INSTALLATION

- A. Scope of Work
 - 1. Work specified under this section relates to the placement of the unit on a prepared building pad.
- B. Location
 - 1. It is the responsibility of the general contractor to:
 - a. Provide exact location by stakes or other approved method.
 - b. Provide clear and level site free of overhead and/or underground obstructions. *See Installation Questionnaire for details.*
 - c. Provide access to the site for truck delivery and sufficient area for the crane to install and the equipment to perform the contract requirements.
 - d. Water, electrical, and sewage site connections to be placed per drawings. Must be placed to easily connect to the building.
- C. Compacting
 - 1. The bottom of the area must be compacted after it has been dug out. After the base has been placed, it must be compacted as well. The bearing of the soil and base should be a minimum of 1,500 pounds per square foot.
- D. Base
 - 1. After compacting the bottom of the area, a minimum of 6" thick and consist of ¾" minus crushed rock (i.e. road base material) compacted to 95% of optimal density in accordance with ASTM D1557. Finished surface of sub-base shall be flat and level, with a maximum deviation of -1/2", +0" from a true horizontal plane.
 - 2. The base should be placed for support, leveling and drainage purposes and also to limit frost action. The base must be confined so as to prevent washout, erosion, or any other undermining.

E. Access to Site

1. Delivery to site made on normal highway trucks and trailers. If at the time of delivery conditions of access are hazardous or unsuitable for truck and equipment due to weather, physical constraints, roadway width or grade, CXT may require an alternate site with better access provided to ensure a safe and quality installation. In any such case, additional costs for cranes, trucking, etc. will be charged to the account of the customer. *See Installation Questionnaire for details.*

10.0 WARRANTY—PRECAST DIVISION

The supplier shall provide a warranty against defects in material or workmanship for a period of twenty (20) years on all concrete components. The warranty is valid only when concrete is used within the specified loadings. Furthermore, said warranty includes only the related material necessary for the construction and fabrication of said concrete components. All other non-concrete components will carry a one (1) year warranty. Supplier warrants that all goods sold pursuant hereto will, when delivered, conform to specifications set forth above. Goods shall be deemed accepted and meeting specifications unless notice identifying the nature of any non-conformity is provided in writing within the specified warranty. The Supplier, at its option, will repair or replace the goods or issue credit for the customer provided CXT is first given the opportunity to inspect such goods.

This warranty shall not apply to:

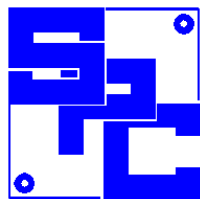
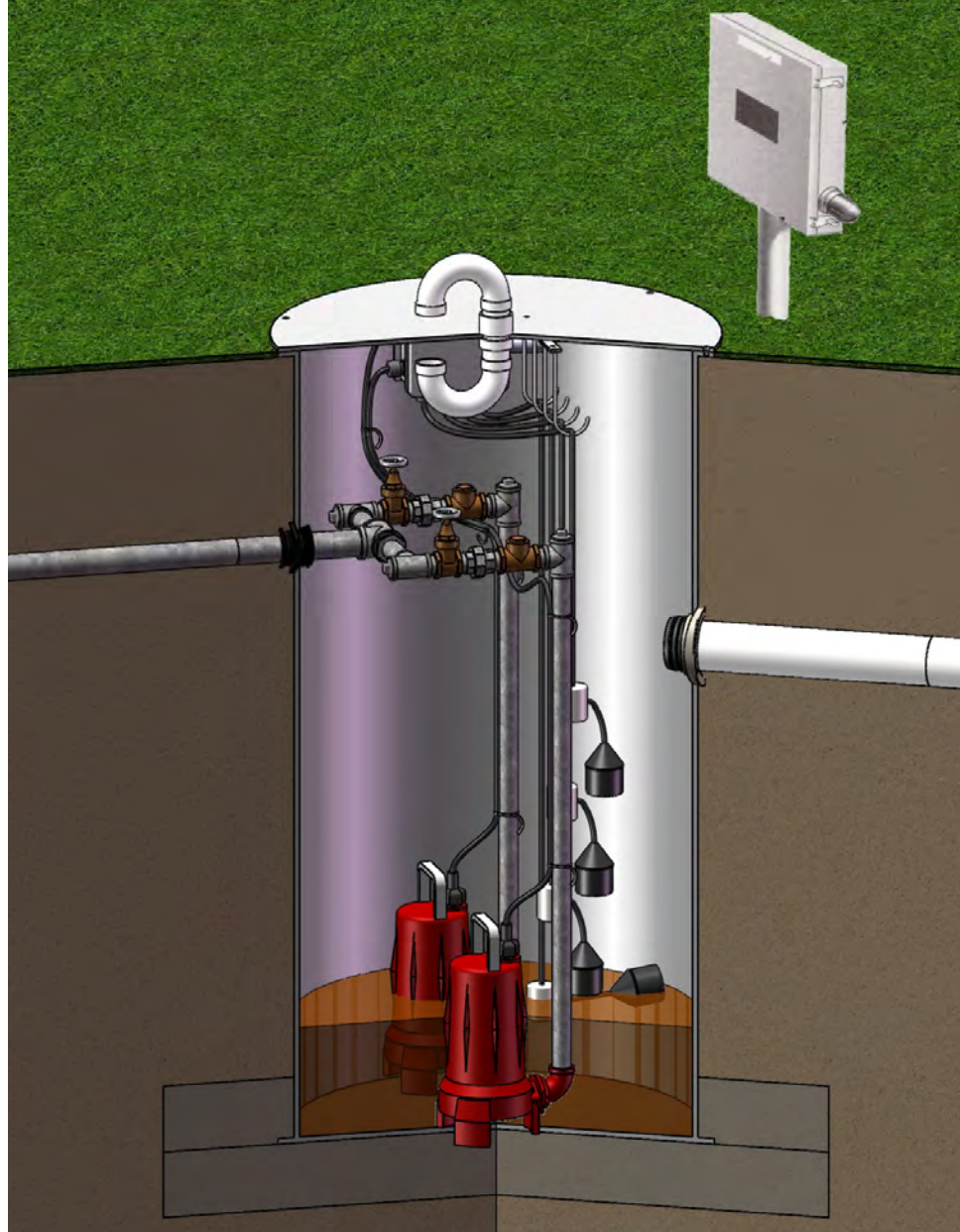
1. Any goods which have been repaired or altered without Supplier's express written consent, in such a way as in the reasonable judgment of Supplier to adversely affect the stability or reliability thereof;
2. To any goods which have been subject to misuse, negligence, acts of God or accidents; or
3. To any goods which have not been installed to manufacturer's specifications and guidelines, improperly maintained, or used outside of the specifications for which such goods were designed.

**A COMPLETELY EQUIPPED,
DEPENDABLE,
DUPLEX GRINDER PUMP,
RAW SEWAGE LIFT STATION**



M/N LDGS Duplex

(featuring the Liberty grinder)



**Smith
Pump
Company, Inc.**

301 M&B Industrial
800-299-8909 254-776-0377

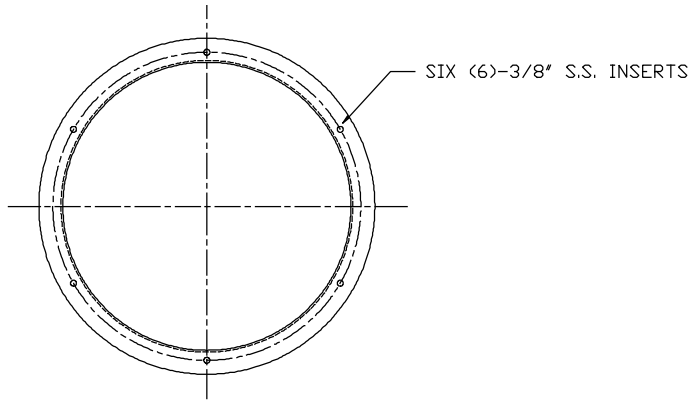
Waco, TX 76712
FAX 254-776-0023

4624 Martin Luther King Fwy	Fort Worth, TX 76119	(817) 589-2060	FAX (817) 595-4900
1900 Howard Lane West	Austin, Texas 78728	(512) 310-1480	FAX (512) 310-1417
13126A Lookout Ridge	San Antonio, TX 78233	(210) 656-0530	FAX (210) 656-2372
1216 East Jasmine, Suite C	McAllen, TX 78501	(956) 687-9949	FAX (956) 687-9969

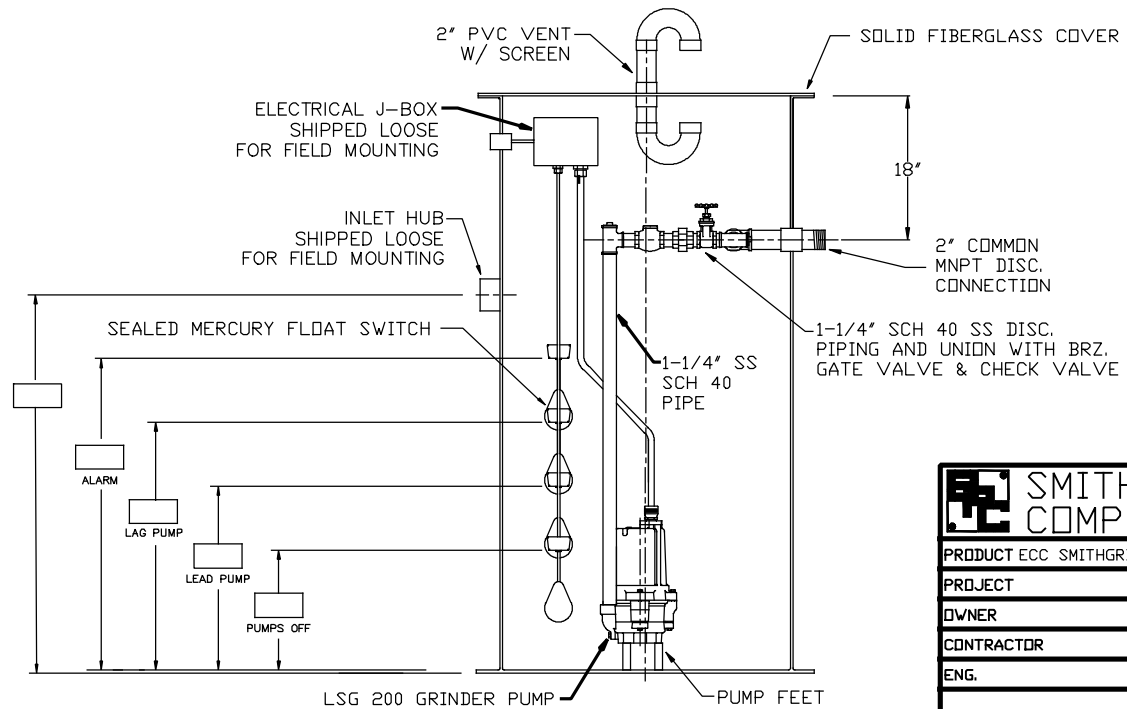
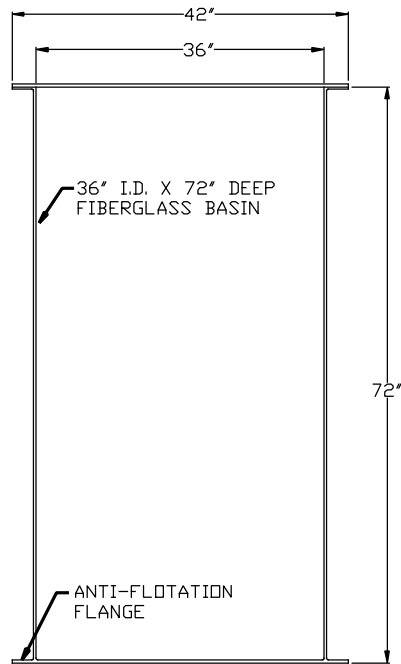
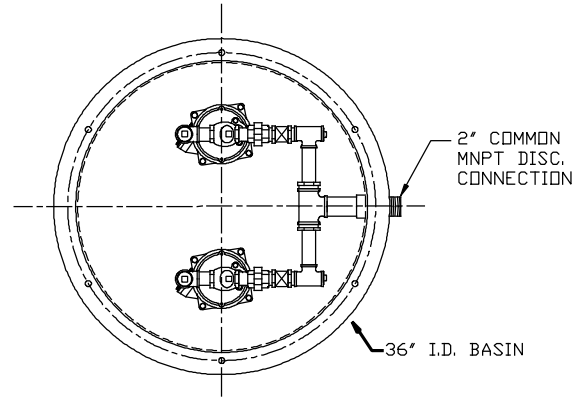
NOTES:


1. LIBERTY SUBMERSIBLE GRINDER PUMPS. M/N LSG200 (2 HP 230V/1PH).
2. DISCHARGE PIPING TERMINATES AT ONE (1) 2" M.N.P.T. CONNECTION.
3. THE INLET/DISCHARGE HUBS ARE "SHIPPED LOOSE" FOR FIELD INSTALLATION.
4. THE "J-BOX" IS "SHIPPED LOOSE" FOR FIELD INSTALLATION.

TOP VIEW



SYSTEM DETAIL



 SMITH PUMP COMPANY				
PRODUCT ECC SMITHGRIND				
PROJECT				
OWNER				
CONTRACTOR				
ENG.				
DWG. NO. T\ECCDUPLEX-LIBERTY				
DATE	SCALE	BY	CK	REV
5-9-2013	NONE	SPCD	SPCD	---

COMPANY CONFIDENTIAL INFORMATION CONTAINED HEREIN IS CONFIDENTIAL TO THE PROPERTY OF SMITH PUMP COMPANY. IT IS TO BE KEPT STRICTLY CONFIDENTIAL AND NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS WITHOUT THE PRIOR WRITTEN CONSENT OF SMITH PUMP COMPANY.

Liberty Pumps®

LSG200-Series



2 hp

1-1/4" Discharge

Features:

- New Patented V-Slice® cutter technology
- One-piece uni-body casting
- Stainless steel impeller
- Quick-disconnect power cord
- Internal or external capacitor models available
- 300 Series SS Rotor Shaft

U.S. Patent # 7,159,806

innovate. evolve.





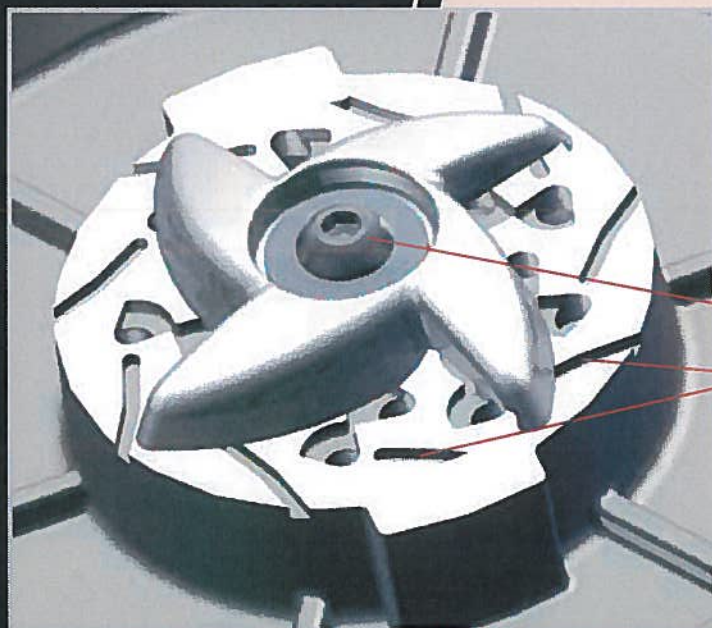
Liberty's LSG200-Series Grinder Pumps meet the demanding needs of commercial and residential sewage applications where difficult solids-handling ability is crucial. The LSG200-Series features a superior cutting system made of hardened 440 stainless steel – Rockwell C 58, for shearing solids into small particles prior to being passed to the discharge by the impeller under high pressure. Applications include individual or groups of homes, motels, schools, shopping centers, lakefront developments and systems requiring high pressure sewage pumping.

LSG200-Series Grinder Pumps

Features:

- 2 hp, heavy-duty motor – oil filled, thermally protected
- Upper and lower ball bearings
- One-piece uni-body cast iron housing
- 300 Series SS Rotor Shaft
- 316 Stainless steel impeller
- Dual seals – Upper seal is unitized durable silicon carbide. Lower seal is Viton® double-lip. (Lower seal ensures that all debris is kept away from main seal)
- Motor windings insulated to Class B (130°C)
- Advanced V-Slice® cutting system made of hardened 440 stainless steel – Rockwell C 58
- Horizontal 1-1/4" FNPT Discharge
- Back vanes on impeller and spiraled bottom plate for superior solids clearing
- All stainless steel fasteners
- Clog-free volute design
- Designed for maximum heat dissipation and cool motor operating temperatures
- Solid state starting circuit - no mechanical relay coil
- 25' power cord with Quick-Disconnect
- Piggy back plug with wide angle float (on automatic model) eliminates need for expensive panel

Viton® is a registered trademark of DuPont Dow Elastomers LLC.



V-Slice® Technology

Superior cutting system provides improved shredding performance over radial cutters. V-pattern provides up to 108 alternated cuts per revolution. Entire cutting system made of 440 stainless steel hardened to 58Rc.

Recessed cutter bolt eliminates wadding

Exclusion cleanout slots and back relief clears debris from under cutter

U.S. Patent # 7,159,806

inno

Stainless steel clasp for lift-out chain

Large stainless steel handle with rubber grip

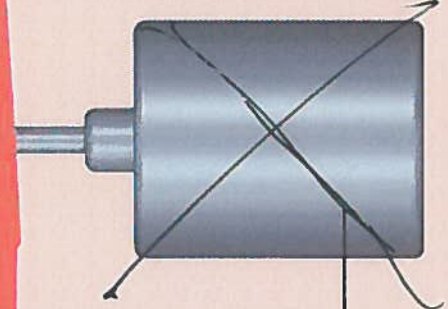
Quick-disconnect power cord

Dependable solid state starting circuit

Internal start/run capacitors on single phase models. "C" models have external caps and require a control panel

Thermal overload (single phase only)

2 hp motor



Wide angle float with piggy-back plug

Unique one-piece unibody casting

Silicon carbide upper seal

300 Series SS Rotor Shaft

316 SS impeller

Viton® double-lip lower seal

Robust motor plate

440 SS cutter plate spiraled for solids clearing

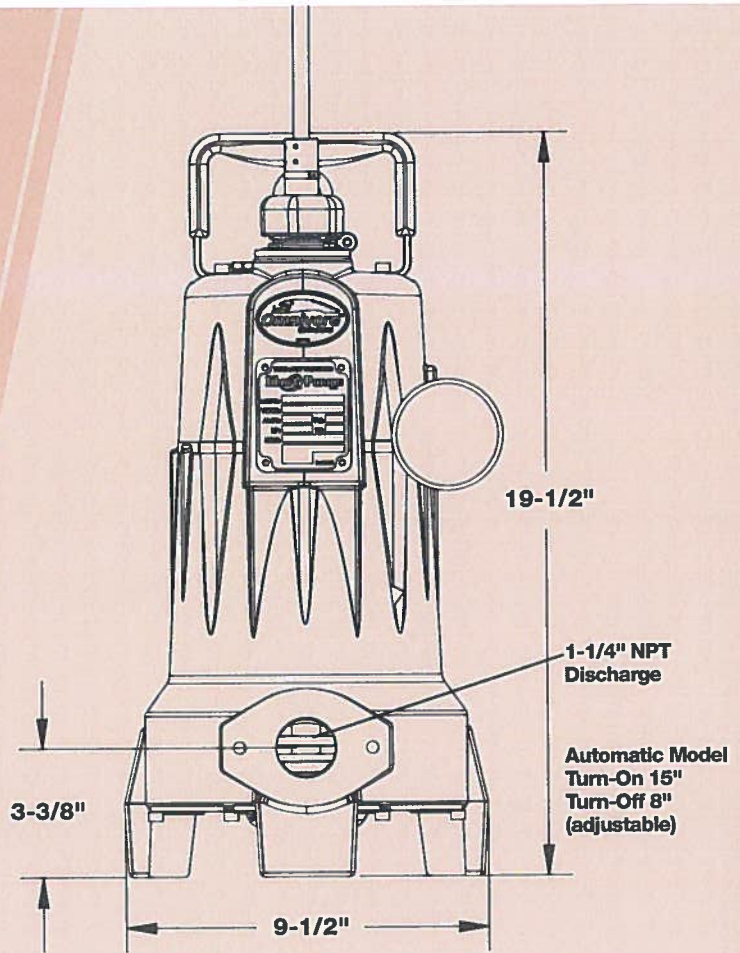
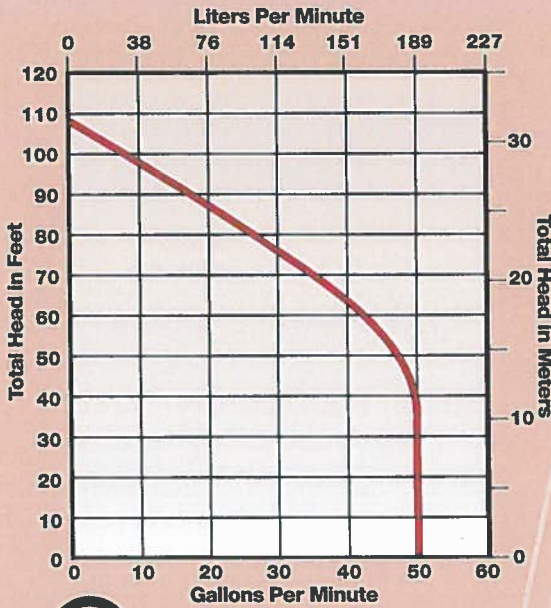
1-1/4" NPT Discharge

440 SS rotary cutter hardened to 58Rc

Improved clog-free volute design

vate. evolve.

LSG200-Series Specifications



Model	HP	Volts	Phase	HZ	Amps	Locked Rotor Amps	Speed (RPM)	Discharge	Switch	Weight
LSG202A	2	208-230	1	60	15	53	3450	1-1/4"	Yes	86
LSG202M	2	208-230	1	60	15	53	3450	1-1/4"	No	84
LSG202M-C	2	208-230	1	60	15	53	3450	1-1/4"	No	84
LSG203M	2	208/230	3	60	10.6	62	3450	1-1/4"	No	84
LSG204M	2	440-480	3	60	5.3	31	3450	1-1/4"	No	84
LSG205M	2	575	3	60	4.9	31	3450	1-1/4"	No	84

Single phase models are thermally protected. 3-phase models require a properly sized control panel. Maximum fluid temperature 140° F

LSG202M and LSG202A feature internal capacitors and do not require a separate control panel for operation. LSG202M-C features external capacitors, requiring a panel with appropriately sized start and run capacitors.

Options for LSG202M-C: External Cap Grinder

Model	Description
K001316	Start/Run Capacitor Kit (for retrofit in existing panels)
SXHC24=3	Simplex NEMA 4X Panel with start/run capacitors
AE24HC=3	Duplex NEMA 4X Panel with start/run capacitors

For complete panel specifications, see SX or AE-series literature.
25' cord standard on all models. LSG202M-C features 35' cord standard.

www.libertypumps.com



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Phone 800-543-2550 Fax (585) 494-1839

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**SMITH
PUMP
COMPANY, INC.**

301 M&B Industrial
(800) 299-8909 (254) 776-0377

Waco, Texas 76712
FAX (254) 776-0023

Duplex Control Panel Summary Sheet

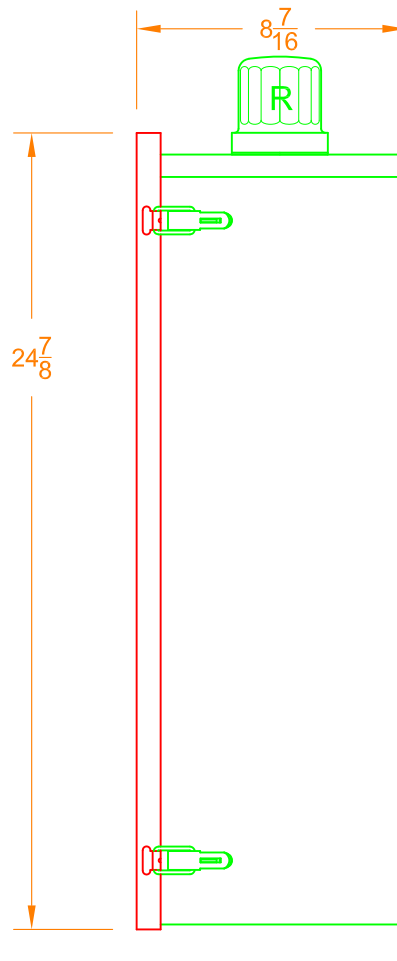
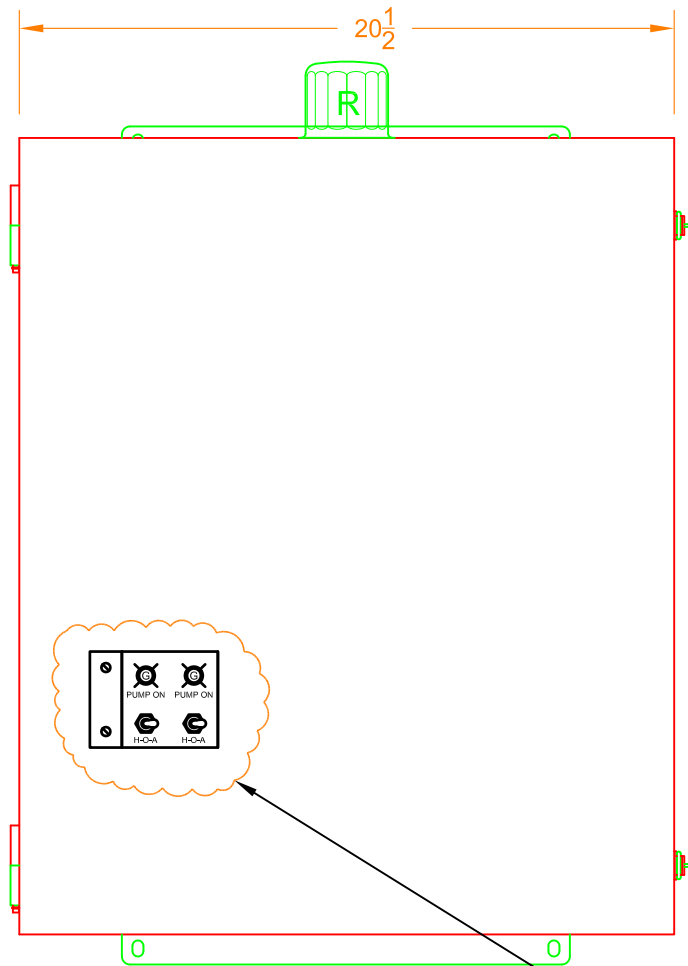
Panel No. **6002-005**

March 28, 2012

Voltage: **230V** Phase: **1** Hz: **60**
H.P. Pump 1: **2** FLA: **15** H.P. Pump 2: **2** FLA: **15**
Enclosure: **NEMA 3R** Size: **24" T x 20" W x 8"D**
Operation: **Four N.O. Floats**

DESCRIPTION:

- Power Distribution Block
- Molded Case Motor Circuit Breakers
- Molded Case Control/ Alarm Circuit Breaker
- Individually Fused Control/ Alarm Circuits
- IEC Motor Contactors with Temperature Compensated Overload Relays
- Hand-Off-Automatic Toggle Switches
- Green Pump Run Lights
- High Water Float Activates Main Flashing Alarm Light




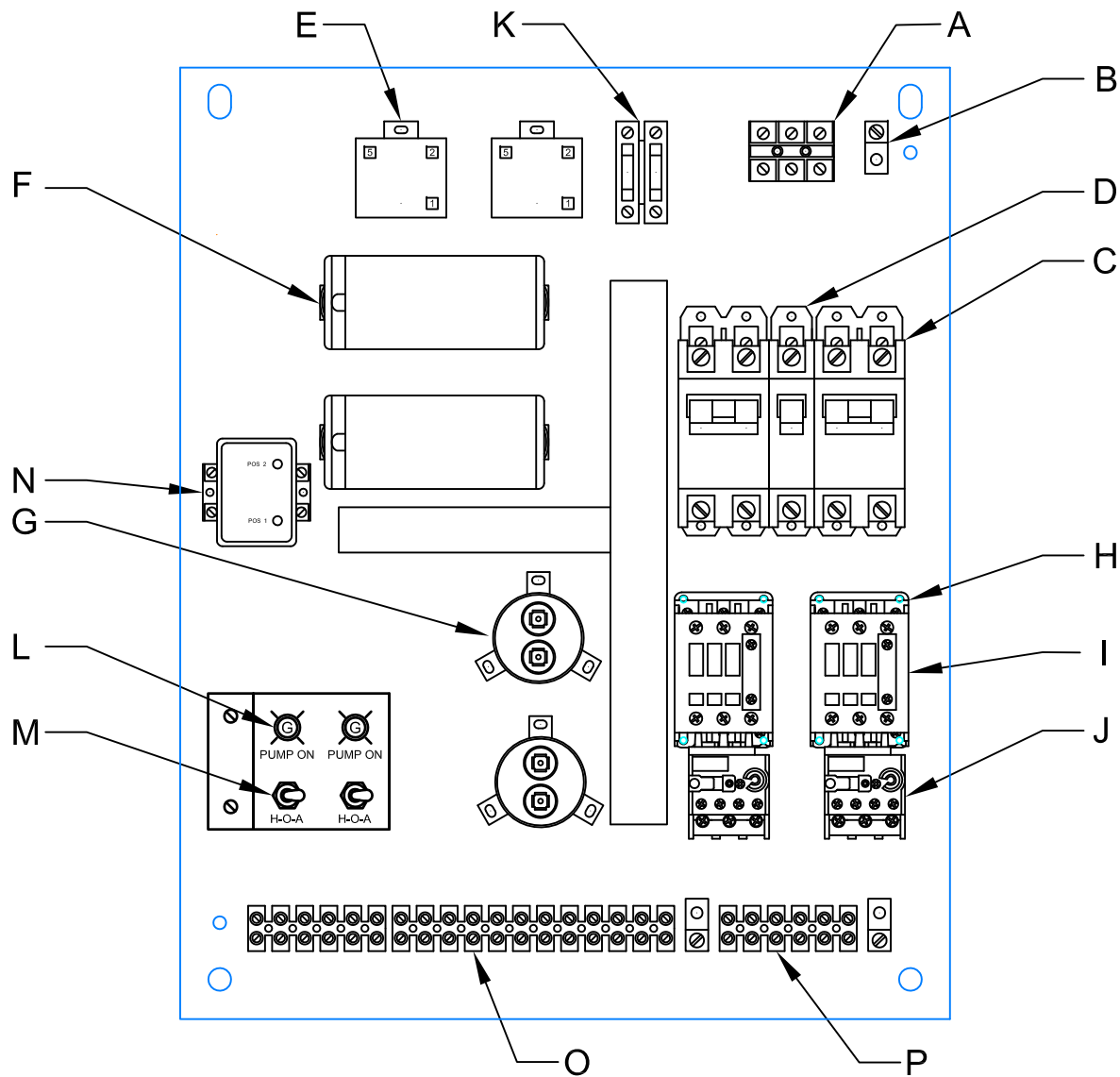
INTERNAL CONTROLS

AS BUILT

NOTE:

1. ENCLOSURE IS 24 x 20 x 8" NEMA 3R HOFFMAN.
2. ALL CONTROLS ARE MOUNTED INTERNALLY.
3. RED FLASHING MAIN ALARM LIGHT.

REV	DATE	DESCRIPTION		
 SMITH PUMP COMPANY				
PRODUCT DUPLEX CONTROL PANEL: 2HP, 230V/ 1PH				
PROJECT SPCO LIBERTY PUMP SUPPLEMENTAL PANEL				
OWNER SMITH PUMP COMPANY				
CONTRACTOR TBD				
PANEL NO. 6002-005				
PROJECT NO TBD				
DWG. NO. T:\CAD DWG\CP...\LAYOUT 6002-005.DWG				
DATE	SCALE	BY	CK	REV
11/11/2010	1:6	JDS	SPCO	-




ITEM	QTY	DESCRIPTION
A	1	INCOMING POWER BLOCK
B	4	GROUNDING LUG
C	2	TWO POLE 230V BREAKER FOR PUMPS
D	1	SINGLE POLE BREAKER FOR CONTROLS
E	2	START RELAY
F	2	START CAPACITOR
G	2	RUN CAPACITOR
H	2	MOTOR CONTACTOR
I	2	AUXILLARY CONTACT
J	2	MOTOR OVERLOAD RELAY
K	1	FUSEBLOCK WITH FUSES
L	2	PUMP RUN LIGHT
M	2	HAND-OFF-AUTOMATIC TOGGLE SWITCH
N	1	ALTERNATOR RELAY
O	1	TERMINAL BLOCK - FLOATS, HMWT
P	2	MOTOR WIRING TERMINALS

AS BUILT

NOTE:

1. ACTUAL COMPONENTS MAY VARY FROM THOSE DEPICTED.
2. PUMP BREAKERS & CAPACITORS ARE SIZED BY APPLICATION.
3. PANEL AS SHOWN FITS HOFFMAN/ WIEGMANN 24x20x8" ENCLOSURE.

REV	DATE	DESCRIPTION		
-	--/--	---		
 SMITH PUMP COMPANY				
PRODUCT DUPLX CONTROL PANEL: 2HP, 230V/ 1PH				
PROJECT SPCO LIBERTY PUMP SUPPLEMENTAL PANEL				
OWNER SMITH PUMP COMPANY				
CONTRACTOR TBD				
PANEL NO. 6002-005				
PROJECT NO. TBD				
DWG. NO. T:\CAD DWG\CP...\LAYOUT 6002-005.DWG				
DATE	SCALE	BY	CK	REV
11/11/2010	1:4	JDS	SPCO	-



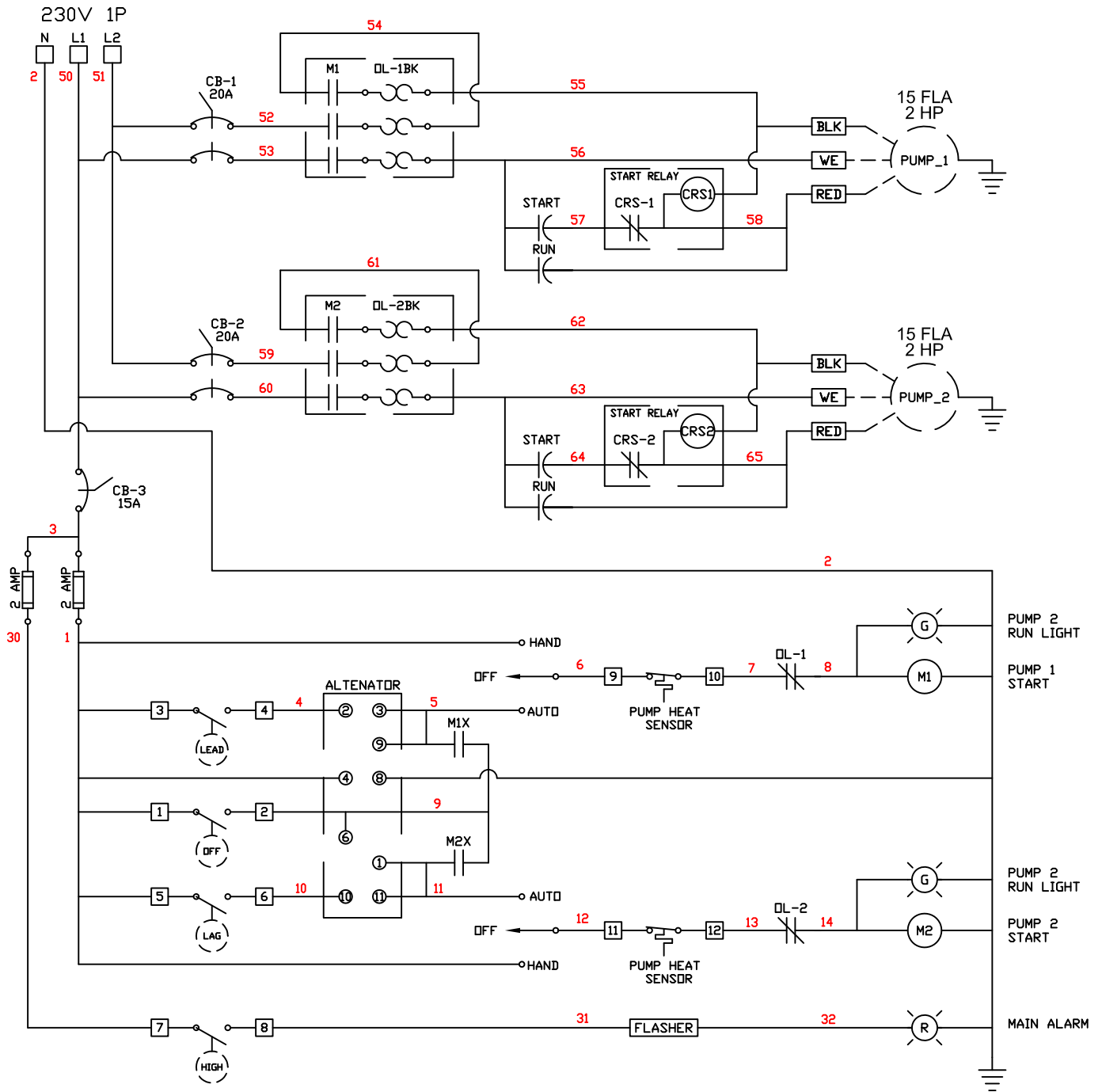
DISCONNECT MEANS PROVIDED BY OTHERS

PUMP: LSG202C-M 15 FLA

OPTIONS:

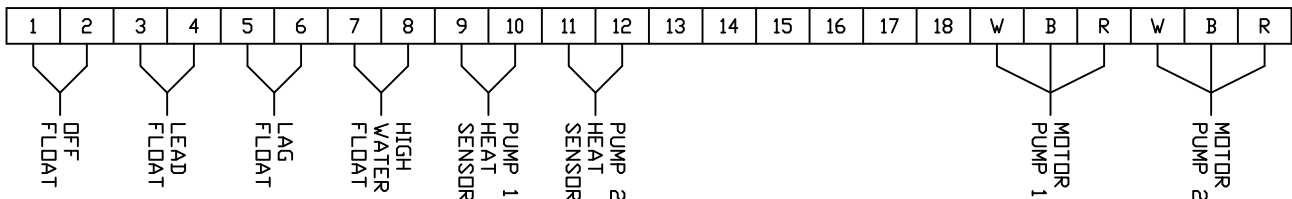
1. START RELAY & CAPACITORS
2. FLASHING MAIN ALARM LIGHT

AS BUILT



TERMINAL LAYOUT

REMOVE HEAT SENSOR JUMPER IN FIELD AND CONNECT TO HIGH MOTOR WINDING TEMPERATURE SENSOR.





BILL OF MATERIAL

DUPLEX CONTROL PANEL: LIBERTY MODEL# LSG202M-C
NEMA 3R PAINTED STEEL CONTROL PANEL

MODEL NO: 6002-005
2 HP, 230 Volt , 1 Phase

SHEET	QTY	DESCRIPTION	MANUFACTURER	PART NO.
1	1	NEMA 3R Steel Enclosure: 24" x 20" x 8"	Hoffman	A24R208HCLO
2	2	IEC Contactor, 25 Amp, 120 Volt Coil, 3 Phase	c3controls	300-S25N30D00
3	2	Normally Open Auxilary Contact Block, 6 Amps	c3controls	300-SFA10
4	2	IEC Bimetallic Overload Relay: 11-17 Amps, 3 Phase	c3controls	320-B2U17
5	2	Potential Start Relay: 332VAC, 243/271 Max, 40/105 Min	Zettler Controls	ZCPRZ2AM2
6	2	Start Capacitor, 216-259 Mfd, 220/250V	Essex	BC-216M-250
7	2	Run Capacitor, 50 Mfd, 370V, Round	Essex	RD-50-370/ 12225
8	2	Start Capacitor Horizontal Mounting Bracket	Essex	BCH-5
8	2	Start Capacitor End Cap	Essex	BCH-27
9	1	Circuit Breaker: 1 Pole, 15 Amps, 120/240 Volts, UL489	General Electric	THQC1115WL
9	2	Circuit Breaker: 2 Pole, 20 Amps, 120/240 Volts, UL489	General Electric	THQC2120WL
9	5	Circuit Breaker Mounting Bracket	General Electric	TQCBMPA
10	3	Grounding Lug	ILSCO	TA-1
11	1	3 Pole Power Splice Block, 90 Amp, 600 Volt, Thermoplastic	Bussmann	NDN111-WH
12	2	12 Pole Double Row Terminal Block, 50 Amp, 600 Volt, #8-#20 AWG	Bussmann	TS140112
13	1	Red Alarm Light, 120 Volt, 15 Watt	Conery	R25-LG-15
13	1	Solid State Flasher, 75 Flashes per Minute	Conery	FL-120V
14	2	Spring Loaded 3 Postion Toggle Switch, "H-O-A"	Gaynor	7252C
15	2	Neon Green Indicating Lamp, 120 Volt	ITW Jemco	XL-7216-45JL
16	1	2 Pole Fuse Block, 30 Amp, 300 Volt	Bussmann	S-8301-2
17	2	2 Amp Fuse, 250 Volt	Bussmann	AGC2R
18	1	Duplex Alternating Relay, 110 Volt, DPDT, 11 Pin	ATC Diversified	ARA120ADA
19	1	11 Pin Socket	Custom Connectors	OT11-PC
20	1	Corrosion Inhibitor Strip	Hoffman	AHCI-1DV